ADDENDUM NO. 1

BID PACKAGE NUMBERS: **All Bidders**

PROJECT **:** ***Bay District Schools***
Mosley Softball Field House

This Addendum is a part of the Contract Documents.

This Addendum #01 consists of Items 1.1 through 1.6:

Item 1.1	- JRA Addendum #1 dated April 10, 2026, which includes civil plan modifications.
Item 1.2	- JRA Addendum #2 dated April 17, 2026, which includes clarifications for spray foam insulation on exterior CMU, Geotechnical Report and plumbing revisions.
Item 1.3	- JRA Addendum #3 dated April 23, 2026, which includes clarifications for dryer exhaust, condensate drain routing and updates to the architectural site plan and floor plan.
Item 1.4	- Prebid RFI Responses (#1- #18) are being included.
Item 1.5	- Bidders Scope Checklist (Please note that these are to be in your bid folders as part of your bid package)
Item 1.6	- Bid date remains Tuesday, May 12, 2026, at 2:00pm CDT at the Bay District Schools.

End of Addendum #01
Dated This 23rd Day of April, 2026

CHILDERS
CONSTRUCTION CO.

Addendum #1

Mosley Softball Fieldhouse

JRA Commission. 25871 CA/BC

Bay District Schools
1311 Balboa Avenue
Panama City, Florida 32401

JRA Architects, Inc.
2211 Navy Blvd- Suite 100
Panama City, Florida 32408

Date of Issue of addendum: **April 10, 2026**

The changes herewith form part of the Construction Documents and modify the original "Construction Documents" dated April 3, 2026.

This Addendum consists of **1** page(s), 0 attachment(s) and **2** revised plan sheet(s) attached and as referenced herein.

ITEM 1.1

Drawings: Replace existing drawing sheets C1.1, C1.2, C1.3, C1.4, C1.5, C1.6, C1.7, C1.8, C1.9, and C1.10 with **Attached Revised Drawing Sheets C1.1, C1.2, C1.3, C1.4, C1.5, C1.5A, C1.6, C2.1, C2.2, C2.3, and C2.4.** (Note revised sheets contain an additive alternate)

END OF ADDENDUM #1

SITE DEMOLITION DRAWING NOTES:

1. SEE SYMBOL LEGEND ON THIS SHEET FOR SYMBOL INFORMATION AND REFERENCED DETAILS.
2. ALL DEMOLISHED MATERIALS (ie., SIGNS, CONCRETE, ASPHALT, ETC...) TO BE REMOVED AND DISPOSED OF IN A LEGAL MANNER.
3. ALTHOUGH EVERY ATTEMPT TO LOCATE UNDERGROUND UTILITIES HAS BEEN MADE, THERE IS THE POSSIBILITY OF UNDERGROUND GAS, ELECTRICAL, WATER SEWER, ETC... THAT HAS NOT BEEN LOCATED. THE CONTRACTOR SHALL FIELD VERIFY THE LOCATION OF ALL UNDERGROUND UTILITIES PRIOR TO CONSTRUCTION.
4. THE DEVELOPER AND/OR CONTRACTOR IS RESPONSIBLE FOR FOLLOWING REQUIRED WASTE MANAGEMENT PRACTICES AS DEFINED IN THE LYNN HAVEN MUNICIPAL CODE SECTION 54-32 "BUILDING CONSTRUCTION WASTES", WHICH MAKES IT UNLAWFUL FOR ANY PERSON TO DUMP, LEAVE OR BURY ANY SOLID WASTE ON PUBLIC OR OR PRIVATE PROPERTY.
5. IT IS THE CONTRACTORS RESPONSIBILITY TO CALL SUNSHINE ONE AT 811 FOR UTILITY LOCATES PRIOR TO CONSTRUCTION.
6. REMOVE AND RELOCATE CLAY PILE PER BAY DISTRICT SCHOOL INSTRUCTIONS.

SYMBOL LEGEND

N1 (SEE NOTE ie., #1 – SEE NOTES ON THIS SHEET)

RM (REMOVE EXISTING MATERIALS)

McNEIL CARROLL
SCALE
1" = 10'

0 05 10 20

ROBERT L. CARROLL LICENSE No. 57988 STATE OF FLORIDA PROFESSIONAL ENGINEER

2/6/2026

Robert L. Carroll, P.E.
PROFESSIONAL ENGINEER
FL LC # 57988

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REVISIONS

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DESIGN DEVELOPMENT	MAY	SLB	1/9/26
CONSTRUCTION DOCUMENTS	MAY	SLB	4/3/26

JRA ARCHITECTS
2211 NAVY BLVD. , STE 100
PANAMA CITY BEACH, FL
PHONE: (850) 236-9832
Commission Number: 25871

CONSULTANTS:

PROJECT:

BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD
HOUSE
MOSLEY HIGH SCHOOL

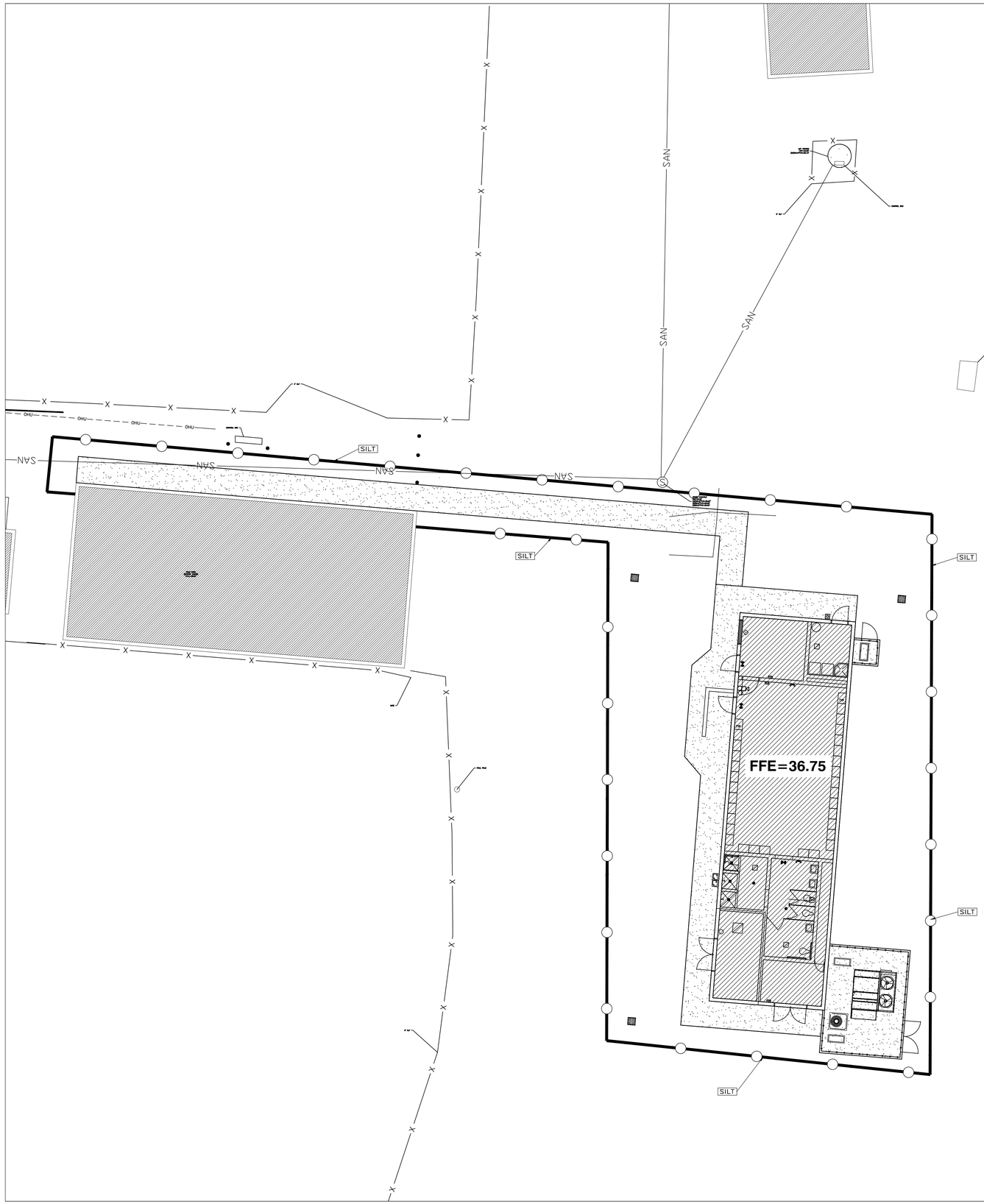
LYNN HAVEN, FLORIDA

SHEET TITLE:

SITE DEMOLITION CONTROL PLAN

SHEET NUMBER:

C1.1



SITE EROSION CONTROL DRAWING NOTES:

1. EROSION CONTROL SHALL BE MAINTAINED FOR THE DURATION FOR THE PROJECT.
2. ALL CONSTRUCTION OUTSIDE OF PROPERTY LINES IS SHOWN IN DETAIL ON PERMIT DRAWINGS. (SEE GENERAL NOTES.).
3. SEE SYMBOL LEGEND ON THIS SHEET FOR SYMBOL INFORMATION AND REFERENCED DETAILS.
4. SEE SECTIONS IN CONSTRUCTION DETAILS.
5. SILT FENCE TO BE INSTALLED AT PERIMETER OF SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES WILL BE UTILIZED THROUGHOUT THE CONSTRUCTION PHASE OF THIS PROJECT TO RESTRICT ANY TURBID RUNOFF FROM LEAVING THE CONSTRUCTION SITE.
6. CONTROL OF SEDIMENT-LADEN RUNOFF SHALL BE PROVIDED WITH HAY BALES AND/OR GEOTECH STYLE FABRICS. ALL CONTROL MEASURES SHALL BE PROPERLY LOCATED AND CONSTRUCTED TO PREVENT SEDIMENT TRANSPORT. THE MEANS FOR RETAINING THE SEDIMENTS WILL BE MAINTAINED BY THE CONTRACTOR UNTIL PERMANENT IMPROVEMENTS ARE COMPLETE.
7. THE CONTRACTOR IS RESPONSIBLE FOR TREATING ALL ONSITE STORMWATER DRAINAGE AS REQUIRED TO MEET THE CRITERIA OF 62-3 FLORIDA ADMINISTRATIVE CODE, F.A.C. PRIOR TO DISCHARGE.
8. ALL CATCH BASINS, INLETS AND ACCESSES TO UNDERGROUND STORMWATER SYSTEMS SHALL BE PROTECTED IN ACCORDANCE WITH THE ATTACHED DETAILS.
9. THE CONTRACTOR IS RESPONSIBLE FOR COMPLYING WITH THE TERMS AND CONDITIONS OF ANY STORMWATER PERMITS THAT MAY APPLY (FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION, FLORIDA DEPARTMENT OF TRANSPORTATION, CITY OF LYNN HAVEN, WATER MANAGEMENT DISTRICT, ETC.).
10. CONSTRUCTION DRIVES SHALL SLOPE AWAY FROM THE ROADWAY AT A MINIMUM SLOPE OF 2.00% TO DISTANCE OF NOT LESS THAN 15 FEET FROM THE EDGE OF PAVEMENT. THE MAXIMUM WIDTH OF THE DRIVE SHALL BE 30 FEET WITH #57 GRAVEL SURFACE 6 INCHES THICK. SIGNS SHALL BE PLACED (IN ACCORDANCE WITH CITY AND STATE REQUIREMENTS) TO WARN APPROACHING DRIVERS AND PEDESTRIANS.
11. THE DEVELOPER AND/OR CONTRACTOR IS RESPONSIBLE FOR FOLLOWING REQUIRED WASTE MANAGEMENT PRACTICES AS DEFINED IN THE LYNN HAVEN MUNICIPAL CODE SECTION 54-32 "BUILDING CONSTRUCTION WASTE", WHICH MAKES IT UNLAWFUL FOR ANY PERSON TO DUMP, LEAVE OR BURY ANY SOLID WASTE ON PUBLIC OR OR PRIVATE PROPERTY.
12. THE DEVELOPER AND/OR CONTRACTOR IS RESPONSIBLE FOR OBTAINING COVERAGE UNDER THE FDEP GENERIC PERMIT FOR STORMWATER DISCHARGE FROM LARGE AND SMALL CONSTRUCTION ACTIVITIES PRIOR TO START OF CONSTRUCTION OR ANY DISTURBANCE OF LAND GREATER THAN 1 ACRE. THE DEVELOPER/CONTRACTOR WILL FORWARD A COPY OF THE PERMIT AND WILL PROVIDE 48 HOUR NOTIFICATION TO THE APPROPRIATE AGENCIES PRIOR TO COMMENCEMENT OF CONSTRUCTION. ALL REQUIRED ELEMENTS OF THE SWPP MUST BE IN PLACE PRIOR TO COMMENCEMENT OF CONSTRUCTION. FAILURE TO COMPLY COULD RESULT IN CODE ENFORCEMENT ACTION AND FINES.
13. QUALIFIED PERSONNEL SHALL INSPECT THE FOLLOWING ITEMS AT LEAST ONCE EVERY SEVEN (7) CALENDAR DAYS AND/OR WITHIN 24 HOURS OF THE END OF A STORM EVENT (RAINFALL) THAT IS A 1/2 INCH OR GREATER:
    A. POINTS OF DISCHARGE TO WATERS OF THE UNITED STATES.
    B. POINTS OF DISCHARGE TO MUNICIPAL SEPARATE STORM WATER SYSTEMS.
    C. DISTURBED AREAS OF THE SITE THAT HAVE NOT BEEN FINALLY STABILIZED.
    D. AREAS USED FOR STORAGE OF MATERIALS THAT ARE EXPOSED TO PRECIPITATION.
    E. STRUCTURAL CONTROLS.
    F. LOCATIONS WHERE VEHICLES ENTER OR EXIT THE SITE.
14. THE CONTRACTOR SHALL INITIATE REPAIRS WITHIN 24 HOURS OF INSPECTION THAT INDICATE ITEMS ARE NOT IN GOOD WORKING ORDER. TO COMPLY, THE CONTRACTOR SHALL INSTALL AND MAINTAIN RAIN GAGES AND DAILY RAINFALL RECORDS. WHERE SITES HAVE BEEN PERMANENTLY STABILIZED, INSPECTIONS SHALL BE CONDUCTED AT LEAST ONCE EVERY MONTH. THE CONTRACTOR SHALL ALSO INSPECT AND CERTIFY THAT CONTROLS INSTALLED IN THE FIELD AGREE WITH THE LATEST STORMWATER POLLUTION PREVENTION PLAN.
15. IF INSPECTIONS INDICATE THAT THE INSTALLED STABILIZATION AND STRUCTURAL PRACTICES ARE NOT SUFFICIENT TO MINIMIZE EROSION, RETAIN SEDIMENT, AND PREVENT DISCHARGING POLLUTANTS, THE CONTRACTOR SHALL PROVIDE ADDITIONAL MEASURES, WHERE SITES HAVE BEEN PERMANENTLY STABILIZED, INSPECTIONS SHALL BE CONDUCTED AT LEAST ONCE EVERY MONTH. AS NEEDED.
16. RECORDS OF THE INSPECTIONS AND THE CONSTRUCTION PERMIT MUST BE MAINTAINED AT THE CONSTRUCTION SITE AND BE READILY AVAILABLE FOR INSPECTION.
17. ALL STORMWATER MANAGEMENT FACILITIES AND EROSION CONTROL MEASURES SHALL BE INSTALLED PRIOR TO COMMENCEMENT OF ANY CONSTRUCTION, DEMOLITION OR OTHER DISTURBANCE TO THE SUBJECT SITE.

CONSTRUCTION SEQUENCE AND BMP'S NWFWMD

18. THE INITIAL PART OF THE CONSTRUCTION PROCESS SHALL BE THE INSTALLATION OF SILT FENCE AROUND THE PERIMETER OF THE AREA THAT IS TO BE DISTURBED TO ENSURE NO TURBID RUNOFF LEAVES THE CONSTRUCTION SITE. THE SILT FENCE SHALL BE INSTALLED PER THE CONSTRUCTION DETAILS. IF THERE IS A POSSIBILITY OF RUNOFF TO A WATER BODY, TURBIDITY CURTAIN SHALL BE INSTALLED PER THE CONSTRUCTION DETAILS. THE SECOND STEP SHALL BE THE INSTALLATION OF THE CONSTRUCTION ENTRANCE AND DEMOLITION OF ANY EXISTING IMPROVEMENTS AS NEEDED (SEE DEMOLITION PLAN). THE THIRD STEP SHALL BE TO CLEAR AND GRUB AREAS WHERE IMPROVEMENTS ARE TO BE INSTALLED. AS FILL IS BROUGHT INTO THE SITE, THE STORMWATER BASIN SHOULD BE CREATED TO CAPTURE ANY OVERLAND FLOW AND ACT AS A SEDIMENT TRAP. IT IS RECOMMENDED THAT THE BASIN BE CONSTRUCTED APPROXIMATELY ½' HIGHER THAN DESIGN AT THIS POINT TO ENSURE ALL SILTS AND FINES ARE REMOVED AT THE TIME OF FINAL GRADING OF THE STORMWATER BASIN.
19. TYPICALLY, THE SANITARY SEWER, STORM SEWER, AND WATER MAINS ARE INSTALLED RESPECTIVELY. UPON INSTALLATION OF THE STORM SEWER, HAY BALES AND FILTER FABRICS SHALL BE USED AT ALL INLET OPENINGS PER THE CONSTRUCTION DETAILS TO THE KEEP THE SYSTEM FREE OF SEDIMENTS DURING THE CONSTRUCTION PHASE. DEPENDING ON SITE CONDITIONS AND SIZE, SEDIMENT TRAPS SHALL BE UTILIZED TO PREVENT TURBID RUNOFF FROM LEAVING THE SITE (SEE EROSION CONTROL PLAN).
20. SITE STABILIZATION SHALL BE PROVIDED AS SOON AS THE GRADING WILL ALLOW IN ORDER TO STOP EROSION AND REDUCE TURBID RUNOFF. SEEDING, SODDING, OR HYDROSEEDING SHALL BE USED WHEN FINAL GRADES ARE ESTABLISHED.
21. EROSION CONTROL MEASURES SHALL BE UTILIZED THROUGHOUT THE CONSTRUCTION PHASE OF THIS PROJECT AND BE MANAGED IN ACCORDANCE THE THE STATE NPDES PROGRAM.
22. THE DESIGN OF THE STORMWATER MANAGEMENT SYSTEM FOR THIS PROJECT COMPLIES WITH THE REQUIREMENTS OF THE FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION AND THE NORTHWEST FLORIDA WATER MANAGEMENT DISTRICT.
23. THE ENGINEER OF RECORD IS RESPONSIBLE FOR MONITORING CONSTRUCTION OF THE STORMWATER MANAGEMENT FACILITY AND SUBMITTING TO THE APPROPRIATE AGENCY NOTICE OF COMMENCEMENT AND AS-BUILT CERTIFICATIONS FOR THE PROJECT WHEN COMPLETED.

SYMBOL LEGEND

- ⟹ (STORMWATER SURFACE FLOW)
- ISB (INLET SEDIMENT BARRIER – SEE CONSTRUCTION DETAILS)
- SILT (SILT FENCE – SEE CONSTRUCTION DETAILS)
- PVG (24' WIDE x 50' DEEP FDOT #1 OR #2 GRAVEL CONSTRUCTION ENTRANCE 6" THICK)

McNEIL CARROLL
SCALE
1" = 10'
0 05 10 20

ROBERT L. CARROLL LICENSE No. 57988 STATE OF FLORIDA PROFESSIONAL ENGINEER
2/6/2026

Robert L. Carroll, P.E.
PROFESSIONAL ENGINEER
FL LIC # 57988

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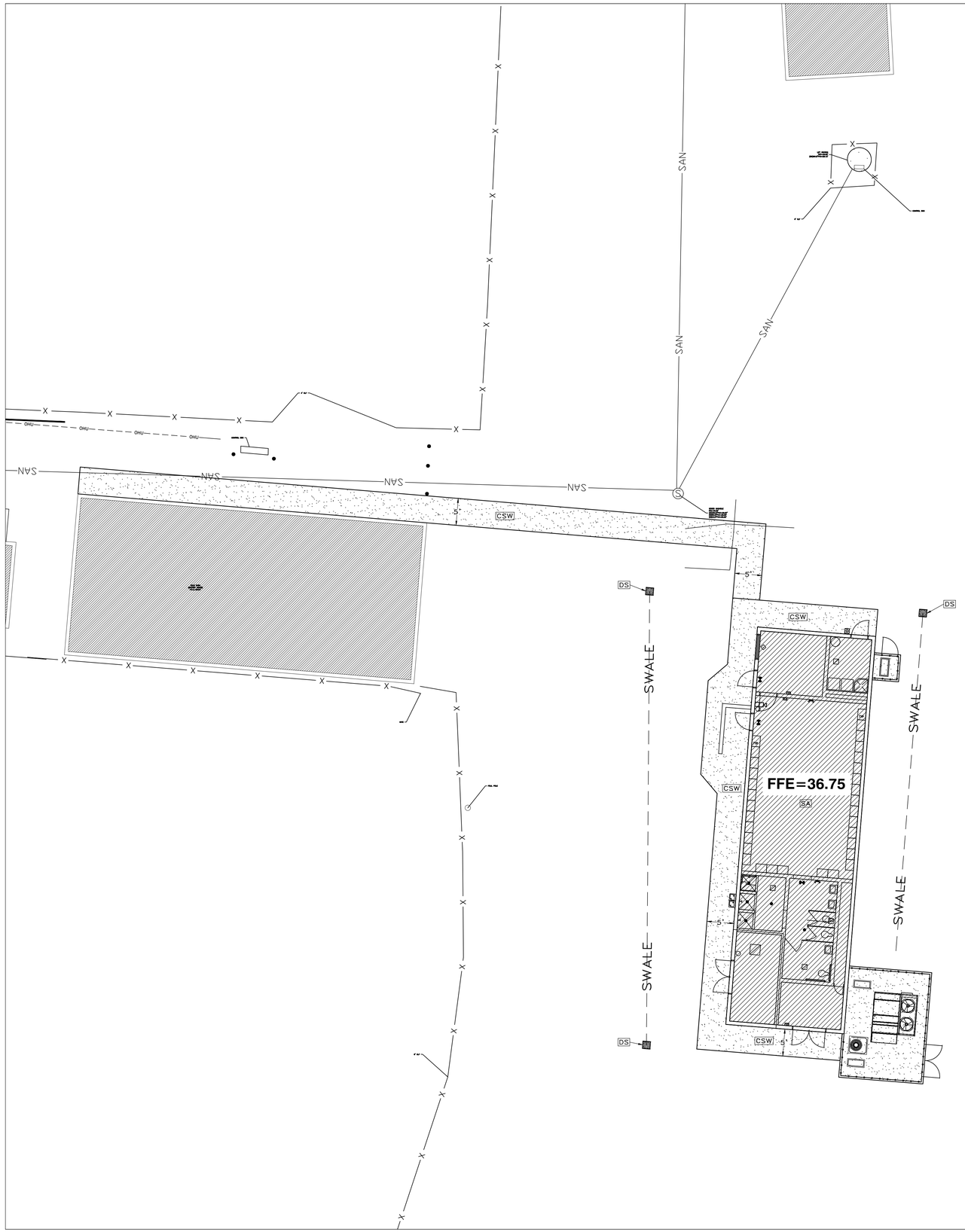
JRA ARCHITECTS
2211 NAVY BLVD. , STE 100
PANAMA CITY BEACH, FL
PHONE: (850) 236-9832
Commission Number: 25871

CONSULTANTS:

PROJECT:
BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD HOUSE
MOSLEY HIGH SCHOOL
LYNN HAVEN, FLORIDA

SHEET TITLE:
SITE EROSION CONTROL PLAN

SHEET NUMBER:
C1.2



SITE LAYOUT DRAWING NOTES:

1. ALL RADII AT FACE OF CURB ARE 5' UNLESS OTHERWISE SHOWN.
2. CONTRACTOR SHALL PROVIDE McNEIL CARROLL ENGINEERING, INC. FIVE (5) SETS OF AS-BUILT DRAWINGS AND ONE (1) DIGITAL COPY (AUTOCAD FORMAT) OF THE COMPLETED PROJECT. DRAWINGS SHALL BE PREPARED AND SIGNED & SEALED BY A FLORIDA REGISTERED SURVEYOR.
3. ALL DIMENSIONS AT CURB ARE FROM FACE OF CURB.
4. ALL DISTURBED AREAS SHALL BE RESTORED TO ORIGINAL CONDITION AND SODDED PER FDOT INDEX 105.
5. A COPY OF ALL REGULATORY PERMITS SHALL BE KEPT ON SITE.
6. THE CONTRACTOR SHALL REVIEW THE COMPLETE NWFWMD PERMIT PRIOR TO CONSTRUCTION COMMENCEMENT.
7. AN 8 1/2 x 11 NWFWMD WEATHER RESISTANT SIGN, INCLUDING THE PERMIT NUMBER SHALL BE PLACED ON THE PROPERTY FACING THE ROAD.
8. ALL PROPOSED UTILITIES TO BE PLACED UNDERGROUND.
9. ALL ABOVE GROUND UTILITIES TO BE SCREENED BY LANDSCAPING.

SYMBOL LEGEND

- CSW (CONCRETE SIDEWALK – SEE CONSTRUCTION DETAILS)
- DS (DRAINAGE STRUCTURE – SEE GRADING & DRAINAGE PLAN)
- N1 (SEE NOTE ie., #1 – SEE NOTES ON THIS SHEET)
- SA (SEE ARCH. PLANS)
- SS (SEWER STRUCTURE – SEE UTILITY PLAN)

McNEIL CARROLL
SCALE
1" = 10'

0 05 10 20

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PANAMA CITY BEACH, FL
PHONE: (850) 236-9832
Commission Number: 25871

CONSULTANTS:

PROJECT:

BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD
HOUSE
MOSLEY HIGH SCHOOL

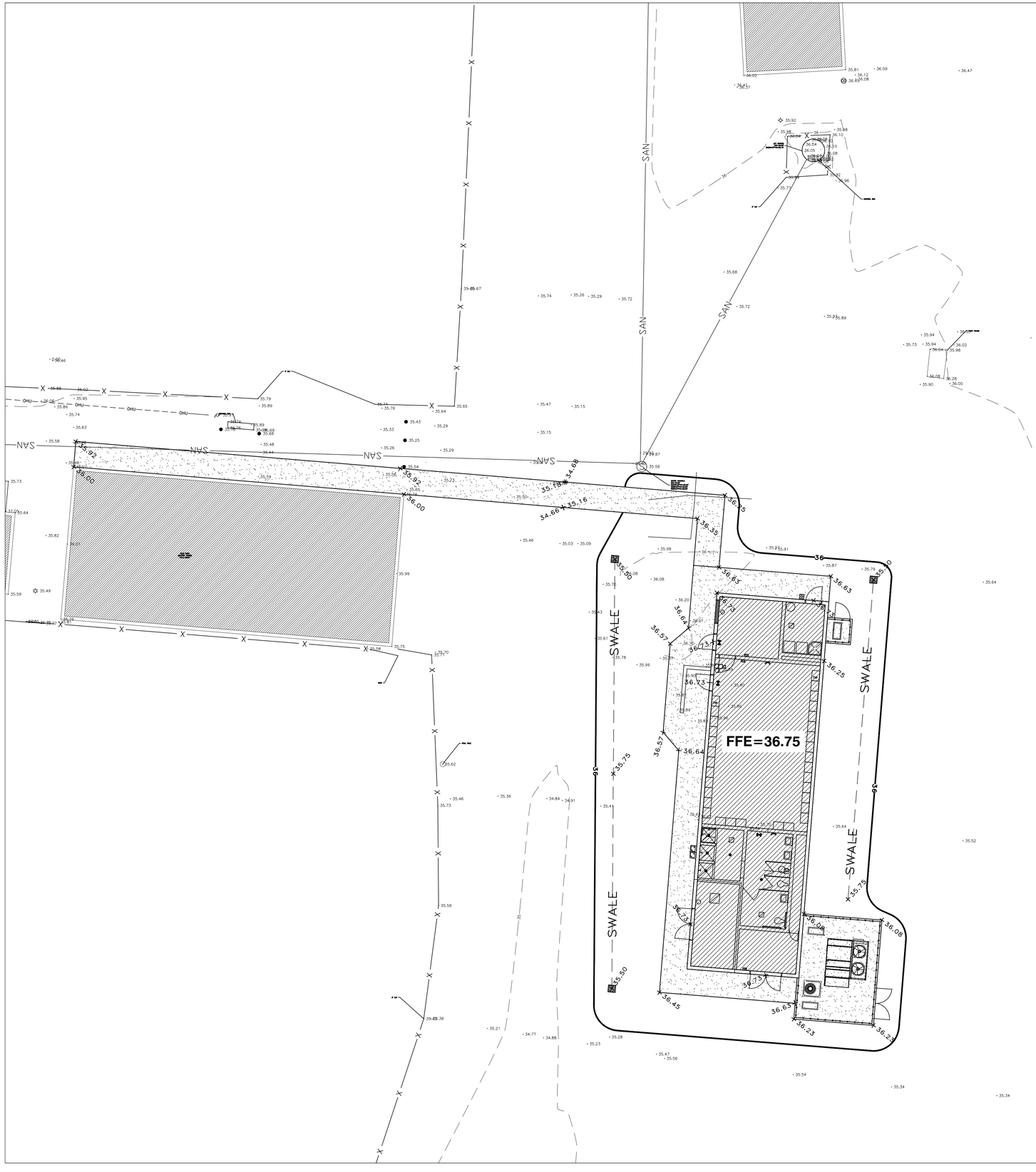
LYNN HAVEN, FLORIDA

SHEET TITLE:

SITE LAYOUT PLAN

SHEET NUMBER:

C1.3



SITE GRADING AND DRAINAGE DRAWING NOTES:

1. SEE SYMBOL LEGEND ON THIS SHEET FOR SYMBOL INFORMATION AND REFERENCED DETAILS.
2. ALL DEMOLISHED MATERIALS (ie., SIGNS, CONCRETE, ASPHALT, ETC...) TO BE REMOVED AND DISPOSED OF IN A LEGAL MANNER. ALL EXISTING MONITORING WELLS ARE NOT TO BE REMOVED. WELLS IN PAVEMENT SHALL HAVE A MANHOLE LID INSTALLED.
3. SEE SECTIONS IN CONSTRUCTION DETAILS.
4. ALL DRAINAGE PIPES TO BE DEDICATED TO THE CITY OF LYNN HAVEN SHALL BE VIDEO RECORDED AND PLACED ON A DIGITAL MEDIA (NO TAPES). VIDEO MUST BE REVIEWED AND APPROVED BY THE CITY.
5. PROPOSED FINISHED FLOORS SHALL BE 1 FOOT (MIN.) ABOVE ROADWAY CENTERLINE.
6. CONTRACTOR SHALL PROVIDE McNEIL CARROLL ENGINEERING, INC. FIVE (5) SETS AND ONE (1) DIGITAL COPY (AUTOCAD FORMAT) OF AS-BUILT DRAWINGS OF THE COMPLETED PROJECT. DRAWINGS SHALL BE PREPARED AND SIGNED & SEALED BY A FLORIDA REGISTERED SURVEYOR.
7. IT IS THE CONTRACTORS RESPONSIBILITY TO CALL SUNSHINE ONE AT 811 FOR UTILITY LOCATES PRIOR TO CONSTRUCTION.
8. ALL DISTURBED AREAS SHALL BE RESTORED TO ORIGINAL CONDITION AND SODDED PER FDOT INDEX 570-010.

SYMBOL LEGEND

34.60 (EXISTING SPOT ELEVATION)
36 (EXISTING CONTOUR)
+12.50 (PROPOSED FINISHED GRADE)
(STORMWATER SURFACE FLOW)

McNEIL CARROLL
SCALE
1" = 10'

0 05 10 20

ROBERT L. CARROLL LICENSE No. 57988 STATE OF FLORIDA PROFESSIONAL ENGINEER

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PHONE: (850) 236-9832
Commission Number: 25871

CONSULTANTS:

PROJECT:

BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD
HOUSE
MOSLEY HIGH SCHOOL

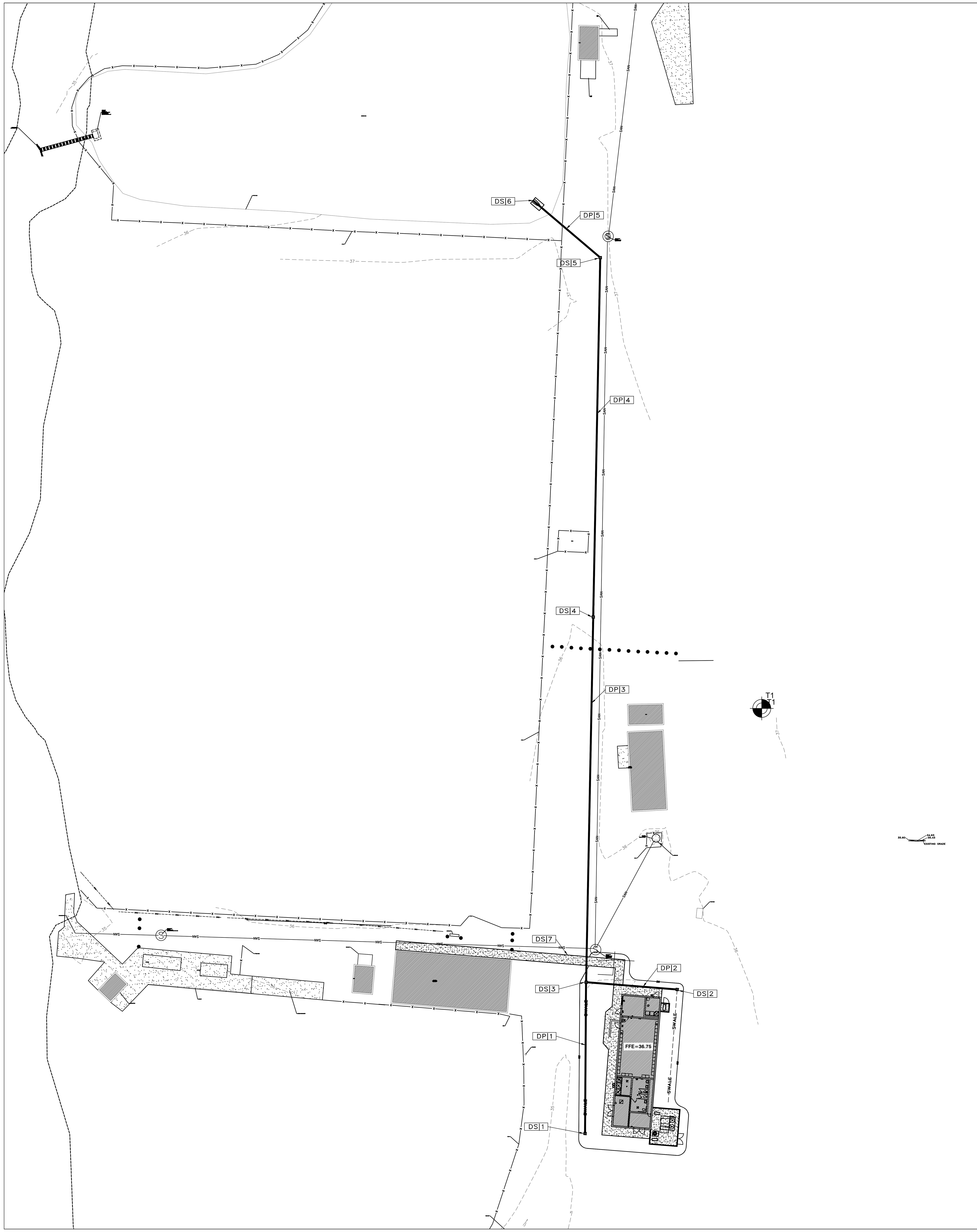
LYNN HAVEN, FLORIDA

SHEET TITLE:

SITE GRADING PLAN

SHEET NUMBER:

C1.4



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3. SEE SECTIONS IN CONSTRUCTION DETAILS.
4. ALL DRAINAGE PIPES TO BE DEDICATED TO THE CITY OF LYNN HAVEN SHALL BE VIDEO RECORDED AND PLACED ON A DIGITAL MEDIA (NO TAPES). VIDEO MUST BE REVIEWED AND APPROVED BY THE CITY.
5. PROPOSED FINISHED FLOORS SHALL BE 1 FOOT (MIN.) ABOVE ROADWAY CENTERLINE.
6. CONTRACTOR SHALL PROVIDE McNEIL CARROLL ENGINEERING, INC. FIVE (5) SETS AND ONE (1) DIGITAL COPY (AUTOCAD FORMAT) OF AS-BUILT DRAWINGS OF THE COMPLETED PROJECT. DRAWINGS SHALL BE PREPARED AND SIGNED & SEALED BY A FLORIDA REGISTERED SURVEYOR.
7. IT IS THE CONTRACTORS RESPONSIBILITY TO CALL SUNSHINE ONE AT 811 FOR UTILITY LOCATES PRIOR TO CONSTRUCTION.
8. ALL DISTURBED AREAS SHALL BE RESTORED TO ORIGINAL CONDITION AND SODDED PER FDOT INDEX 570-010.

SYMBOL LEGEND

·34.60	(EXISTING SPOT ELEVATION)
—36—	(EXISTING CONTOUR)
+12.50	(PROPOSED FINISHED GRADE)
⟹	(STORMWATER SURFACE FLOW)
CSW	(CONCRETE SIDEWALK– SEE CONSTRUCTION DETAILS)
DP 16	(SEE DRAINAGE PIPE SCHEDULE THIS SHEET ie.,#16)
DS 12	(SEE DRAINAGE STRUCTURE SCHEDULE THIS SHEET ie.,#12)
FC	(CHAINLINK FENCE – SEE CONSTRUCTION DETAILS)
N 1	(SEE NOTE ie.,#1 – SEE NOTES THIS SHEET)
SA	(SEE ARCHITECTURAL PLANS)

DRAINAGE PIPE SCHEDULE

NO.	SIZE	LF	TYPE	SLOPE
DP1	12"	83	ADS	0.50%
DP2	12"	50	ADS	0.80%
DP3	12"	202	ADS	0.50%
DP4	12"	199	ADS	0.50%
DP5	12"	49	ADS	0.50%

ALL ADS PIPE SHALL BE AS SHOWN OR EQUAL
ALL PERFORATED PIPE SHALL HAVE A GRAVEL PACK
ALL ADS PIPE SHALL BE RATED N-12
SEE CONSTRUCTION DETAILS.

DRAINAGE STRUCTURE SCHEDULE

NO.	TYPE STRUCTURE	TOP OF GRATE	PIPE INVERT	SLOT INVERT
DS1	15" ADS INLET	EL. 35.50	EL. 33.50	EL.
DS2	15" ADS INLET	EL. 35.50	EL. 33.50	EL.
DS3	15" ADS INLET	EL. 35.50	EL. 33.09	EL.
DS4	15" ADS INLET WITH SOLID LID	EL. 36.20	EL. 32.08	EL.
DS5	15" ADS INLET WITH SOLID LID	EL. 3735	EL. 31.08	EL.
DS6	15" CONCRETE MITERED END	EL.	EL. 30.83	EL.
DS7	5 LF TRENCH DRAIN EVERGRATE T-14C CONTINUUM	EL. N 35.65 S 35.63	EL.	EL.

SEE CONSTRUCTION DETAILS.
2' SUMP UNLESS OTHERWISE NOTED.

STORMWATER OPERATION AND MAINTENANCE SCHEDULE

AFTER EACH RAINFALL EVENT

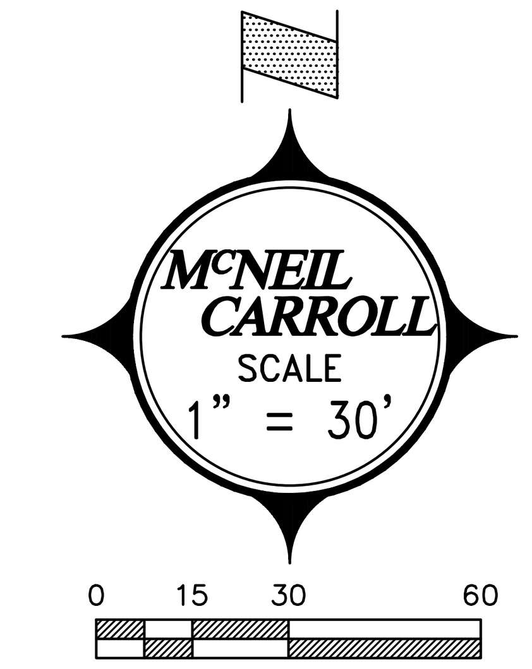
1. PAVEMENT AREAS: CLEAN/SWEEP DEBRIS AND DIRT FROM PAVEMENT AREAS.
2. SEDIMENTS IN RETENTION/DETENTION AREAS: REMOVE IMMEDIATELY.
3. DEBRIS IN RETENTION/DETENTION AREAS: ALL DEBRIS AND FOREIGN MATERIAL SHALL BE REMOVED IMMEDIATELY.
4. YARD INLETS, CATCH BASINS, ETC.: ALL DEBRIS AND FOREIGN MATERIALS SHALL BE REMOVED IMMEDIATELY.

PERIODIC POND/SYSTEM MAINTENANCE

1. CLEANING/ SWEEPING OF PAVEMENT AREAS SHALL BE ACCOMPLISHED WEEKLY OR AS REQUIRED.
2. INSPECT POND AT LEAST TWICE A MONTH FOR ACCUMULATION OF TRASH AND DEBRIS AND REMOVE UPON DISCOVERY. UPON ANY DISCOVERY OF TRASH AND DEBRIS ACCUMULATION, REMOVE AT ONCE.
3. MOWING AND LANDSCAPING MAINTENANCE SHOULD BE DONE ON A MONTHLY BASIS DURING THE ACTIVE GROWING SEASON FOR THIS AREA. INSPECT AND MAINTAIN AS REQUIRED DURING THE GROWING SEASON.
4. WEEDS OR UNDESIRABLE GROWTH SHALL BE REMOVED UPON DISCOVERY.
5. ALL STORMWATER STRUCTURES AND PIPES BASINS SHALL BE FLUSHED AS NECESSARY.
6. THE OWNER SHALL RE-GRADE AND RE-STABILIZE SWALE/ RETENTION/ DETENTION AREAS AS REQUIRED TO MAINTAIN THE APPROVED DESIGN, CROSS-SECTIONS, GRADES, ETC.
7. REMOVE SEDIMENT FROM POND WHEN ACCUMULATION REACHES TWO (2) INCHES. MEASUREMENTS FOR ACCUMULATION SHALL BE PERFORMED EVERY SIX MONTHS AND AFTER EACH MAJOR STORM EVENT.

INSPECTIONS

1. A MAINTENANCE INSPECTION SHALL BE PERFORMED AT MINIMUM EVERY FIFTH YEAR BY A REGISTERED PROFESSIONAL. THE INSPECTOR SHALL BE WORKING UNDER THE CHARGE OF A LICENSED PROFESSIONAL ENGINEER.
2. THE MAINTENANCE INSPECTION SHALL BE DOCUMENTED ON THE FDEP AND/OR NWFWMD STANDARD INSPECTION FORM 62-330-311(1).
3. THE INSPECTION SHALL BE SIGNED, SEALED AND DATED BY THE REGISTERED PROFESSIONAL AND SUBMITTED TO EITHER THE FDEP OR NWFWMD WITHIN 30 DAYS OF THE INSPECTION.
4. THE INSPECTION MUST BE CONDUCTED USING THE PLANS, CALCULATIONS AND SPECIFICATIONS APPROVED BY THE FDEP AND/OR NWFWMD.
5. THERE MUST BE AN INSPECTION COMPLETED BY A REGISTERED PROFESSIONAL ONE YEAR AFTER THE CONVERSION INTO OPERATION.



ROBERT L. CARROLL
LICENSE
No. 57988
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
2/6/2026

Robert L. Carroll, P.E.
PROFESSIONAL ENGINEER
FL LC # 57988

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REVISIONS

NO.	DESCRIPTION	DRAWN	CHECKED	DATE

PHASE	DRAWN	CHECKED	DATE
SCHEMATIC DESIGN	MAY	SLB	10/17/25
DESIGN DEVELOPMENT	MAY	SLB	1/9/26
CONSTRUCTION DOCUMENTS	MAY	SLB	4/3/26

JRA ARCHITECTS
2211 NAVY BLVD. , STE 100
PANAMA CITY BEACH, FL
PHONE: (850) 236-9832
Commission Number: 25871

CONSULTANTS:

PROJECT:

BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD
HOUSE
MOSLEY HIGH SCHOOL

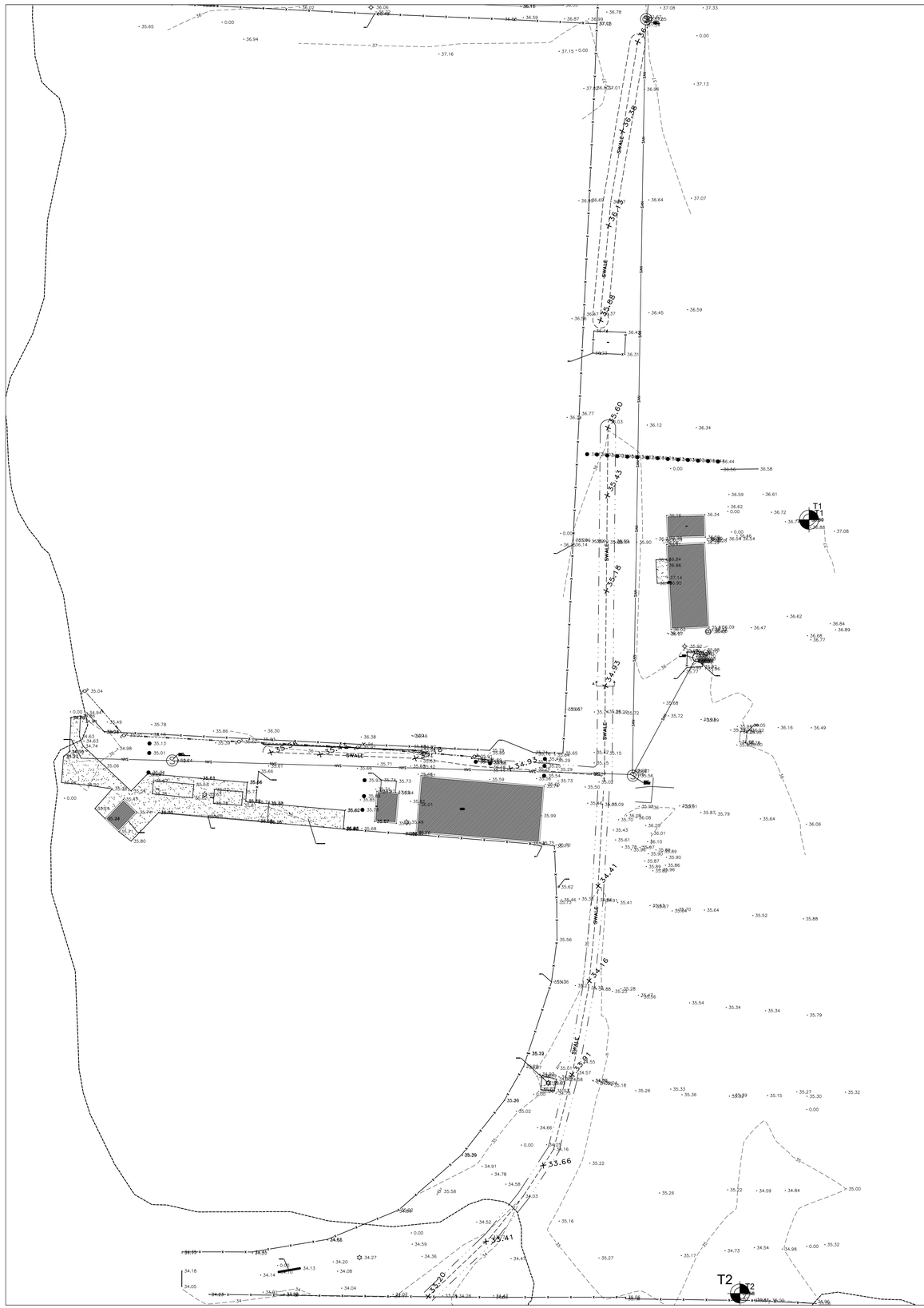
LYNN HAVEN, FLORIDA

SHEET TITLE:

SITE DRAINAGE PLAN

SHEET NUMBER:

C1.5



SITE GRADING AND DRAINAGE DRAWING NOTES:

1. SEE SYMBOL LEGEND ON THIS SHEET FOR SYMBOL INFORMATION AND REFERENCED DETAILS.
2. ALL DEMOLISHED MATERIALS (ie., SIGNS, CONCRETE, ASPHALT, ETC...) TO BE REMOVED AND DISPOSED OF IN A LEGAL MANNER. ALL EXISTING MONITORING WELLS ARE NOT TO BE REMOVED. WELLS IN PAVEMENT SHALL HAVE A MANHOLE LID INSTALLED.
3. SEE SECTIONS IN CONSTRUCTION DETAILS.
4. ALL DRAINAGE PIPES TO BE DEDICATED TO THE CITY OF LYNN HAVEN SHALL BE VIDEO RECORDED AND PLACED ON A DIGITAL MEDIA (NO TAPES). VIDEO MUST BE REVIEWED AND APPROVED BY THE CITY.
5. PROPOSED FINISHED FLOORS SHALL BE 1 FOOT (MIN.) ABOVE ROADWAY CENTERLINE.
6. CONTRACTOR SHALL PROVIDE McNEIL CARROLL ENGINEERING, INC. FIVE (5) SETS AND ONE (1) DIGITAL COPY (AUTOCAD FORMAT) OF AS-BUILT DRAWINGS OF THE COMPLETED PROJECT. DRAWINGS SHALL BE PREPARED AND SIGNED & SEALED BY A FLORIDA REGISTERED SURVEYOR.
7. IT IS THE CONTRACTORS RESPONSIBILITY TO CALL SUNSHINE ONE AT 811 FOR UTILITY LOCATES PRIOR TO CONSTRUCTION.
8. ALL DISTURBED AREAS SHALL BE RESTORED TO ORIGINAL CONDITION AND SODDED PER FDOT INDEX 570-010.

SYMBOL LEGEND

34.60 (EXISTING SPOT ELEVATION)
36 (EXISTING CONTOUR)
+12.50 (PROPOSED FINISHED GRADE)
(STORMWATER SURFACE FLOW)

McNEIL CARROLL
SCALE
1" = 30'

0 15 30 60

ROBERT L. CARROLL LICENSE No. 57988 STATE OF FLORIDA PROFESSIONAL ENGINEER

2/6/2026

Robert L. Carroll, P.E.
PROFESSIONAL ENGINEER
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JRA ARCHITECTS
2211 NAVY BLVD. , STE 100
PANAMA CITY BEACH, FL
PHONE: (850) 236-9832
Commission Number: 25871

CONSULTANTS:

PROJECT:

BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD HOUSE
MOSLEY HIGH SCHOOL

LYNN HAVEN, FLORIDA

SHEET TITLE:

ADDITIVE ALTERNATE GRADING PLAN

SHEET NUMBER:

C1.5A

SITE UTILITY DRAWING NOTES:

1. SEE SYMBOL LEGEND ON THIS SHEET FOR SYMBOL INFORMATION AND REFERENCED DETAILS.
2. SEE SECTION AND DETAILS IN CONSTRUCTION DETAILS.
3. ALL PROPOSED UTILITIES SHALL BE PLACED UNDERGROUND.
4. ALL SEWER LINES TO BE DEDICATED TO THE CITY OF LYNN HAVEN SHALL BE VIDEO RECORDED AND PLACED ON A DIGITAL MEDIA (NO TAPES). VIDEO MUST BE REVIEWED AND APPROVED BY THE CITY.
5. CONTRACTOR SHALL PROVIDE McNEIL CARROLL ENGINEERING, INC. FIVE (5) SETS AND ONE (1) DIGITAL COPY (AUTOCAD FORMAT) OF AS-BUILT DRAWINGS OF THE COMPLETED PROJECT. DRAWINGS SHALL BE PREPARED AND SIGNED & SEALED BY A FLORIDA REGISTERED SURVEYOR.
6. IT IS THE CONTRACTORS RESPONSIBILITY TO CALL SUNSHINE ONE AT 811 FOR UTILITY LOCATES PRIOR TO CONSTRUCTION.
7. ALL GRAVITY SEWER LINES SHALL BE VIDEO RECORDED AND PLACED ON DIGITAL MEDIA (NO TAPES). VIDEO MUST BE REVIEWED AND APPROVED BY McNEIL CARROLL ENGINEERING, INC.
8. ALL DISTURBED AREAS SHALL BE RESTORED TO ORIGINAL CONDITION AND SODDED PER FDOT INDEX 570-010.

SYMBOL LEGEND

- ⊠ CO (4" CLEANOUT – SEE CONSTRUCTION DETAILS)
- LAT (4" SEWER LATERAL – SEE CONSTRUCTION DETAILS)
- N|1 (SEE NOTE ie., #1 – SEE NOTES THIS SHEET)
- SA (SEE ARCHITECTURAL PLANS)
- SS|1|2 (SEE SEWER STRUCTURE SCHEDULE THIS SHEET ie.,#12 SEE CONSTRUCTION DETAILS)
- WF|1|4 (SEE WATER FIXTURE SCHEDULE THIS SHEET ie.,#14 SEE CONSTRUCTION DETAILS)
- WP|1|0 (SEE WATER PIPE SCHEDULE THIS SHEET ie.,#10 SEE CONSTRUCTION DETAILS)

WATER MAIN PIPE SCHEDULE			
NO.	SIZE	LF	TYPE
WP1	2"	2	PVC
WP2	2"	---	90° BEND
WP3	2"	23	PVC
WP4	2"	6	PVC

LESS THAN 4" WATER MAIN – ASTM D2241 SDR-21
4"-6" WATER MAIN – AWWA C900 DR18
8"-12" WATER MAIN – AWWA C900 DR25
ALL LINES SHALL BE THE COLOR BLUE.

WATER FIXTURE SCHEDULE	
NO.	TYPE
WF1	2" TAPPING SADDLE W/ VALVE ASSEMBLY

SEE CONSTRUCTION DETAILS.
ALL LINES SHALL BE THE COLOR BLUE

SEWER STRUCTURE SCHEDULE						
NO.	TYPE STRUCTURE	TOP OF M.H.	NORTH INVERT	SOUTH INVERT	NORTHEAST INVERT	WEST INVERT
SS1	EXISTING MANHOLE	EL. 35.93 EXIST	EL. 30.01	EL.	EL. 29.97	EL. 30.21

SEE CONSTRUCTION DETAILS.

BAY COUNTY UTILITY CONTACTS		
UTILITY	CONTACT	
TELEPHONE	SUNSHINE STATE ONE CALL	1-800-432-4770
ELECTRIC	GULF POWER	1-850-872-3212
CABLE TV	COMCAST WOW CABLE	1-850-769-0392 1-850-747-0139
WATER & SEWER	BAY COUNTY UTILITIES	1-850-872-4785
GAS	TECO PEOPLES GAS FLORIDA GAS TRANSMISSION	1-850-872-6140 1-800-432-4770

McNEIL CARROLL
SCALE
1" = 10'
0 05 10 20

ROBERT L. CARROLL LICENSE No. 57988 STATE OF FLORIDA PROFESSIONAL ENGINEER
2/6/2026

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CONSULTANTS:

PROJECT:
BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD
HOUSE
MOSLEY HIGH SCHOOL
LYNN HAVEN, FLORIDA

SHEET TITLE:
SITE UTILITY PLAN

SHEET NUMBER:
C1.6

GENERAL NOTES

PREVENTION, CONTROL AND ABATEMENT OF EROSION

ALL ON AND OFF SITE WORK INCLUDED CONSISTS OF BUT NOT LIMITED TO THE FOLLOWING:

I0.2643; SEEDING AND MULCHING OR SODDING FOR STABILIZATION.

CONSTRUCTION OF SEDIMENT BASINS, CHECK DAMS OR FLOATING BARRIERS.

PLACEMENT OF SILTATION FENCES DURING THE COURSE OF CONSTRUCTION.

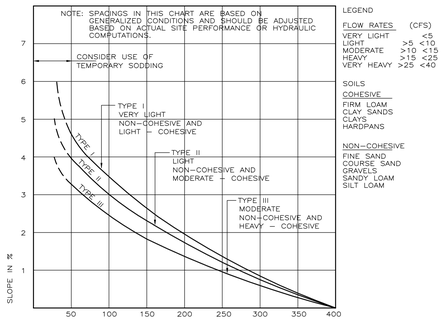
SILT FENCE TO BE INSTALLED AT PERIMETER OF SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES WILL BE UTILIZED THROUGHOUT THE CONSTRUCTION PHASE OF THIS PROJECT TO RESTRICT ANY TURBID RUNOFF FROM LEAVING THE CONSTRUCTION SITE.

CONTROL OF SEDIMENT–LADEN RUNOFF SHALL BE PROVIDED WITH HAY BALES AND/OR GEOTECH STYLE FABRICS. ALL CONTROL MEASURES SHALL BE PROPERLY LOCATED AND CONSTRUCTED TO PREVENT SEDIMENT TRANSPORT. THE MEANS FOR RETAINING THE SEDIMENTS WILL BE MAINTAINED BY THE CONTRACTOR UNTIL PERMANENT IMPROVEMENTS ARE COMPLETE.

THE CONTRACTOR IS RESPONSIBLE FOR TREATING ALL ONSITE STORM WATER DRAINAGE AS REQUIRED TO MEET THE CRITERIA OF 62–3 FLORIDA ADMINISTRATIVE CODE, F.A.C. PRIOR TO DISCHARGE.

ALL CATCH BASINS, INLETS AND ACCESSES TO UNDERGROUND STORM WATER SYSTEMS SHALL BE PROTECTED IN ACCORDANCE WITH THE ATTACHED DETAILS.

THE CONTRACTOR IS RESPONSIBLE FOR COMPLYING WITH THE TERMS AND CONDITIONS OF ANY STORM WATER PERMITS THAT MAY APPLY (FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION, FLORIDA DEPARTMENT OF TRANSPORTATION, BAY COUNTY, WATER MANAGEMENT DISTRICT, ETC.).



LEGEND

FLOW RATES (CFS)
VERY LIGHT <5
LIGHT >5 <10
MODERATE >10 <15
HEAVY >15 <25
VERY HEAVY >25 <40

SOILS
COHESIVE
FIRM LOAM
CLAY SANDS
CLAYS
HARDPANS

NON–COHESIVE
FINE SAND
COURSE SAND
GRAVELS
SANDY LOAM
SILT LOAM

TYPE I
VERY LIGHT
NON–COHESIVE AND
LIGHT – COHESIVE

TYPE II
LIGHT
NON–COHESIVE AND
MODERATE – COHESIVE

TYPE III
MODERATE
NON–COHESIVE AND
HEAVY – COHESIVE

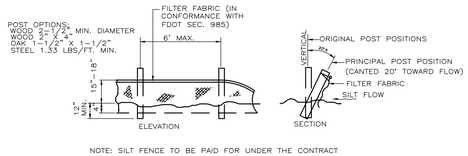
FDOT CHART I (BALED HAY OR STRAW BARRIERS AND SILT FENCES - INDEX 102

PARTIAL INLET

DITCH BOTTOM INLET

ANCHOR BALES WITH 2 – 2"x2"x4' STAKES PER BALE (SEE DETAIL)

PROTECTION AROUND INLETS OR SIMILAR STRUCTURES

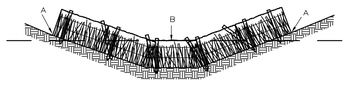


NOTE: SILT FENCE TO BE PAID FOR UNDER THE CONTRACT UNIT PRICE FOR THE STAKED SILT FENCE (LF.)

TYPE III SILT FENCE

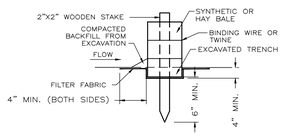
NOTE:

L0.315,T0.252;DO NOT DEPLOY SILT FENCES IN A MANNER THAT WILL ACT AS A DAM ACROSS PERMANENT FLOWING WATERCOURSES.
SILT FENCES ARE TO BE USED AT UPLAND LOCATIONS AND AS TURBIDITY BARRIERS USED AT PERMANENT BODIES OF WATER.
SILT FENCE SHOULD BE REPLACED EVERY SIX (6) MONTHS.
SILT FENCE SHALL EXTEND A MINIMUM OF 4" BELOW GRADE.
SILT FENCE SHALL EXTEND 4" AWAY FROM STAKES.



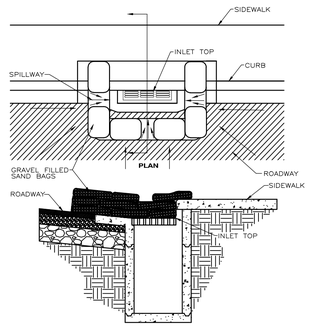
POINTS A SHOULD BE HIGHER THAN B

PROPER PLACEMENT OF BALES IN DRAINAGE WAY



EROSION CONTROL DETAILS

NOT TO SCALE



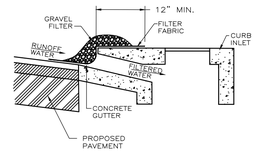
PLAN

SECTION

NOTE:
1. GRAVEL FILLED BAGS TO BE PLACED TIGHTLY TOGETHER AS TO NOT ALLOW ANY GAPS IN BETWEEN THE BAGS VERTICALLY AND HORIZONTALLY.
2. IF SEDIMENT IS PENETRATING BAGS, BAGS MAY NEED TO BE WRAPPED IN ADDITIONAL FILTER FABRIC.

CURB INLET SEDIMENT BARRIER (OPTION 1)

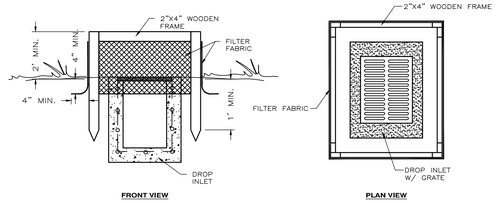
NOT TO SCALE



NOTES:
1. GRAVEL SHALL BE #57 STONE.

GRAVEL CURB INLET SEDIMENT FILTER (OPTION 2)

NOT TO SCALE



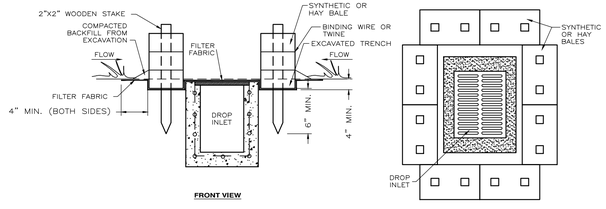
FRONT VIEW

PLAN VIEW

NOTES:
1. TOP FRAME REQUIRED FOR STABILITY.
2. THIS METHOD OF INLET PROTECTION IS NOT TO BE USED IN RIGHT–OF–WAYS OR MEDIANS.
3. STAKES SHALL EXTEND 1' MIN. BELOW GRADE.
4. FILTER FABRIC SHALL EXTEND 4" MIN. BELOW GRADE AND 4" MIN. BEYOND WOODEN FRAME.

DROP INLET SEDIMENT FILTER OPTION 1

NOT TO SCALE



FRONT VIEW

PLAN VIEW

DROP INLET SEDIMENT FILTER OPTION 2

NOT TO SCALE

WORK IN RIGHTS-OF-WAYS

ALL WORK WITHIN RIGHTS–OF–WAYS SHALL BE IN STRICT ACCORDANCE WITH THE APPLICABLE REQUIREMENTS OF THE RESPECTIVE AGENCIES.

THE CONTRACTOR SHALL COOPERATE WITH THE GOVERNING STATE AND LOCAL AGENCIES IN ALL PROCEDURES, MATERIALS AND METHODS OF CONSTRUCTION.

ALL OFF–SITE WORK INCLUDED CONSISTS OF BUT IS NOT LIMITED TO THE FOLLOWING:

CONSTRUCTION OF DRIVEWAY CONNECTIONS TO EXISTING MUNICIPAL ROADWAYS AS SHOWN ON PLANS.

PLACEMENT OF ABOVE OR BELOW GROUND UTILITIES AND CONNECTION TO EXISTING UTILITIES AS SHOWN ON PLANS.

SITE CLEARING AND DEMOLITION

ANY WORK WITHIN STREET OR HIGHWAY RIGHT–OF–WAY SHALL BE DONE IN ACCORDANCE WITH THE REQUIREMENTS OF THE GOVERNMENTAL AGENCIES HAVING JURISDICTION AND SHALL NOT BEGIN UNTIL THESE GOVERNING AUTHORITIES HAVE BEEN NOTIFIED AND PROPER PERMITS OBTAINED.

KEEP ALL AREAS WITHIN THE CONSTRUCTION AREA SUFFICIENTLY DAMPENED TO PREVENT DUST FROM RISING DUE TO CONSTRUCTION. COMPLY WITH ALL ANTI–POLLUTION ORDINANCES.

THIS SUBCONTRACTOR SHALL SEE TO IT THAT TRUCKS LEAVING THE SITE SHALL DO SO IN SUCH A MANNER THAT MUD AND EARTH WILL NOT BE DEPOSITED ON ADJACENT STREET PAVEMENTS. ANY MUD OR EARTH DEPOSITED ON STREET PAVEMENTS SHALL BE PROMPTLY REMOVED BY THIS SUBCONTRACTOR.

ALL CLEARING SHALL BE PERFORMED IN A MANNER SUCH AS TO PREVENT ANY WASH–OFF OF SOILS FROM THE SITE INTO STREAMS AND/OR STORM DRAINAGE SYSTEMS. APPROPRIATE SEDIMENTATION PONDS, DIKES, COLLARS, AND FILTER MEDIA SHALL BE EMPLOYED TO INSURE COMPLIANCE WITH THESE REQUIREMENTS. WHERE A SPECIFIC STATUTE GOVERNS THESE PROCEDURES, SUCH STATUTE SHALL BE COMPLIED WITH IN IT'S ENTIRETY.

TOPSOIL IS DEFINED AS FRIABLE CLAY LOAM SURFACE SOIL FOUND IN A DEPTH OF NOT LESS THAN 4". SATISFACTORY TOPSOIL IS REASONABLY FREE OF SUBSOIL, CLAY LUMPS, STONES, AND OTHER OBJECTS OVER 2" IN DIAMETER, AND WITHOUT WEEDS, ROOTS, AND OTHER OBJECTIONABLE MATERIAL.

STRIP TOPSOIL TO WHATEVER DEPTHS ENCOUNTERED IN A MANNER TO PREVENT INTERMINGLING WITH UNDERLYING SUBSOIL OTHER OBJECTIONABLE MATERIAL.

REMOVE HEAVY GROWTHS OF GRASS FROM AREAS BEFORE STRIPPING. WHERE TREES ARE INDICATED TO BE LEFT STANDING, STOP TOPSOIL STRIPING A SUFFICIENT DISTANCE TO PREVENT DAMAGE TO MAIN ROOT SYSTEM. DISPOSE OF UNSUITABLE OR EXCESS TOPSOIL SAME AS WASTE MATERIAL, HEREIN SPECIFIED.

BURNING WILL NOT BE PERMITTED ON PROJECT SITE.

ALL EXISTING STRUCTURES, UTILITIES AND OTHER OBSTACLES IN CONFLICT WITH THE PROPOSED FACILITY SHALL BE REMOVED AND DISPOSED OF IN A LEGAL MANNER. SEE OTHER UTILITY AND MISCELLANEOUS NOTES CONCERNING REMOVAL.

ALLOW TESTING SERVICES TO INSPECT AND APPROVE SUBGRADE AND FILL LAYERS BEFORE FURTHER CONSTRUCTION WORK IS PERFORMED.

ATTENTION IS CALLED TO THE FACT THAT THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING ALL UTILITY COMPANIES TO OBTAIN LOCATIONS OF ALL EXISTING UTILITIES OR OBSTRUCTIONS WHICH HE MAY ENCOUNTER DURING CONSTRUCTION.

AFTER LOCATION OF UTILITIES BY THE APPROPRIATE UTILITY COMPANY, IT IS THE CONTRACTOR'S LIABILITY TO PROTECT ALL SUCH UTILITY LINES, INCLUDING SERVICE LINES AND APPURTENANCES, AND TO REPLACE AT HIS OWN EXPENSE ANY WHICH MAY BE DAMAGED BY THE CONTRACTOR'S EQUIPMENT OR FORCES DURING CONSTRUCTION.

TO PROTECT PERSON FROM INJURY AND TO AVOID PROPERTY DAMAGE, ADEQUATE BARRICADES, CONSTRUCTION SIGNS, TORCHES, RED LANTERNS AND GUARDS AS REQUIRED SHALL BE PLACED AND MAINTAINED DURING THE PROGRESS OF THE CONSTRUCTION WORK.

ADEQUATE PROVISION SHALL BE MADE FOR THE FLOW OF SEWERS, DRAINS, AND WATER COURSES ENCOUNTERED DURING CONSTRUCTION, AND THE STRUCTURES WHICH MAY HAVE BEEN DISTURBED SHALL BE SATISFACTORILY RESTORED BY THE CONTRACTOR.

EXCAVATING, FILLING AND GRADING

ALL ON AND OFF–SITE WORK INCLUDED CONSISTS OF BUT IS NOT LIMITED TO THE FOLLOWING:

ALL ON AND OFF–SITE PREPARATION WORK FOR EXCAVATION, PIPE BED PREPARATION AND BACKFILL FOR UNDERGROUND UTILITIES.

COMPACTION OF BACKFILL.

REMOVAL OF ALL EXCESS OR UNUSABLE MATERIAL.

APPROVAL REQUIRED: ALL FILL MATERIAL SHALL BE SUBJECT TO APPROVAL OF THE GEO–TECHNICAL ENGINEER.

ALL ON–SITE FILL MATERIAL SHALL BE SOIL–ROCK MIXTURE WHICH IS FREE FROM ORGANIC MATTER (LESS THAN 3% BY IGNITION), AND OTHER DELETERIOUS SUBSTANCE. IT SHALL CONTAIN NO ROCKS OR LUMPS OVER SIX (6) INCHES IN GREATEST DIMENSION AND NOT MORE THAN 15% OF THE ROCKS OR LUMPS BY DRY WEIGHT, SHALL BE LARGER THAN 2 AND 1/2 INCHES IN GREATEST DIMENSION.

ALL IMPORTED FILL MATERIAL SHALL MEET THE REQUIREMENTS OF ON–SITE FILL MATERIAL AND SHALL IN ADDITION, BE PREDOMINANTLY GRANULAR WITH A MAXIMUM PARTICLE SIZE OF TWO (2) INCHES AND A PLASTICITY INDEX OF 12 OR LESS.

ALL ON–SITE FILL MATERIAL USED FOR TRENCH AND STRUCTURAL BACKFILL SHALL MEET THE REQUIREMENTS OF ARTICLE ABOVE.

ALL IMPORTED COHESIONLESS MATERIAL USED FOR TRENCH AND STRUCTURAL BACKFILL SHALL BE FREE FROM ORGANIC SUBSTANCE (LESS THAN 3% BY IGNITION) AND OTHER DELETERIOUS MATTER, SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER.

PRIOR TO ALL WORK OF THIS SECTION, CONTRACTOR IS TO BECOME THOROUGHLY FAMILIAR WITH THE SITE, THE SITE CONDITIONS, AND ALL PORTIONS OF THE WORK, FALLING WITHIN THIS SECTION.

DO NOT ALLOW OR CAUSE ANY OF THE WORK PERFORMED OR INSTALLED TO BE COVERED UP OR ENCLOSED BY WORK OF THIS SECTION PRIOR TO ALL REQUIRED INSPECTIONS, TESTS AND APPROVALS.

AFTER THE WORK HAS BEEN COMPLETELY TESTED, INSPECTED AND APPROVED, MAKE ALL REPAIRS AND REPLACEMENTS NECESSARY TO RESTORE THE WORK TO THE CONDITION IN WHICH IT WAS FOUND AT THE TIME OF UNCOVERING, ALL AT NO ADDITIONAL COST TO THE OWNER.

FOR SETTING AND ESTABLISHING FINISH ELEVATIONS AND LINES, SECURE THE SERVICES OF A REGISTERED CIVIL ENGINEER OR LAND SURVEYOR ACCEPTABLE TO THE OWNER, CAREFULLY PRESERVE ALL DATA AND ALL MONUMENTS SET BY THE CIVIL ENGINEER OR LAND SURVEYOR, AND IF DISPLACED OR LOST, IMMEDIATELY REPLACE TO THE APPROVAL OF THE OWNER AND AT NO ADDITIONAL COST TO THE OWNER.

PROVIDE ALL LABOR, MATERIALS, EQUIPMENT, SERVICES, ETC. NECESSARY AND INCIDENTAL TO THE COMPLETION OF ALL EARTHWORK AS SHOWN ON THE DRAWINGS AND SPECIFICATIONS. ALL OFF–SITE WORK INCLUDED CONSISTS OF BUT IS NOT LIMITED TO THE FOLLOWING: THIS WORK CONSISTS OF GRADING IN ORDER TO ACHIEVE FINISHED ELEVATIONS SHOWN ON THE CONSTRUCTION PLANS.

ALL GRADED SURFACES SHALL BE SMOOTH AND UNIFORM, WITHOUT ABRUPT CHANGES IN SLOPE OR GRADE, AREAS TO BE COVERED WITH PAVING SHALL BE FINE GRADED TO THE REQUIRED ELEVATIONS AND SLOPES. FINISHED SURFACES IN ALL OTHER AREAS MAY VARY UP TO 0.1 FEET FROM THE REQUIRED ELEVATIONS.

PERFORM EXCAVATION WORK IN COMPLIANCE WITH APPLICABLE REQUIREMENTS OF GOVERNING AUTHORITIES HAVING JURISDICTION. ALL MATERIALS AND CONSTRUCTION METHODS SHALL BE IN ACCORDANCE WITH SECTION 120 OF THE STANDARD SPECIFICATIONS FOR ROADS AND BRIDGES, STATE OF FLORIDA, DEPARTMENT OF TRANSPORTATION, LATEST EDITION.

EMPLOY, AT CONTRACTOR'S EXPENSE, DESIGN LABORATORY TO PERFORM SOIL TESTING AND INSPECTION SERVICE FOR QUALITY CONTROL TESTING DURING EARTHWORK OPERATIONS. SUBMIT FOLLOWING REPORTS DIRECTLY TO ENGINEER AND COPIES TO CITY ENGINEERING DEPARTMENT FROM THE TESTING SERVICES, WITH COPY TO THE CONTRACTOR.

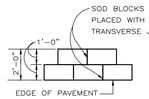
TEST REPORTS ON BORROW MATERIAL.

FIELD DENSITY TEST REPORTS.

ONE OPTIMUM MOISTURE–MAXIMUM DENSITY CURVE FOR EACH TYPE OF SOIL ENCOUNTERED.

WHERE REQUIRED, THE SITE SHALL BE EXCAVATED TO THE GRADES COURSE, EXCAVATED MATERIAL THAT IS SUITABLE SHALL BE USED IN THE FILL SECTIONS OF THE SITE. NO SUITABLE MATERIAL SHALL BE REMOVED FROM THE SITE. ANY EXCESS SUITABLE MATERIAL SHALL BE PLACED AT THE DIRECTION OF THE ENGINEER.

EXCAVATION FOR MANHOLES, CATCH BASINS, AND OTHER ACCESSORIES SHALL BE SUFFICIENT TO LEAVE AT LEAST 12 INCHES IN THE CLEAR BETWEEN THEIR OUTER SURFACES AND THE EMBANKMENT OF TIMBER THAT MAY BE USED TO PROTECT THEM.



PLAN VIEW

THIS WORK CONSISTS OF SODDING AREAS CLEARED DURING CONSTRUCTION AND NOT PAVED, OR AS OTHERWISE SHOWN ON THE CONSTRUCTION PLANS. ALL MATERIAL AND CONSTRUCTION METHODS SHALL BE IN ACCORDANCE WITH SECTION 570, 571, 573, OR 575 OF THE FLORIDA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS, LATEST REVISION.

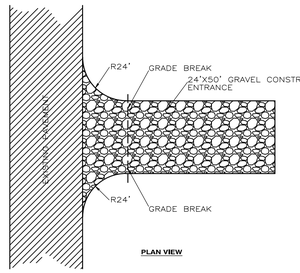
USE ST. AUGUSTINE GRASS (FLORATAM) SOD. THE SOD SHALL BE LIVE, FRESH AND UNINJURED AT THE TIME OF PLANTING AND SHALL HAVE A THICK MAT OF ROOTS WITH ENOUGH ADHERING SOIL TO ASSURE GROWTH. APPLY SOD WITHIN 72 HOURS OF CUTTING OR STACKING TO KEEP MOIST.

PREPARE THE GROUND BY LOOSENING THE SOIL. PLACE SOD ON THE PREPARED SOIL WITH EDGES IN CLOSE CONTACT. STAGGER THE SOD PIECES SO AS TO AVOID A CONTINUOUS DOWNHILL SEAM. TAMP THE OUTER EDGES OF THE SODDED AREA TO PRODUCE A SMOOTH CONTOUR.

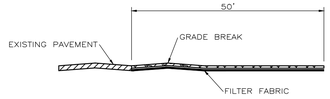
KEEP SOD CONTINUOUSLY MOIST TO A DEPTH BELOW THE ROOT ZONE FOR THREE WEEKS AFTER PLACEMENT.

SODDING DETAIL

NOT TO SCALE



PLAN VIEW



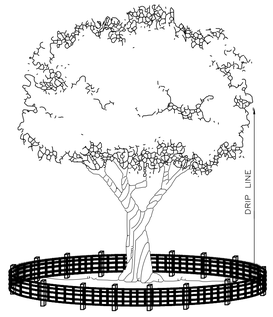
PROFILE VIEW

NOTE:

1. GRAVEL CONSTRUCTION ENTRANCE SHALL BE 24' WIDE AND 50' LONG.
2. CONSTRUCTION ENTRANCE SHALL BE 6" OF #57 STONE OVERLAYING FILTER FABRIC.
3. ALGEBRAIC DIFFERENCE OF SLOPE FROM EXISTING ROAD AND SLOPE FROM EDGE OF PAVEMENT TO GRADE BREAK SHALL NOT EXCEED 12%.
4. CONSTRUCTION ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHT–OF–WAYS.
5. WHEN NECESSARY, WHEELS SHALL BE CLEANED PRIOR TO ENTRANCE ONTO PUBLIC RIGHT–OF–WAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE THAT DRAINS INTO AN APPROVED SEDIMENT TRAP OR BASIN.

CONSTRUCTION ENTRANCE DETAIL

NOT TO SCALE



CORD FENCE – POSTS WITH A MINIMUM SIZE 2 INCHES SQUARE OR 2 INCHES IN DIAMETER SET SECURELY IN GROUND AND PROTRUDING AT LEAST 4 FEET ABOVE THE GROUND SHALL BE PLACED AT THE LIMITS OF CLEARING WITH ORANGE CONSTRUCTION MESH FENCING RUNNING BETWEEN POSTS.

TREE BARRIER DETAIL

NOT TO SCALE

2/6/2026

Robert L. Carroll, P.E.
PROFESSIONAL ENGINEER
FL LC # 57988

THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY ROBERT L. CARROLL, PE. ON DATE USING A DIGITAL SIGNATURE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED WITHOUT THE DIGITAL COPY. THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES IN ACCORDANCE WITH RULE 61G15–23.004 F.A.C.

REVISIONS

NO.	DESCRIPTION	DRAWN	CHECKED	DATE

PHASE	DRAWN	CHECKED	DATE
SCHEMATIC DESIGN	MAY	SLB	10/17/25
DESIGN DEVELOPMENT	MAY	SLB	1/9/26
CONSTRUCTION DOCUMENTS	MAY	SLB	4/3/26

AJRA ARCHITECTS
2211 NAVY BLVD. , STE 100
PANAMA CITY BEACH, FL
PHONE: (850) 236–9832
Commission Number: 25871

CONSULTANTS:

PROJECT:

BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD
HOUSE
MOSLEY HIGH SCHOOL

LYNN HAVEN, FLORIDA

SHEET TITLE:

CONSTRUCTION DETAILS

SHEET NUMBER:

C2.1

SITE DRAINAGE

ALL OFF–SITE AND ON–SITE WORK INCLUDED CONSISTS OF BUT IS NOT LIMITED TO THE FOLLOWING:

EXCAVATION, BEDDING, FILTER MATERIAL AND BACKFILL FOR ALL STORM SEWER, SUBSURFACE DRAINS AND DRAINAGE STRUCTURES.

COMPLETE INSTALLATION OF ALL STORM SEWER, SUBSURFACE DRAINS, CATCH BASINS, JUNCTION BOXES, MANHOLES, ETC., INCLUDING ALL RELATED FITTINGS, JOINTS COVERS, GRATES, FRAMES, RUNGS, ETC.

ANY WORK WITHIN STREET OR HIGHWAY RIGHT–OF–WAY SHALL BE DONE IN ACCORDANCE WITH THE REQUIREMENTS OF THE GOVERNMENTAL AGENCIES HAVING JURISDICTION AND SHALL NOT BEGIN UNTIL ALL OF THESE GOVERNING AUTHORITIES HAVE BEEN NOTIFIED.

POLYVINYL CHLORIDE (PVC), FOR PIPE UP TO AND INCLUDING TEN INCHES (10") IN DIAMETER, SHALL CONFORM TO ASTM D3034 SDR 35 WITH ELASTOMERIC GASKET JOINTS CONFORMING TO ASTM D3212.

REINFORCED CONCRETE PIPE, FOR PIPE TWELVE INCHES (12") IN DIAMETER AND UP, SHALL CONFORM TO ASTM C–76, CLASS IV OR AASHTO M–170 WITH BELL AND SPIGOT OR TONGUE AND GROOVE COMPRESSION JOINT CONFORMING TO ASTM C–443.

MANHOLES, CATCH BASINS, ETC. SHALL BE SIZE AND TYPE INDICATED ON THE DRAWINGS AND SHALL BE CONSTRUCTED OF THE FOLLOWING:

REINFORCED PRECAST CONCRETE MANHOLE SECTIONS INCLUDING CONCENTRIC OR ECCENTRIC CONES AND GRADE RINGS SHALL BE 4000 PSI CONCRETE AND CONFORM TO ASTM C478 OR AASHTO M–199. SECTIONS SHALL BE COMPLETE WITH 3/4" ROUND CAST IN PLACE WROUGHT IRON STEPS.

BRICK SHALL BE SOUND, HARD BURNED THROUGHOUT AND OF UNIFORM SIZE AND QUALITY AND SHALL BE IN ACCORDANCE WITH ASTM C–32, GRADE MS OR MM.

CONCRETE MASONRY SHALL BE SOLID PRECAST SEGMENTAL CONCRETE MASONRY UNITS CONFORMING TO ASTM C–139.

IRON CASTINGS SHALL CONFORM TO ASTM A–48, CLASS 30. BEARING SURFACES BETWEEN CAST IRON FRAMES, COVERS AND GRATES SHALL BE MACHINED, FITTED TOGETHER AND MATCH MARKED TO PREVENT ROCKING.

SYSTEM IDENTIFYING LETTER 2" HIGH SHALL BE STAMPED OR CAST INTO ALL COVERS SO THAT THE MAY BE PLAINLY VISIBLE.

CASTINGS SHALL BE MANUFACTURED BY EAST JORDAN IRON WORKS, INC. NEENAH FOUNDRY COMPANY, VULCAN FOUNDRY COMPANY OR EQUAL.

MANHOLE STEPS FOR BRICK OR CONCRETE MASONRY STRUCTURES SHALL BE CAST IRON ASPHALT COATED, NEENAH FOUNDRY COMPANY "R–1980–E" OR EQUAL.

CONCRETE AND MASONRY MATERIALS FOR CONSTRUCTION OF STORM DRAINAGE STRUCTURES SHALL CONSIST OF THE FOLLOWING:

PORTLAND CEMENT SHALL BE STANDARD BRAND OF PORTLAND CEMENT CONFORMING TO ASTM C–150, TYPE I OR II.

FINE AND COARSE AGGREGATES FOR CONCRETE SHALL BE PER ASTM C–33. AGGREGATES SHALL BE WELL GRADED FROM FINE TO COARSE WITHIN LIMITS SPECIFIED IN ASTM C–33. MAXIMUM SIZE OF COARSE AGGREGATE SHALL BE 3/4".

AGGREGATE FOR CEMENT MORTAR SHALL BE CLEAN, SHARP SAND CONFORMING TO ASTM C–144. GRADE SAND FROM COARSE TO FINE WITH 100% PASSING NO. 8 SIEVE, AND NOT OVER 10 TO 30% PASSING NO. 50 SIEVE. HYDRATED LIME SHALL COMPLY WITH ASTM C–207, TYPE S. WATER SHALL BE CLEAN AND FREE FROM DELETERIOUS MATERIALS.

ALL MATERIAL USED FOR CONCRETE AND THE DESIGN OF ALL CONCRETE MIXES SHALL CONFORM WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE (ACI 211.1–81).

ALL CONCRETE, UNLESS NOTED OTHERWISE, SHALL DEVELOP A 28–DAY COMPRESSIVE STRENGTH OF 3000 PSI.

JOINT SEALANT SHALL BE HOT LAID BITUMINOUS SEALER.

RIP RAP SHALL BE SOUND, TOUGH DURABLE ROCK OR BROKEN CONCRETE AS APPROVED BY THE GEOTECHNICAL ENGINEER. RIP RAP SHALL BE AT LEAST EIGHT INCH (8") IN ONE DIMENSION AN SHALL HAVE A VOLUME OF NOT LESS THAN J1/3 CUBIC FOOT. SMALLER PIECES PERMITTED FOR FILLING VOIDS.

REINFORCING STEEL FOR CONCRETE SHALL BE INTERMEDIATE GRADE NEW BILLET STEEL CONFORMING TO ASTM A–615, GRADE 40. WELDED WIRE MESH SHALL CONFORM TO ASTM DESIGNATION A185 FOR SMOOTH WIRE AND ASTM A497 FOR DEFORMED WIRE.

FORMS FOR FOUNDATIONS AND OTHER CONCRETE WORK SHALL BE WOOD. FORMS SHALL BE OF SUFFICIENT STRENGTH TO PREVENT DEFORMATION UNDER LOAD AND TIGHT ENOUGH TO PREVENT LEAKAGE. FOUNDATIONS MAY BE POURED AGAINST EARTH WHERE CONDITIONS PERMIT.

ALL REINFORCEMENT SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH ACI 318–77. WELDED WIRE MESH SHALL BE LAPPED 6–INCHES AT ALL EDGES.

THE MIXING, PLACING, CURING AND FINISHING OF CONCRETE SHALL COMPLY WITH ACI 304 AND ACI 318. ALL EXPOSED SURFACES SHALL BE GIVEN A HARD STEEL TROWEL FINISH WITH NO TROWEL MARKS REMAINING. NO CEMENT SHALL BE DUSTED ON THE SURFACE. ALL CONCRETE SHALL BE CURED BY COATING WITH A CLEAR CURING NO CEMENT CONFORMING TO ASTM C–304, OR BY KEEPING IT WET FOR AT LEAST SIX DAYS AFTER POURING. AFTER THE FORMS ARE STRIPPED, ALL EXPOSED CONCRETE SURFACES SHALL BE POINTED AS NEEDED AND RUBBED TO A UNIFORM FINISH.

CONCRETE, UNLESS OTHERWISE NOTED, SHALL HAVE COMPRESSIVE STRENGTH AFTER 28 DAYS OF 3000 PSI MINIMUM. MIX SHALL BE SO PROPORTIONED TO PROVIDE A MINIMUM OF 517 POUNDS OF CEMENT PER CUBIC YARD.

CONCRETE FILL BELOW GRADE FOR PIPE CRADLES ETC. MAY BE 2500 PSI AT 28 DAYS.

CONCRETE, WHERE EXPOSED TO THE WEATHER, SHALL BE AIR ENTRAINED. AIR ENTRAINMENT SHALL BE ACCOMPLISHED BY THE USE OF ADDITIVES CONFORMING TO ASTM C–260. AIR CONTENT SHALL BE 6% + 1%. ADDITIVE SHALL BE USED STRICTLY IN ACCORDANCE WITH MANUFACTURER'S PRINTED DIRECTIONS.

READY–MIX CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF ASTM C–94.

CEMENT MORTAR SHALL BE AS SPECIFIED HEREINAFTER. USE METHODS OF MIXING MORTAR MATERIALS CAN BE CONTROLLED AND ACCURATELY MAINTAINED DURING WORK PROGRESS. MORTAR SHALL NOT BE MIXED IN GREATER QUANTITIES THAN SATISFACTORY WORKABILITY. RETEMPERING OF MORTAR IS NOT PERMITTED.

MORTAR FOR LAYING BRICK OR CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C–270, TYPE M, AVERAGE COMPRESSIVE STRENGTH 2500 AT 28 DAYS. MORTAR MIX SHALL BE PROPORTIONED BY VOLUME. MORTAR FOR PARGING SHALL CONSISTS OF ONE PART PORTLAND CEMENT AND TWO PARTS SAND.

MORTAR FOR GROUTING OF RIP RAP SHALL CONSIST OF ONE PART PORTLAND CEMENT AND THREE PARTS SAND.

STORM WATER SEWERS: STORM SEWERS SHALL BE INSTALLED IN LOCATIONS AND OF SIZES INDICATED ON DRAWING.

LAY PIPE, EMBED IT FIRMLY TO REQUIRED LINE AND GRADE WITH BELLS OF GROOVE END UP–GRADE. FIT ENDS TOGETHER, EXCAVATE BELL HOLES SO THAT SEWER WILL HAVE SMOOTH AND UNIFORM INVERT THROUGHOUT ITS LENGTH.

CORRUGATED METAL PIPE SHALL BE PLACED ON A FLAT BOTTOM TRENCH WITH HAUNCHES SOLIDLY SUPPORTED BY TAMPED BEDDING MATERIAL.

WHERE GROUND IS FOUND UNSUITABLE TO SUPPORT PIPE, PROVIDE CONCRETE CRADLES. DEPOSIT CONCRETE FULL WIDTH OF TRENCH 4" DEEP MINIMUM TO BOTTOM OF PIPE, REINFORCE CONTINUOUSLY WITH TWO (2) NO. 4 REINFORCING BARS. BEFORE CONCRETE IS SET, EMBED PIPE EVENLY, DEPOSIT REMAINDER OF CONCRETE TO CENTERLINE OF PIPE AND TAMP IN A MANNER TO AVOID DISTURBING PIPE.

WHERE STORM SEWER CROSSES A SANITARY SEWER OR WATER LINE AND THE STORM SEWER IS WITHIN ONE AND A HALF (1–1/2) FEET OF THE SANITARY SEWER PIPE OR WATER LINE, THE INTERSECTION OF THE PIPE OR LINE SHALL BE EMBEDDED IN CONCRETE FOR A DISTANCE OF FIVE FEET (5') EACH WAY FROM CENTERLINE OF INTERSECTION.

PROVIDE POURED CONCRETE FOUNDATIONS FOR DRAINAGE STRUCTURES.

PRECAST CONCRETE BASE MAY BE USED WHERE APPROVED BY THE GEO–TECHNICAL ENGINEER. PRECAST CONCRETE BASE MUST BE SET LEVEL ON SAND CUSHION OF NOT LESS THAN 2" NOR MORE THAN 4".

MANHOLES AND CATCH BASINS SHALL BE CONSTRUCTED OF BRICK, CONCRETE MASONRY OR PRECAST CONCRETE WITH CAST IRON FRAMES, COVERS AND MANHOLE STEPS, AS INDICATED ON DRAWINGS AND SPECIFIED HEREIN.

RIP RAP SHALL BE LAID OVER FILTER FABRIC FROM THE BOTTOM UPWARD, STONES SHALL BE LAID BY HAND WITH EIGHT (8") INCH MINIMUM DIMENSION PERPENDICULAR TO GRADE WITH WELL BROKEN JOINTS, COMPACTED AS IT GOES, TRUE TO LINE. ALL JOINTS SHALL BE FILLED WITH CEMENT MORTAR SURFACE OF STONE TO BE EXPOSED. CLEAN JOINTS WITH SIRE BRUSH.

BEFORE BACKFILLING AROUND DRAINAGE STRUCTURES, ALL FORMS, TRASH AND DEBRIS SHALL BE REMOVED AND CLEARED AWAY. SELECTED EXCAVATED MATERIAL SHALL BE PLACED SYMMETRICALLY ON ALL SIDES IN EIGHT INCH (8") MAXIMUM LAYERS; EACH LAYER SHALL BE MOISTENED AND COMPACTED WITH MECHANICAL OR HAND TAMPERS.

INFILTRATION OF THE STORM DRAINAGE SYSTEM SHALL NOT EXCEED 0.60 GALLONS PER INCH OF INTERNAL PIPE DIAMETER PER ONE HUNDRED FEET (100') OF PIPELINE PER HOUR WITH A MAXIMUM HYDROSTATIC HEAD AT THE CENTER LINE OF THE PIPE OF TWENTY FIVE FEET (25'), OR AS REQUIRED BY GOVERNING CODE AUTHORITIES.

CATCH BASIN FRAMES AND GRATINGS: ASPHALT COATED GRAY CAST IRON, ANSI/ASTM A 48, CLASS 30 B.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO SUPPLY ALL MATERIALS NECESSARY TO COMPLETE DRAINAGE.

SEEDING RATE ZONES

SEEDING RATES (LBS/AC)								
TYPE OF SEED	ZONE I				ZONE II			
	COASTAL		INLAND		COASTAL		INLAND	
	MAR. TO NOV.	NOV. TO MAR.	MAR. TO OCT.	OCT. TO MAR.	FEB. TO DEC.	DEC. TO FEB.	FEB. TO DEC.	DEC. TO FEB.
PERMANENT GRASS								
UNHULLED BERMUDA	15	15	10	15	15	15	10	15
BAHIA ARGENTINA OR PENSACOLA			30	30			30	30
QUICK GROWING								
BROWN TOP MILLET	20		20		20		20	
ANNUAL RYE GRASS		20		20		20		20
TOTAL POUNDS PER ACRE	35	35	60	65	35	35	60	65

NOTE: THE SEEDING RATES SHOWN IN THIS TABLE APPLY ONLY WHEN SEED IS SPREAD BY AN APPROVED MECHANICAL SPREADER MEETING THE REQUIREMENTS OF SECTION 570 AND 577 OF THE STANDARD SPECIFICATIONS.

GENERAL NOTES

1. SPECIAL ATTENTION IS TO BE DIRECTED TO THE CONSTRUCTION OF THE REQUIRED 1" DROP–OFF AT EDGE OF PAVEMENT.
2. FERTILIZE ENTIRE UNPAVED SHOULDER AND FRONT SLOPE TO TOE OF SLOPE OR BOTTOM OF DITCH.
3. TOPSOIL OBTAINED FROM BORROW PITS OR OTHER SOURCES MAY BE USED IN LIEU OF EXCAVATED TURF AND TOPSOIL WHEN ECONOMICALLY FEASIBLE. NO ADDITIONAL PAYMENT WILL BE MADE FOR SUBSTITUTING TOPSOIL FOR EXCAVATED TURF OR TOPSOIL.

TOPSOIL: IF THE QUANTITY OF EXISTING STORED OR EXCAVATED TOPSOIL IS INADEQUATE FOR PLANTING, SUFFICIENT ADDITIONAL TOPSOIL SHALL BE FURNISHED. TOPSOIL FURNISHED SHALL BE A NATURAL, FERTILE, FRIABLE SOIL, POSSESSING CHARACTERISTICS OF REPRESENTATIVE PRODUCTIVE SOILS IN THE VICINITY. IT SHALL BE OBTAINED FROM NATURALLY WELL–DRAINED AREAS.

TOPSOIL SHALL BE WITHOUT ADMIXTURE OF SUBSOIL AND FREE FROM JOHNSON GRASS (SORGHUM HALEPENSE), NUT GRASS (CYPERUS ROTUNDUS) AND OBJECTIONABLE WEEDS AND TOXIC SUBSTANCES.

GROUND LIMESTONE (DOLOMITE) CONTAINING NOT LESS THAN 85 PERCENT OF TOTAL CARBONATES, AND SHALL BE GROUND TO SUCH A FINENESS THAT 50 PERCENT WILL PASS A 100–MESH SIEVE AND 90 PERCENT WILL PASS A 20–MESH SIEVE.

16–16–16 FORMULATION OF WHICH 60 PERCENT OF THE NITROGEN IS IN THE UREA–FORMALDEHYDE FORM AND SHALL CONFORM TO THE APPLICABLE STATE FERTILIZER LAWS. IT SHALL BE GRANULATED SO THAT 80 PERCENT IS HELD ON A 16–MESH SCREEN, UNIFORM IN COMPOSITION, DRY AND FREE–FLOWING. MULCH: CLEAN HAY OR FREE STRAW MULCH.

AREAS TO BE GRASSED SHALL BE GRADED TO REMOVE DEPRESSIONS, UNDULATIONS, AND IRREGULARITIES IN THE SURFACE BEFORE GRASSING.

PLACING TOPSOIL: AREAS TO BE GRASSED SHALL HAVE A MINIMUM TOPSOIL OVER OF TWO INCHES. TOPSOIL SHALL NOT BE PLACED WHEN THE SUBGRADE IS EXCESSIVELY WET, EXTREMELY DRY OR IN A CONDITION OTHERWISE DETRIMENTAL TO THE PROPOSED PLANTING OR PROPER GRADING

TILLAGE: THE AREA TO BE GRASSED SHALL BE THOROUGHLY TILLED TO A DEPTH OF FOUR INCHES USING A PLOW AND DISC HARROW OR ROTARY TILLING MACHINERY UNTIL A SUITABLE BED HAS BEEN PREPARED AND NO CLODS OR CLUMPS REMAIN LARGER THAN 1–1/2 INCHES IN DIAMETER.

APPLICATION OF LIME: THE PH OF THE SOIL SHALL BE DETERMINED. IF THE PH IS BELOW 5.0, SUFFICIENT LIME SHALL BE ADDED TO PROVIDE A PH BETWEEN 5.5 AND 6.5. THE LIME SHALL BE THOROUGHLY INCORPORATED INTO THE TOP THREE TO FOUR INCHES OF THE SOIL. LIME AND FERTILIZER MAY BE APPLIED IN ONE OPERATION.

APPLICATION OF FERTILIZER: FERTILIZER SHALL BE APPLIED AT THE RATE OF 6 POUNDS PER 1,000 SQUARE FEET AND SHALL BE THOROUGHLY INCORPORATED INTO THE TOP THREE TO FOUR INCHES OF SOIL.

ALL AREAS DISTURBED DURING CONSTRUCTION SHALL BE SEEDED AS SPECIFIED HEREIN. IMMEDIATELY BEFORE SEEDS ARE SOWN AND AFTER FERTILIZER AND LIME ARE APPLIED, THE GROUND SHALL BE SCARIFIED AS NECESSARY AND SHALL BE RAKED UNTIL HE SURFACE IS SMOOTH, FRIABLE, AND OF UNIFORMLY FINE TEXTURE. AREAS TO BE GRASSED SHALL BE SEEDED EVENLY WITH A MECHANICAL SPREADER, RAKED LIGHTLY, ROLLED WITH A 200–POUND ROLLER, AND WATERED WITH A FINE SPRAY.

SEEDS SHALL BE APPLIED AT THE FOLLOWING RATE:

SEEDS RATE OF APPLICATION

BERMUDA 6 LBS./1000 SQ.FT.

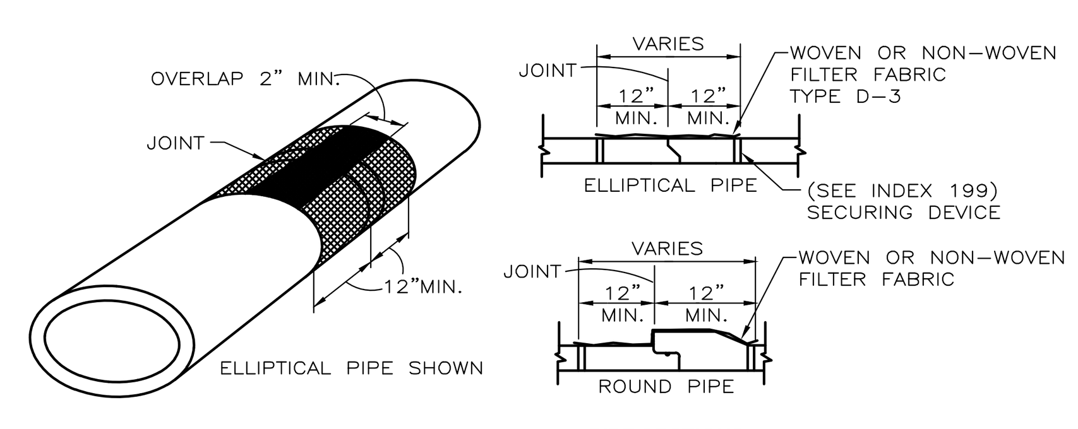
SEEDED AREAS SHALL BE MULCHED AT THE RATE OF NOT LESS THAN 1–1/2" LOOSE MEASUREMENT OVER ALL SEEDED AREAS. SPREAD BY HAND, BLOWER, OR OTHER SUITABLE EQUIPMENT. MULCH SHALL BE CUT INTO THE SOIL WITH EQUIPMENT CAPABLE OF CUTTING THE MULCH UNIFORMLY INTO THE SOIL. MULCHING SHALL BE DONE WITHIN 24 OURS OF THE TIME SEEDING IS COMPLETED.

AFTER SEEDING AND MULCHING, A CULTIPACKER, TRAFFIC ROLLER, OR OTHER SUITABLE EQUIPMENT SHALL BE USED FOR ROLLING THE GRASSED AREAS. AREAS SHALL THEN BE WATERED WITHIN A FINE SPRAY.

ALL AREAS TO BE GRASSED SHALL BE PROTECTED AGAINST EROSION AT ALL TIMES. FOR PROTECTION DURING WINTER MONTHS (NOVEMBER 1ST THRU MARCH 31ST) ITALIAN RYE GRASS SHALL BE PLANTED AT A RATE OF FOUR POUNDS PER 1,000 SQUARE FEET ON ALL AREAS WHICH ARE NOT PROTECTED BY PERMANENT GRASS.

SEEDING DETAIL

NOT TO SCALE



COST OF FILTER FABRIC JACKET TO BE INCLUDED IN COST OF PIPE CULVERTS. FOR ALL PIPE TYPES – CONCRETE PIPE SHOWN

FILTER FABRIC JACKET DETAIL

NOT TO SCALE

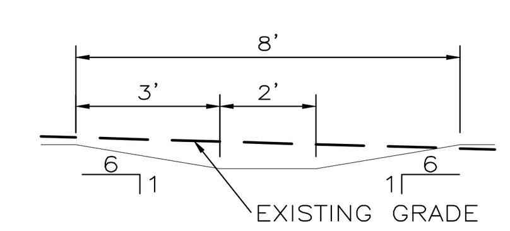
MINIMUM TECHNICAL STANDARDS CHECKLIST FOR UTILITY AS-BUILTS

CITY OF PANAMA CITY BEACH
DATED MAY, 2012

SURVEYORS AND MAPPERS MUST MEET THE FOLLOWING MINIMUM STANDARDS OF ACCURACY, COMPLETENESS, AND QUALITY FOR THE CITY OF PANAMA CITY BEACH TO ACCEPT AS-BUILTS:

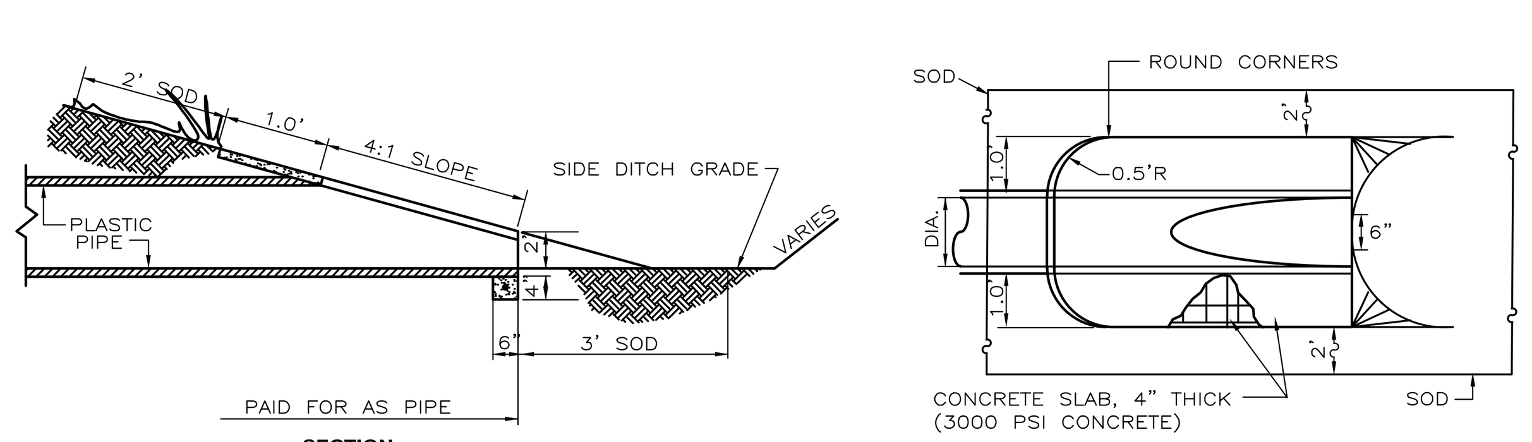
1. MUST IDENTIFY THE RESPONSIBLE SURVEYOR AND MAPPER.
2. SHALL STATE THE TYPE OF SURVEY IT DEPICTS AND THE PURPOSE OF THE SURVEY.
3. MUST BEAR THE NAME, CERTIFICATE OF AUTHORIZATION NUMBER, AND STREET AND MAILING ADDRESS OF THE BUSINESS ENTITY ISSUING THE AS–BUILT SURVEY, ALONG WITH THE NAME AND LICENSE NUMBER OF THE SURVEYOR IN RESPONSIBLE CHARGE.
4. MUST REFLECT A SURVEY DATE, WHICH IS THE DATE OF ACQUISITION. WHEN THE GRAPHICS OF THE AS–BUILT SURVEY ARE REVISED, BUT THE SURVEY DATE STAYS THE SAME, THE AS–BUILT SURVEY MUST LIST DATES FOR ALL REVISIONS.
5. MUST BE SIGNED AND SEALED BY THE SURVEYOR IN RESPONSIBLE CHARGE.
6. A DESIGNATED "NORTH ARROW" AND EITHER A STATED SCALE OR GRAPHIC SCALE SHALL BE SHOWN.
7. APPROPRIATE LINE TYPES, LINE WEIGHTS, AND LINE WIDTHS SHALL BE USED ON THE AS–BUILT DRAWING TO DIFFERENTIATE EXISTING FROM PROPOSED AND WATER FROM SEWER, RECLAIM, AND STORM. ALL PHYSICAL ITEMS (I.E. PIPES, VALVES, ETC.), SURVEYED BOUNDARIES, AND EASEMENTS SHOULD BE CLEARLY MARKED, AND DIMENSIONED, AND IDENTIFIED BY SIZE AND MATERIAL.
8. ALL UTILITIES IN THE PUBLIC RIGHT OF WAY AND WITHIN EASEMENTS OR TO THE END OF THE PUBLICLY OWNED PORTION OF THE UTILITY (I.E. METER AND BACKFLOW PREVENTER, CLEANOUT, ETC.) SHALL BE SHOWN WITH ASSOCIATED SIZES LABELED. THIS INCLUDES, BUT IS NOT LIMITED TO, STUB–OUTS/LATERALS, METERS, BFP'S, WATER MAINS, FORCE MAINS, GRAVITY SEWER MAINS, MANHOLES, STORM WATER PIPING AND ASSOCIATED STRUCTURES, VALVES, FIRE HYDRANTS, LIFT STATIONS, ETC. ALL PIPE LINE WORK MUST BE CONNECTED WITHIN THE SITE AS WELL AS THE CONNECTION TO EXISTING UTILITIES ADJACENT TO THE SITE (IT IS THE SURVEYOR'S RESPONSIBILITY TO COORDINATE WITH ALL CONTRACTORS FOR LOCATIONS AND SIZING). ALL UTILITY CONNECTIONS TO THE BUILDINGS MUST BE SHOWN.
9. ALL PROPOSED UTILITY/INGRESS/EGRESS EASEMENTS MUST BE SHOWN ON THE DRAWING AND MUST HAVE THE ASSOCIATED LEGAL DESCRIPTION WRITTEN.
10. EDGE OF PAVEMENT, ROADS (ASPHALT SHADED), CURBS, DRIVEWAY CONNECTIONS, BUILDINGS, PARKING LOTS, RIGHT–OF–WAY, AND STREET NAMES MUST BE SHOWN IN ALL APPLICATIONS. ALL ITEMS MENTIONED ABOVE MUST BE FIELD LOCATED.
11. IF A LIFT STATION IS TO BE DEDICATED TO THE CITY THE PLAN MUST SHOW A DETAIL SCALED AT 1"=10' SHOWING ALL IMPROVEMENTS INCLUDING: WATER AND SEWER SERVICES, MANHOLES, INVERTS, RIMS, BFP'S, YARD HYDRANTS, CONTROL PANELS, FENCING, PARCEL BOUNDARY, LEGAL DESCRIPTION OF PARCEL BOUNDARY, WET WELL, VALVE BOX, FORCE MAIN, FLOW METER (IF APPLICABLE), DRIVEWAY, GATE.
12. PROPERTY BOUNDARY MUST BE CLEARLY LABELED AND DIMENSIONED.
13. INVERTS, GRATES, TOPS, RIMS MUST BE SHOWN FOR ALL STORM WATER DRAINAGE STRUCTURES. INVERTS (PIPES AND CLEANOUTS) AND RIMS MUST BE SHOWN FOR ALL GRAVITY SEWER MANHOLES. SLOPES MUST BE SHOWN ON EACH RUN OF PIPE FOR REVIEW AND APPROVAL.
14. "AS–BUILT" PROFILE OF ALL DIRECTIONAL BORES AND JACK–AND–BORES INDICATING GRADE AND PIPE ELEVATIONS AT 10 FOOT INTERVALS SHALL BE PROVIDED ON AS–BUILT PLAN SHEETS BASED ON BORE LOGS DEVELOPED BY BORING CONTRACTOR DURING INSTALLATION. PROFILES SHALL USE HORIZONTAL STATIONING WHICH TIES TO STATIONING ON PLANS. PROFILES SHALL ALSO SHOW EXISTING SURFACE ELEVATIONS AS WELL AS ANY PROPOSED SURFACE ELEVATIONS ON THE PROFILE. SURFACE PROFILES MUST SHOW ANY PAVEMENT, SIDEWALKS, DITCHES, SWALES ETC. NOTE THAT PROFILES LOCATING PIPE SOLELY BY "DEPTH BELOW EXISTING GROUND" WILL NOT BE ACCEPTED.
15. COASTAL SETBACK LINE OR COASTAL CONSTRUCTION CONTROL LINE SHOULD BE DESIGNATED.
16. ELEVATIONS AND LOCATION OF ANY FLOOD ZONES ALONG THE FLOOD HAZARD BOUNDARIES SHALL BE DELINEATED.
17. NEARBY WETLANDS AND OTHER ENVIRONMENTALLY SIGNIFICANT RESOURCES CLEARLY LABELED.
18. STORM WATER MANAGEMENT SYSTEM FEATURES INCLUDING DIMENSIONS OF : WET AND DRY SWALES, WET AND DRY PONDS, CONVEYANCE SYSTEMS, EASEMENTS, ALONG WITH ALL ASSOCIATED M.E.S. STRUCTURES AND INVERTS, OUTFALL STRUCTURES AND INVERTS, SKIMMERS, DISCHARGE STRUCTURES AND INVERTS AND SLOT ELEVATIONS, TOP OF BANK, SLOPE OF BANK AND BOTTOM OF ALL PONDS, SWALES, CLOSED AND OPEN CONVEYANCES. FOR FEMA LOMR SUBMITTALS ALSO PROVIDE: FINISHED FLOOR ELEVATIONS, SPOT ELEVATIONS AND/OR CONTOURS SHOWING LOWEST LOT ELEVATIONS.
19. THE ENGINEER OF RECORD SHALL REVIEW AND APPROVE THE AS–BUILT PRIOR TO SUBMISSION TO THE CITY FOR FINAL APPROVAL. WRITTEN APPROVAL BY THE ENGINEER OF RECORD SHALL BE NOTED ON A TRANSMITTAL WITH A STATEMENT OF NO EXCEPTIONS TO MINIMUM STANDARDS PROVIDED HEREIN.

STORM WATER REQUIREMENTS FOR THE AS–BUILT SURVEYS ONLY APPLY TO PARCELS WITHIN CITY LIMITS. PLEASE SUBMIT THREE (3) HARD COPIES AND ONE (1) DIGITAL (AUTOCAD FORMAT & PDF) FOR REVIEW AND APPROVAL.



TYPICAL SWALE DETAIL

NOT TO SCALE



CONCRETE MITERED END DETAIL

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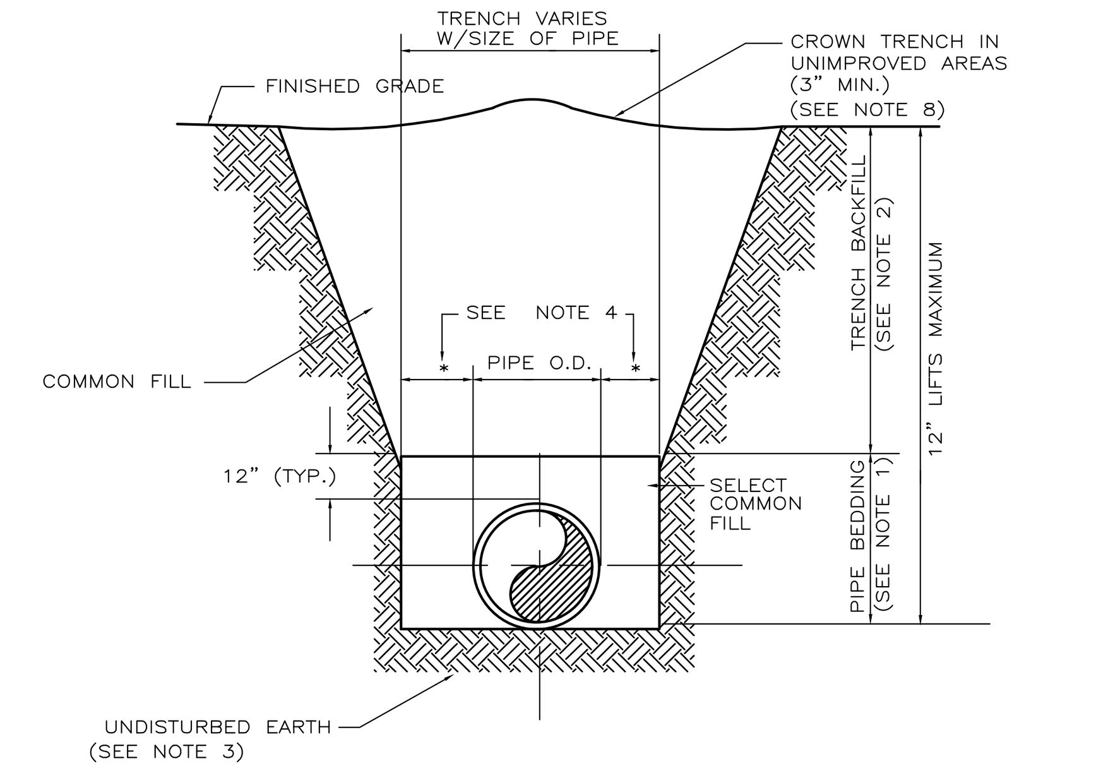


NOTES:

1. PIPE BEDDING: SELECT COMMON FILL COMPACTED TO 95% OF THE MAXIMUM DENSITY AS PER AASHTO T–180.
2. TRENCH BACKFILL: COMMON FILL COMPACTED TO 95% (6" LIFTS) OF THE MAXIMUM DENSITY AS PER AASHTO T–180.
3. PIPE BEDDING UTILIZING SELECT COMMON FILL OR BEDDING ROCK IN ACCORDANCE WITH TYPE A BEDDING AND TRENCHING DETAIL MAY BE REQUIRED AS DIRECTED BY THE CITY.
4. (*): 15" MAX. FOR PIPE DIAMETER LESS THAN 24", AND 24" MAX. FOR PIPE DIAMETER 24" AND LARGER.
5. WATER SHALL NOT BE PERMITTED IN THE TRENCH DURING CONSTRUCTION.
6. ALL PIPE TO BE INSTALLED WITH BELL FACING UPSTREAM TO THE DIRECTION OF THE FLOW
7. REFER TO SECTION 32.5 OF THE MANUAL FOR SHEETING AND BRACING IN EXCAVATIONS.
8. FINAL RESTORATION IN IMPROVED AREAS SHALL BE IN COMPLIANCE WITH ALL APPLICABLE REGULATIONS OF GOVERNING AGENCIES. SURFACE RESTORATION WITHIN CITY RIGHT–OF–WAY SHALL COMPLY WITH REQUIREMENTS OF RIGHT–OF–WAY UTILIZATION REGULATIONS AND ROAD CONSTRUCTION SPECIFICATIONS.

TRENCHES AND EXCAVATION PITS SHALL NOT BE BACKFILLED UNTIL ALL TESTS AND INSPECTIONS COVERING THE INSTALLATION OF THE STORM DRAINAGE SYSTEM HAVE BEEN PERFORMED AND APPROVED.

ALL TIMBER SHEETING BELOW A PLANE ONE FOOT ABOVE TOP OF PIPE SHALL REMAIN IN PLACE IN ORDER NOT TO DISTURB PIPE GRADING. BEFORE BACKFILLING, REMOVE ALL OTHER SHEETING, BRACING AND SHORING. PIPE TO BE CAREFULLY COMPACTED TO NINETY FIVE PERCENT (95%) OF MAXIMUM DENSITY AS PER ASTM D–1557 UNTIL ONE FOOT (1') OF COVER EXISTS OVER PIPE.

IN STREETS, DRIVES, PARKING LOTS AND OTHER AREAS TO HAVE OR HAVING IMPROVED HARD SURFACES, BACKFILL SHALL BE MATERIAL SPECIFIED AS FOR PIPE BEDDING AND SHALL BE DEPOSITED IN SIX INCH (6"0 LOOSE LAYERS AS OPTIMUM MOISTURE CONTENT (+ 2%) AND COMPACTED TO 95% OF MAXIMUM DENSITY AS DETERMINED BY ASTM D–1557 WHERE SERVICES OR UTILITY LINES CROSS STREET, BEDDING SHALL BE CARRIED TO FIVE FEET (5') BEHIND THE CURB, OR WHERE SIDEWALKS EXISTS, TO THE SIDE OF SIDEWALK FARTHEST AWAY FROM THE STREET.

MATERIAL USED FOR BEDDING SHALL MEET CURRENT RECOMMENDATIONS OF THE PIPE MANUFACTURER AND SHALL BE APPROVED BY THE ENGINEER. THE SPECIFIED COHESIONLESS MATERIAL SHALL BE PLACED IN THE TRENCH SIMULTANEOUSLY ON EACH SIDE OF THE PIPE TO THE FULL WIDTH OF THE TRENCH. MATERIAL WILL BE PLACED IN A MAXIMUM LIFT OF SIX (6) INCHES (COMPACTED DEPTH) TO A MINIMUM DEPTH OF ONE (1) FOOT ABOVE THE CROWN OF THE PIPE.

PERFORM ALL TRENCHING REQUIRED FOR THE INSTALLATION OF UTILITIES AS SHOWN ON PLANS AND SPECIFIED HEREIN. MAKE ALL TRENCHES OPEN VERTICAL CONSTRUCTION WITH SUFFICIENT WIDTH TO PROVIDE FREE WORKING SPACE AT BOTH SIDE OF THE TRENCH AND AROUND THE INSTALLED ITEMS AS REQUIRED FOR CAULKING, JOINING, BACKFILLING AND COMPACTING.

PROPERLY SUPPORT ALL TRENCHES IN STRICT ACCORDANCE WITH ALL PERTINENT RULES AND REGULATIONS.

GRADE THE TRENCH BOTTOM TO PROVIDE A SMOOTH, FIRM AND STABLE FOUNDATION FREE OF ROCK POINTS THROUGHOUT THE LENGTH OF THE PIPE. IN AREAS WHERE SOFT, UNSTABLE MATERIALS ARE ENCOUNTERED AT THE SURFACE UPON WHICH COHESIONLESS MATERIAL IS TO BE PLACED, REMOVE THE UNSTABLE MATERIAL AND REPLACE IT WITH MATERIAL APPROVED BY THE ENGINEER, MAKE SUFFICIENT DEPTH TO DEVELOP A FIRM FOUNDATION FOR THE ITEM BEING INSTALLED.

AT EACH JOINT IN PIPE, RECESS THE BOTTOM OF THE TRENCH AS REQUIRED INTO THE FIRM FOUNDATION IN SUCH A MANNER AS TO RELIEVE THE BELL OF THE PIPE OF ALL LOAD AND TO ENSURE CONTINUOUS BEARING OF THE PIPE BARREL ON THE FIRM FOUNDATION.

ACCURATELY SHAPE ALL PIPE SUBGRADE AND FIT THE BOTTOM OF THE TRENCH TO THE PIPE SHAPE; USE A DRAG TEMPLATE SHAPED TO CONFORM TO THE OUTER SURFACE OF THE PIPE IF OTHER METHODS DO NOT PRODUCE SATISFACTORY RESULTS. SHAPING WILL CONFORM TO THE OUTSIDE OF THE PIPE FOR A DEPTH OF NOT LESS THAN 10% OF THE TOTAL HEIGHT (OUTSIDE DIMENSION) OF THE PIPE.

PIPE TRENCHES SHALL BE EXCAVATED TO A DEPTH THAT WILL INSURE A MINIMUM OF THIRTY INCHES OF COVER LESS OTHERWISE SHOWN ON THE DRAWINGS OR DIRECTED.

BACKFILL OF EARTH AROUND MANHOLES SHALL BE FILLED WITH THOROUGHLY COMPACTED SAND OR GRAVEL AT THE EXPENSE OF THE CONTRACTOR.

TRENCHES SHALL BE BACKFILLED WITH EXCAVATED MATERIALS, FREE FROM LARGE CLODS OR STONES. BACKFILL SHALL BE DEPOSITED IN LAYERS NOT TO EXCEED 6–INCHES (6") IN THICKNESS, MOISTENED, AND COMPACTED TO DENSITY EQUAL TO OR GREATER THAN 95% OF THE MAXIMUM DENSITY OF AASHTO STANDARD METHOD T–99 , TO A MINIMUM DEPTH OF 12–INCHES OVER THE PIPE. THE REMAINDER OF THE BACKFILL SHALL BE PLACED IN 8–INCH LAYERS COMPACTED TO 95% MAXIMUM DENSITY UNLESS THE BACKFILL IS BENEATH PAVED OR BUILDING AREAS IN WHICH CASE IT SHALL BE COMPACTED TO 95% OF A MODIFIED PROCTOR.

EXCAVATIONS FOR PIPE LAYING OPERATIONS SHALL BE CONSTRUCTED IN A MANNER TO CAUSE THE LEAST INTERRUPTION TO TRAFFIC. WHEN TRAFFIC MUST CROSS OPEN TRENCHES THE CONTRACTOR SHALL PROVIDE SUITABLE BRIDGES.

TYPE B BEDDING AND TRENCHING DETAIL

NOT TO SCALE

ROBERT L. CARROLL LICENSE No. 57988 STATE OF FLORIDA PROFESSIONAL ENGINEER

2/6/2026

Robert L. Carroll, P.E.
PROFESSIONAL ENGINEER
FL LC # 57988

THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY ROBERT L. CARROLL, PE. ON DATE USING A DIGITAL SIGNATURE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED WITHOUT THE DIGITAL COPY. THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES IN ACCORDANCE WITH RULE 61G15–23.004 F.A.C.

REVISIONS

NO.	DESCRIPTION	DRAWN	CHECKED	DATE

PHASE	DRAWN	CHECKED	DATE
SCHEMATIC DESIGN	MAY	SLB	10/17/25
DESIGN DEVELOPMENT	MAY	SLB	1/9/26
CONSTRUCTION DOCUMENTS	MAY	SLB	4/3/26

JRA ARCHITECTS
2211 NAVY BLVD. , STE 100
PANAMA CITY BEACH, FL
PHONE: (850) 236-9832
Commission Number: 25871

CONSULTANTS:

PROJECT:

BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD HOUSE
MOSLEY HIGH SCHOOL

LYNN HAVEN, FLORIDA

SHEET TITLE:

CONSTRUCTION DETAILS

SHEET NUMBER:

C2.2

SITE UTILITIES

MATERIALS: WHERE GROUND IS FOUND UNSUITABLE TO SUPPORT PIPE, PROVIDE CRADLES OF 2500 PSI. CONCRETE FULL WIDTH OF TRENCH WITH TWO NO. 4 REINFORCING BARS CONTINUOUSLY ALONG THE BOTTOM OF PIPE.

BACKFILL, UNLESS OTHERWISE NOTED, SHALL BE COARSE SAND, FINE GRAVEL OR EARTH HAVING A LOW PLASTICITY INDEX, FREE OF ROCKS, DEBRIS AND OTHER FOREIGN MATERIALS AND DEFINED AS ALL PASSING THROUGH A 3/8" SIEVE AND NOT MORE THAN TEN PERCENT (10%) BY VOLUME PASSING THROUGH A 200 MESH SIEVE.

UTILITY PIPING AND FITTINGS SHALL BE SIZE AND TYPE INDICATED ON THE DRAWINGS AND SHALL CONFORM TO THE FOLLOWING: MANHOLES STRUCTURES SHALL BE SIZE AND TYPE INDICATED ON THE DRAWINGS AND SHALL BE CONSTRUCTED OF THE FOLLOWING:

REINFORCED PRECAST CONCRETE MANHOLE SECTIONS INCLUDING CONCENTRIC OR ECCENTRIC CONES AND GRADE RINGS SHALL BE 4000 PSI. CONCRETE AND CONFORM TO ASTM C-478 OR AASHTO M-199. SECTIONS SHALL BE COMPLETE WITH 3/4" ROUND CAST IN PLACE WROUGHT IRON STEPS.

BRICK SHALL BE SOUND, HARD BURNED THROUGHOUT AND OF UNIFORM SIZE AND QUALITY AND SHALL BE IN ACCORDANCE WITH ASTM C-32, GRADE MS OR MM.

CONCRETE MASONRY SHALL BE SOLID PRECAST SEGMENTAL CONCRETE MASONRY UNITS CONFORMING TO ASTM C-139.

IRON CASTING SHALL CONFORM TO ASTM A-48, CLASS 30. BEARING SURFACES BETWEEN CAST IRON FRAMES, COVERS, GRATES SHALL BE MACHINED, FITTED TOGETHER AND MATCH MARKED TO PREVENT ROCKING. SYSTEM IDENTIFYING LETTER 2" HIGH SHALL BE STAMPED OR CAST INTO ALL COVERS SO THAT THEY MAY BE PLAINLY VISIBLE. CASTING SHALL BE MANUFACTURED BY EAST JORDAN IRON WORKS, INC., NEENAH FOUNDRY COMPANY OR EQUAL.

CONCRETE AND MASONRY MATERIALS FOR CONSTRUCTION OF SITE UTILITY STRUCTURES AND PADS SHALL CONSIST OF THE FOLLOWING:

PORTLAND CEMENT SHALL BE STANDARD BRAND OF PORTLAND CEMENT CONFORMING TO ASTM C-150, TYPE I OR II.
FINE OR COARSE AGGREGATES FOR CONCRETE SHALL BE PER ASTM C-33. AGGREGATES SHALL BE WELL GRADED FROM FINE TO COARSE WITHIN LIMITS SPECIFIED IN ASTM C-33. MAXIMUM SIZE OF COARSE AGGREGATE SHALL BE 3/4".
AGGREGATE FOR CEMENT MORTAR SHALL BE CLEAN, SHARP SAND CONFORMING TO ASTM C-144. GRADE SAND FROM COARSE TO FINE WITH 100% PASSING NO. 8 SIEVE, AND NOT OVER 10% TO 30% PASSING NO. 50 SIEVE.
HYDRATED LIME SHALL COMPLY WITH ASTM C-207, TYPE S.
WATER SHALL BE CLEAN AND FREE FROM DELETERIOUS MATERIALS.

REINFORCING STEEL FOR CONCRETE SHALL BE INTERMEDIATE GRADE NEW BILLET STEEL CONFORMING TO ASTM A-615, GRADE 40.

FORMS FOR CONCRETE WORK SHALL BE WOOD. FORMS SHALL BE SUFFICIENT STRENGTH TO PREVENT DEFORMATIONS UNDER LOAD AND TIGHT ENOUGH TO PREVENT LEAKAGE. FOUNDATIONS MAY BE POURED AGAINST EARTH WHERE CONDITIONS PERMIT.

CONCRETE, UNLESS OTHERWISE NOTED, SHALL HAVE COMPRESSIVE STRENGTH AFTER 28 DAYS OF 3000 PSI MINIMUM. MIX SHALL BE SO PROPORTIONED TO PROVIDE A MINIMUM OF 517 POUNDS OF CEMENT PER CUBIC YARD. CONCRETE FILL BELOW GRADE FOR THRUST BLOCKS, PIPE CRADLES ETC. MAY BE 2500 PSI. AT 28 DAYS.

CONCRETE, WHERE EXPOSED TO THE WEATHER, SHALL BE AIR ENTRAINED. AIR ENTRAINMENT SHALL BE ACCOMPLISHED BY THE USE OF ADDITIVES CONFORMING TO ASTM C-260. AIR CONTENT SHALL BE 6% + 1%. ADDITIVE SHALL BE USED IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED DIRECTIONS.

READY-MIX CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF ASTM C-94.

TYPE M, AVERAGE COMPRESSIVE STRENGTH 2500 PSI. AT 28 DAYS. MORTAR MIX SHALL BE PROPORTIONED BY VOLUME.

MORTAR FOR PARGING SHALL CONSIST OF ONE PART PORTLAND CEMENT AND TWO PARTS SAND.

BACKFILL SHALL BE SAME MATERIAL SPECIFIED FOR PIPE BEDDING. WHERE SERVICE OR UTILITY LINES CROSS A STREET, BEDDING SHALL BE CARRIED TO FIVE FEET (5') BEHIND THE CURB, OR WHERE SIDEWALKS EXIST, TO THE SIDE OF THE SIDEWALK FARTHEST AWAY FROM THE STREET.

FLUSHING REQUIREMENTS FOR WATER AND SEWER FORCE MAINS

FLUSHING TIME SHALL BE AT LEAST THAT AMOUNT OF TIME NEEDED TO FLUSH 6 TIMES THE PIPE VOLUME AFTER 3 FPS VELOCITY IS REACHED OR UNTIL CLEAR, WHICHEVER IS LONGER. MAXIMUM LENGTH OF PIPE BETWEEN FLUSHING ASSEMBLIES SHALL BE 5,000 FEET.

SEWER COLLECTION SYSTEM

POLY (VINYL CHLORIDE) PIPE (PVC): PLASTIC GRAVITY SEWER PIPE AND FITTINGS SHALL BE UNPLASTICIZED POLYVINYL CHLORIDE (PVC) MEETING AND/OR EXCEEDING ASTM SPECIFICATIONS D-3034 (LATEST EDITION).

PIPE LENGTHS SHALL NOT EXCEED 20 FEET AND PROVISIONS SHALL BE MADE AT EACH JOINT TO ACCOMMODATE EXPANSION AND CONTRACTIONS.

COMPLY WITH REQUIREMENTS OF FS RR-F-621, FOR TYPE AND STYLE REQUIRED.

MATERIALS FOR SEWER FORCE MAINS: PVC PIPE FOR FORCE MAINS SHALL CONFORM TO THE REQUIREMENTS OF ASTM SDR-21 FOR PRESSURE RATING OF 200 PSI 230 C (73 DEGREES F). HDPE FORCE MAIN SHALL BE SDR-11. PIPE JOINTS SHALL BE INTEGRAL BELL AND SPIGOT TYPE WITH RUBBER RING SEALING GASKET. THE PIPE BELL SHALL BE DESIGNED TO BE AT LEAST AS STRONG AS THE PIPE WALL. STANDARD LENGTHS SHALL BE 20 FEET, EXCEPT THAT 15% OF TOTAL FOOTAGE FOR A PARTICULAR PROJECT MAY BE RANDOM LENGTHS OF NOT LESS THAN 10 FEET EACH. EACH PIECE OF PIPE SHALL BE TESTED BY THE MANUFACTURER OF 6000 PSI FOR A MINIMUM OF 5 SECONDS. THE BELL SHALL BE TESTED WITH THE PIPE. ALL PIPE SHALL BE LISTED BY UNDERWRITER'S LABORATORIES, INC., AND BY FACTORY MUTUAL AS APPROVED FOR USE IN UNDERGROUND MUNICIPAL WATER DISTRIBUTION SYSTEMS AND PRIVATE FIRE PROTECTION SYSTEM. CAST IRON OR DUCTILE IRON FITTINGS SHALL BE USED WITH PVC PIPE.

CAST IRON FITTINGS SHALL BE MECHANICAL JOINT AND SHALL CONFORM TO ANSI SPECIFICATION A21.10 FOR SIZES 3 INCHES THROUGH 12 INCHES AND SHALL BE CLASS 250. FITTINGS 14 INCHES AND LARGER SHALL BE CLASS 150 AND SHALL BE OF THE DIMENSIONS AND METAL THICKNESSES AS SHOWN IN THE HANDBOOK OF CAST IRON PIPE AS PUBLISHED BY THE CAST IRON PIPE RESEARCH ASSOCIATION. CAST IRON FITTINGS MAY BE USED IN DUCTILE IRON OR CAST IRON LINES, EXCEPT WHERE SHOWN OTHERWISE ON THE DRAWINGS.

DUCTILE IRON FITTINGS SHALL BE DESIGNED FOR PRESSURE RATING OF 250 PSI AND SHALL BE IN ACCORDANCE WITH ANSI SPECIFICATIONS A21.10. FITTING SHALL BE MECHANICAL JOINT. DUCTILE IRON FITTINGS MAY BE USED IN DUCTILE IRON OR CAST IRON LINES, EXCEPT WHERE SHOWN OTHERWISE ON THE DRAWINGS.

THE EXTERIOR OF ALL CAST IRON AND DUCTILE IRON FITTINGS SHALL BE COATED WITH AN APPROVED BITUMINOUS COATING. THE INTERIOR OF THE PIPE SHALL BE EPOXY LINED (PROTECTO 401) IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATION (40 MILS NOMINAL DRY FILM THICKNESS).

MATERIALS FOR CONCRETE MANHOLES: PRECAST OF CAST-IN-PLACE, AT CONTRACTOR'S OPTION. USE CONCRETE WHICH WILL ATTAIN A 28-DAY COMPRESSIVE STRENGTH OF 3000 PSI.

INSPECTIONS AND TESTS: IT IS IMPERATIVE THAT ALL SEWERS AND MANHOLES BE BUILT PRACTICALLY WATERTIGHT AND THAT THE CONTRACTOR MUST ADHERE RIGIDLY TO THE SPECIFICATIONS FOR MATERIAL AND WORKMANSHIP.

THE ALLOWABLE LIMIT OF GROUNDWATER INFILTRATION FOR THE GRAVITY SYSTEM OF NEW SEWERS OR ANY ONE TRUNK, OR INTERCEPTOR, SHALL BE IN COMPLETE ACCORDANCE WITH ASTM 425-71T AND SHALL NOT EXCEED A LIMIT OF INFILTRATION EQUAL TO 0.2 GAL/INCH DIAMETER/HOUR/100 LINEAR FEET OF PIPE.

THE TEST WILL BE MADE BY MEASURING THE INFILTRATED FLOW OF WATER OVER A MEASURING WEIR SET UP IN THE INVERT OF THE SEWER, OR BY ALTERNATE METHOD APPROVED BY THE ENGINEER, A KNOWN DISTANCE FROM A TEMPORARY BULKHEAD OR OTHER LIMITING POINT OF INFILTRATION. AFTER THE SEWER OF SEWERS HAVE BEEN PUMPED OUT, AND NORMAL INFILTRATION CONDITIONS PREVAIL, TESTS SHALL BE STARTED.

TESTS SHALL BE RUN CONTINUOUSLY FOR A PERIOD OF NOT LESS THAN THREE HOURS, WITH WEIR READINGS TAKEN AT 20 MINUTE INTERVALS.

PRESSURE AND LEAKAGE TESTS OF SEWAGE FORCE MAIN PIPING

CONTRACTOR SHALL FURNISH ALL GAUGES, METERS, PRESSURE PUMPS, EQUIPMENT, FITTINGS, AND LABOR NEEDED TO TEST THE LINE. THE COST OF THESE ITEMS SHALL BE INCLUDED IN THE PRICE OF THE PIPE. CONTRACTOR SHALL NOTIFY ENGINEER 48 HOURS PRIOR TO START OF TEST. ALL PIPE INSTALLED SHALL BE TESTED AND WRITTEN ACCEPTANCE ISSUED BY THE ENGINEER PRIOR TO CONNECTION OF NEW LINE TO EXISTING LINES.

THE CONTRACTOR MAY TEST THE SYSTEM WITH JOINTS EXPOSED OR BACKFILLING COMPLETE AT HIS OPTION. THE CONTRACTOR SHALL OBTAIN AND PAY FOR ALL WATER USED. CARE SHALL BE USED TO PREVENT BACKFLOW OF TEST WATER INTO POTABLE WATER SOURCE. POTABLE WATER SOURCE SHALL BE DISCONNECTED PRIOR TO PRESSURIZING TEST LINE. WATER USED DURING TEST SHALL BE TAKEN FROM A CONTAINER, NOT DIRECTLY FROM THE EXISTING WATER SYSTEM.

AT LEAST 24 HOURS PRIOR TO THE START OF THE PRESSURE AND LEAKAGE TEST, PRESSURE SHALL BE RAISED TO 150 PSIG AND HELD TO ALLOW ANY "SOIL CREEP" OR OTHER STRESS RELAXATION TO OCCUR. IF ANY PRESSURE REDUCTION OCCURS DURING THE 24 HOUR "SHAKEDOWN" PERIOD, REESTABLISH THE REQUIRED HYDROSTATIC TEST PRESSURE, THEN PROCEED WITH THE LEAKAGE TEST.

THE PRESSURE REQUIRED FOR THE FIELD HYDROSTATIC PRESSURE TEST SHALL BE 150 PSI. THE CONTRACTOR SHALL PROVIDE TEMPORARY PLUGS AND BLOCKING NECESSARY TO MAINTAIN THE REQUIRED TEST PRESSURE. CORPORATION COCKS AT LEAST ¾ INCHES IN DIAMETER, PIPE RISERS AND ANGLE GLOBE VALVES SHALL BE PROVIDED AT EACH PIPE DEAD-END AND HIGH POINTS IN ORDER TO BLEED AIR FROM THE LINE. DURATION OF PRESSURE TEST SHALL BE AT LEAST TWO HOURS. ALL LEAKS EVIDENT AT THE SURFACE SHALL BE REPAIRED AND LEAKAGE ELIMINATED REGARDLESS OF TOTAL LEAKAGE AS SHOWN BY TEST. LINES WHICH FAIL TO MEET TESTS SHALL REPAIRED AND RETESTED AS NECESSARY UNTIL TEST REQUIREMENTS ARE COMPLIED WITH. DEFECTIVE MATERIALS, PIPES, VALVES AND ACCESSORIES SHALL BE REMOVED AND REPLACED. THE PIPE LINES SHALL BE TESTED IN SUCH SECTION AS MAY BE DIRECTED BY THE ENGINEER BY SHUTTING VALVES OR INSTALLING TEMPORARY PLUGS AS REQUIRED. THE LINE SHALL BE FILLED WITH WATER, ALL AIR REMOVED, AND TEST PRESSURE SHALL BE MAINTAINED IN THE PIPE FOR THE ENTIRE TEST PERIOD BY MEANS OF A GASOLINE OR ELECTRIC DRIVEN TEST PUMP TO BE FURNISHED BY THE CONTRACTOR. ACCURATE MEANS SHALL BE PROVIDED FOR MEASURING THE WATER REQUIRED TO MAINTAIN THIS PRESSURE. THE AMOUNT OF WATER REQUIRED IS A MEASURE OF THE LEAKAGE.

NO PIPE INSTALLATION WILL BE ACCEPTED UNTIL THE LEAKAGE (EVALUATED ON A PRESSURE BASIS OF 150 PSI) IS LESS THAN 2.2 GALLONS PER 24 HOURS PER THOUSAND FEET PER INCH NOMINAL DIAMETER. THE FOLLOWING TABULATES THE ALLOWABLE LEAKAGE:

ALLOWABLE LEAKAGE PER 1000 FT OF PIPELINE (IN GALLONS)

DURATION OF TEST	2	3	4	6	8	10	12	14
1 HOUR	0.18	0.28	0.37	0.55	0.74	0.92	1.10	1.29
2 HOURS	0.37	0.55	0.74	1.10	1.47	1.84	2.20	2.57

WHERE ANY SECTION OF A MAIN IS PROVIDED WITH CONCRETE REACTION BACKING THE HYDROSTATIC PRESSURE TEST SHALL NOT BE MADE UNTIL AT LEAST FIVE (5) DAYS HAVE ELAPSED AFTER THE CONCRETE REACTION BACKING WAS INSTALLED. IF HIGH EARLY-STRENGTH CEMENT IS USED IN THE CONCRETE REACTION BACKING, THE HYDROSTATIC PRESSURE TEST SHALL NOT BE MADE UNTIL AT LEAST THREE (3) DAYS HAVE ELAPSED.

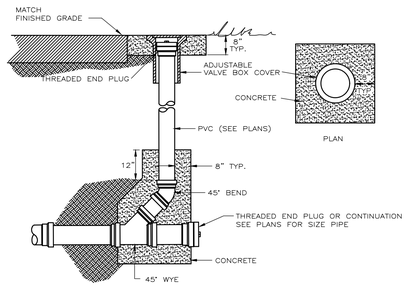
LEAKAGE TESTS FOR GRAVITY SEWER

LINES SHALL BE TESTED FOR LEAKAGE BY LOW PRESSURE AIR TESTING. LOW PRESSURE AIR TESTING FOR CONCRETE PIPES SHALL BE AS PRESCRIBED IN ASTM C 828. LOW PRESSURE AIR TESTING FOR PVC PIPE SHALL BE AS PRESCRIBED IN ASTM F1417, AND PRESSURE DROP LIMITS SHALL BE DETERMINED BY USING ASTM F1417 TABLE 1, SHOWN BELOW. LOW PRESSURE AIR TESTING PROCEDURES FOR OTHER PIPE MATERIALS SHALL USE THE PRESSURES AND TESTING TIMES PRESCRIBED IN ASTM C 828 AND ASTM C 924, AFTER CONSULTATION WITH THE PIPE MANUFACTURER. VISIBLE LEAKS ENCOUNTERED SHALL BE CORRECTED REGARDLESS OF LEAKAGE TEST RESULTS. WHEN LEAKAGE EXCEEDS THE MAXIMUM AMOUNT SPECIFIED, SATISFACTORY CORRECTION SHALL BE MADE AND RETESTING ACCOMPLISHED. TESTING, CORRECTION, AND RETESTING SHALL BE MADE AT NO ADDITIONAL COST TO THE OWNER.

ASTM F1417 TABLE 1
MINIMUM SPECIFIED TIME REQUIRED FOR 1.0 PSIG PRESSURE DROP FOR SIZE AND LENGTH OF PIPE INDICATED FOR Q=0.0015

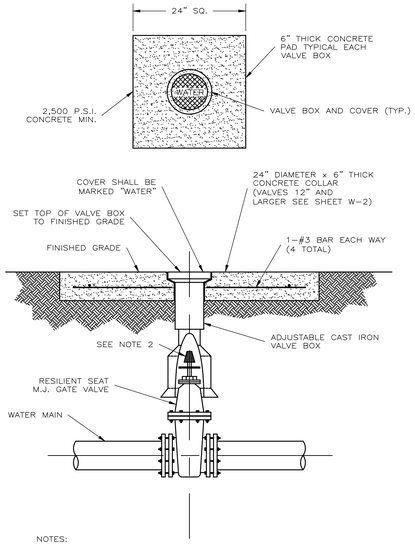
1. SEE PRACTICE UNI-B-6-90.
2. CONSULT WITH PIPE AND APPURTENANCE MANUFACTURER FOR MAXIMUM TEST PRESSURE FOR PIPE SIZE GREATER THAN 30IN DIA.

PIPE DIAMETER IN.	MINIMUM TIME MIN:S	LENGTH FOR MINIMUM TIME,FT	TIME FOR LONGER LENGTH,S	SPECIFICATION TIME FOR LENGTH (L) SHOWN, MIN:S							
				100FT	150FT	200FT	250FT	300FT	350FT	400FT	450FT
4	3:46	597	0.380L	3:46	3:46	3:46	3:46	3:46	3:46	3:46	3:46
6	5:40	398	0.854L	5:40	5:40	5:40	5:40	5:40	5:40	5:42	6:24
8	7:34	298	1.520L	7:34	7:34	7:34	7:34	7:36	8:52	10:08	11:24
10	9:26	239	2.374L	9:26	9:26	9:26	9:53	11:52	13:51	15:49	17:48
12	11:20	199	3.418L	11:20	11:20	11:24	14:15	17:05	19:56	22:47	25:38
15	14:10	159	5.342L	14:10	14:10	17:48	22:15	26:42	31:09	35:36	40:04
18	17:00	133	7.692L	17:00	19:13	25:38	32:03	38:27	44:52	51:16	57:41
21	19:50	114	10.470L	19.50	26:10	34:54	43:37	52:21	61:00	69:48	78:31
24	22:40	99	13.674L	22:47	34:11	45:34	56:58	68:22	79:46	91:10	102:33
27	25:30	88	17.306L	28:51	43:16	57:41	72:07	86:32	100:57	115:22	129:48
30	28:20	80	21.366L	35:37	53:25	71:13	89:02	106:50	124:38	142:26	160:15
33	31:10	72	25.852L	43:05	64:38	86:10	107:43	129:16	150:43	172:21	193:53
36	34:00	66	30.768L	51:17	76:55	102:34	128:12	153:50	179:29	205:07	230:46



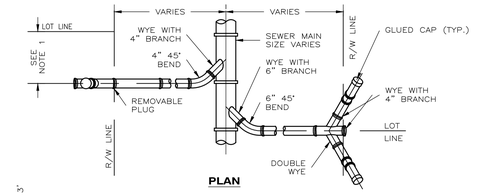
NOTE: SEE SEWER SIZE IN PLANS

CLEANOUT DETAIL
NOT TO SCALE



NOTES:
1. PVC EXTENSIONS SHALL NOT BE USED ON VALVE BOX INSTALLATION.
2. THE ACTUATING NUT FOR DEEPER VALVES SHALL BE EXTENDED TO COME UP TO 4 FOOT DEPTH BELOW FINISHED GRADE.

GATE VALVE 12" & SMALLER
NOT TO SCALE



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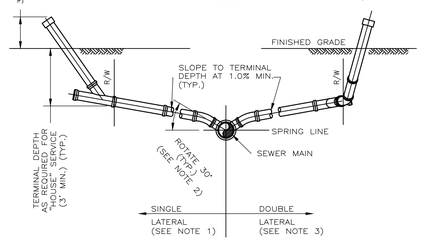
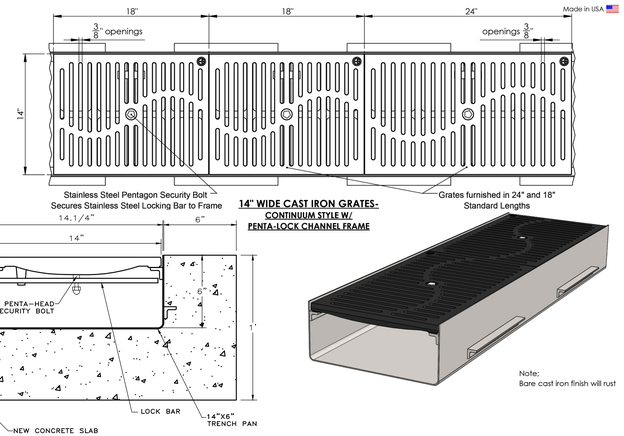


PROFILE

NOTES:
1. LOCATE SINGLE LATERAL AS NEAR TO CENTER OF LOT AS POSSIBLE.
2. INVERT OF SERVICE LATERAL SHALL NOT ENTER SEWER MAIN BELOW SPRING LINE.
3. DOUBLE SERVICE LATERALS ONLY PERMITTED ON TAPS TO EXISTING GRAVITY MAINS WERE EXISTING ROAD PAVEMENT MUST BE CUT.

SERVICE LATERAL DETAIL
NOT TO SCALE



14" WIDE CAST IRON GRATES- CONTINUUM STYLE W/ PENTA-LOCK CHANNEL FRAME

Note:
Bare cast iron finish will rust

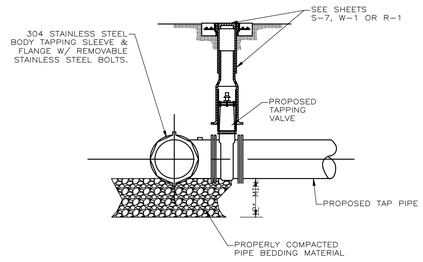
CROSS SECTION

Product No. : T-14C Continuum

Grate Material: ASTM-A536-65-45-12 Ductile Iron	Grate + Frame Weight: 36.3 lbs./LF.
Grate Finish: Plain or anti-skid Black Powder Coat	14" x 18" : Open Area 79.0 sq. in.
Frame Material: A36 Mild Steel	14" x 24" : Open Area 99.0 sq. in.
Frame Finish: Plain or Hot Dip Galvanized	Load Class-B, ADA Compliant Grating

EVERGRATE
A DIVISION OF JR HOE

PLAN



SECTION A-A

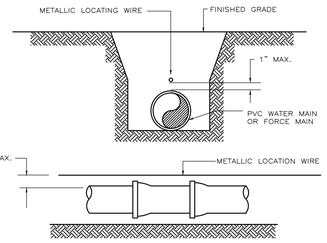
NOTES:

1.) NO TAPPING CUTS SHALL BE MADE BEFORE:
A 60 MINUTE TEST AT 100 P.S.I. FOR FORCEMAINS, OR 150 P.S.I. FOR POTABLE WATERMAINS AND RECLAIM WATERMAINS IS PERFORMED.

2.) ALL TAPS MUST BE PLACED NO CLOSER THAN 30" OR A DISTANCE EQUAL TO (1) MAIN PIPE DIAMETER PLUS (2) TAP PIPE DIAMETERS (WHICHEVER IS LARGER) FROM A JOINT OR FITTING.

3.) CONTRACTOR TO SUPPLY A DRY HOLE, PROPERLY CONFIGURED, FOR TAPPING CREW TO WORK AND A BACK-HOE TO LOWER MACHINE INTO HOLE. TAPPING ASSEMBLY MUST BE BOLTED ON & PRESSURE TESTED BY THE CONTRACTOR & WITNESSED BY THE CITY PRIOR TO TAP.

TAPPING SLEEVE ASSEMBLY AND VALVE BLOCKING DETAIL
NOT TO SCALE



NOTES:
1. PVC PIE SHALL REQUIRE INSULATED METALLIC LOCATING WIRE. (12 GAUGE COPPER) CAPABLE OF DETECTION BY A CABLE LOCATOR AND SHALL BE BURIED DIRECTLY ABOVE THE CENTERLINE OF THE PIPE.
2. LOCATING WIRE SHALL TERMINATE AT THE TOP OF EACH VALVE BOX AND BE CAPABLE OF EXTENDING 12" ABOVE TOP OF BOX IN SUCH A MANNER SO AS NOT TO INTERFERE WITH VALVE OPERATION.
3. USE DUCT TAPE AS NECESSARY TO HOLD WIRE DIRECTLY ON THE TOP OF THE PIPE.
4. ALL SPLICES SHALL BE MADE USING A WATER TIGHT SEALING METHOD APPROVED BY THE CITY.

PVC PIPE LOCATING WIRE DETAIL
NOT TO SCALE

ROBERT L. CARROLL LICENSE No. 57988 STATE OF FLORIDA PROFESSIONAL ENGINEER
2/6/2026

Robert L. Carroll, P.E.
PROFESSIONAL ENGINEER
FL LC # 57988
THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY ROBERT L. CARROLL, PE. ON DATE USING A DIGITAL SIGNATURE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED WITHOUT THE DIGITAL COPY. THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES IN ACCORDANCE WITH RULE 61G15-23.004 F.A.C.

REVISIONS

NO.	DESCRIPTION	DRAWN	CHECKED	DATE

PHASE	DRAWN	CHECKED	DATE
SCHEMATIC DESIGN	MAY	SLB	10/17/25
DESIGN DEVELOPMENT	MAY	SLB	1/9/26
CONSTRUCTION DOCUMENTS	MAY	SLB	4/3/26

JRA ARCHITECTS
2211 NAVY BLVD., STE 100
PANAMA CITY BEACH, FL
PHONE: (850) 236-9832
Commission Number: 25871

CONSULTANTS:

PROJECT:
BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD HOUSE
MOSLEY HIGH SCHOOL
LYNN HAVEN, FLORIDA

SHEET TITLE:
CONSTRUCTION DETAILS

SHEET NUMBER:
C2.3

ASPHALTIC CONCRETE PAVING

PROVIDE ALL LABOR, MATERIALS, EQUIPMENT, SERVICES, ETC. NECESSARY AND INCIDENTAL TO THE COMPLETION OF ALL PAVEMENT AS SHOWN ON THE DRAWINGS AND AS SPECIFIED HEREIN.

SUBMIT A "LETTER OF INTENT" FOR THE FOLLOWING:

ASPHALT PAVING MATERIAL AND MIX DESIGN. PROVIDE COPIES OF MATERIALS CERTIFICATES SIGNED BY MATERIAL PRODUCER AND CONTRACTOR, CERTIFYING THAT EACH MATERIAL ITEM COMPLIES WITH, OR EXCEEDS, SPECIFIED REQUIREMENTS.

WEATHER LIMITATIONS: APPLY PRIME AND TACK COATS WHEN AMBIENT TEMPERATURE IS ABOVE 50 DEGREES F. (10 DEGREES C), AND WHEN TEMPERATURE HAS NOT BEEN BELOW 35 DEGREES F. (1 DEGREE C) FOR 12 HOURS IMMEDIATELY PRIOR TO APPLICATION. DO NOT APPLY WHEN BASE IS WET OR CONTAINS AN EXCESS OF MOISTURE.

CONSTRUCT ASPHALT CONCRETE SURFACE COURSE WHEN ATMOSPHERIC TEMPERATURE IS ABOVE 40 DEGREES F. (4 DEGREES C), AND WHEN BASE IS DRY. SURFACE COURSE MAY BE PLACED WHEN AIR TEMPERATURE IS ABOVE 30 DEGREES F. (-1 DEGREE C) AND RISING.

GRADE CONTROL: ESTABLISH AND MAINTAIN REQUIRED LINES AND ELEVATIONS.

THE SUBCONTRACTOR SHALL WARRANT ALL ASPHALT PAVING AGAINST DEFECTS IN MATERIALS AND WORKMANSHIP FOR A PERIOD OF TWO YEARS.

PRODUCTS: USE LOCALLY AVAILABLE MATERIALS AND GRADATIONS WHICH EXHIBIT A SATISFACTORY RECORD OF PREVIOUS INSTALLATIONS.

AGGREGATE: CRUSHED STONE, CRUSHED GRAVEL, AND SHARP-EDGED NATURAL SAND.

MAXIMUM AGGREGATE SIZE SHALL BE NO GREATER THAN ONE-HALF OF THE DESIGN THICKNESS OF THE WEARING OR BINDER COURSE.

SURFACE PREPARATION: PROOF ROLL PREPARED BASE SURFACE TO CHECK FOR UNSTABLE AREAS AND AREAS REQUIRING ADDITIONAL COMPACTION.

NOTIFY CONTRACTOR OF UNSATISFACTORY CONDITIONS. DO NOT BEGIN PAVING WORK UNTIL DEFICIENT BASE AREAS HAVE BEEN CORRECTED AND ARE READY TO RECEIVE PAVING.

PRIME COAT: APPLY AT RATE OF 0.2 TO 0.5 GAL. PER SQ. YD., OVER COMPACTED BASE. APPLY MATERIAL TO PENETRATE AND SEAL, BUT NOT FLOOD SURFACE. CURE AND DRY AS LONG AS NECESSARY TO ATTAIN PENETRATION AND EVAPORATION OF VOLATILE.

TACK COAT: APPLY TO CONTACT SURFACE OF PREVIOUSLY CONSTRUCTED ASPHALT OR PORTLAND CEMENT CONCRETE AND SURFACES ABUTTING OR PROJECTING INTO ASPHALT CONCRETE PAVEMENT. DISTRIBUTE AT RATE OF 0.05 TO 0.51 GAL. PER SQ. YD. OF SURFACE.

ALLOW TO DRY UNTIL AT PROPER CONDITION TO RECEIVE PAVING.

ASPHALT CONCRETE MIX: THIS ITEM SHALL CONSIST OF A WEARING SURFACE CONSTRUCTED OF ASPHALTIC CONCRETE ON A PREPARED BASE.

THE MATERIALS AND CONSTRUCTION METHODS SHALL COMPLY WITH THOSE SET FORTH FOR ASPHALTIC CONCRETE IN THE LATEST F.D.O.T. EDITION OF THE STANDARD SPECIFICATIONS.

THE ASPHALTIC CEMENT SHALL MEET THE REQUIREMENTS OF AASHTO SPECIFICATIONS M-20, VISCOSITY GRADE AC-20 (PENETRATION GRADE 60-70).

JOB MIX FORMULA: THE MARSHALL TESTING WILL BE USED IN ESTABLISHING THE JOB MIX FORMULA AND FOR CONTROL TESTING THROUGHOUT THE WORK.

THE DENSITY OF FIELD SAMPLES SHALL NOT BE LESS THAN 95% OF THE MARSHALL LABORATORY COMPACTED MIXTURE COMPOSED OF THE SAME MATERIALS IN LIKE PROPORTIONS.

THE THICKNESS OF THE SURFACE SHALL BE AS SPECIFIED IN THE SITE WORK PLANS. THIS REQUIREMENT SHALL BE CHECKED BY CORES AND WHERE A DEFICIENCY OF MORE THAN 1/4" EXISTS, THE CONTRACTOR SHALL BE REQUIRED TO CORRECT THE DEFICIENCY EITHER BY REPLACING THE FULL THICKNESS OR OVERLAYING THE AREAS TO THE SATISFACTION OF THE ENGINEER.

SAND ASPHALT BASE

PLACE ASPHALT CONCRETE MIXTURE ON PREPARED SURFACE, SPREAD AND STRIKE-OFF. SPREAD MIXTURE AT MINIMUM TEMPERATURE OF 225 DEGREES F. (107 DEGREES C).

PLACE IN STRIPS NOT LESS THAN 10' WIDE, UNLESS OTHERWISE ACCEPTABLE TO THE ENGINEER, AFTER FIRST STRIP HAS BEEN PLACED AND ROLLED, PLACE SUCCEEDING STRIPS AND EXTENDED ROLLING TO OVERLAP PREVIOUS STRIPS. COMPLETE BASE COURSE FOR A SECTION BEFORE PLACING SURFACE COURSE.

MAKE JOINTS BETWEEN OLD AND NEW PAVEMENTS, OR BETWEEN SUCCESSIVE DAYS' WORK, TO ENSURE CONTINUOUS BOND BETWEEN ADJOINING WORK. CLEAN CONTACT SURFACES AND APPLY TACK COAT.

BEGIN ROLLING WHEN MIXTURE WILL BEAR ROLLER WEIGHT WITHOUT EXCESSIVE DISPLACEMENT.

ACCOMPLISH BREAKDOWN OR INITIAL ROLLING IMMEDIATELY FOLLOWING ROLLING OF JOINTS AND OUTSIDE EDGE. CHECK SURFACE AFTER BREAKDOWN ROLLING, AND REPAIR DISPLACED AREAS BY LOOSENING AND FILLING, IF REQUIRED, WITH HOT MATERIAL. CONTINUE SECOND ROLLING UNTIL MIXTURE HAS BEEN THOROUGHLY COMPACTED.

PERFORM FINISH ROLLING WHILE MIXTURE IS STILL WARM ENOUGH FOR REMOVAL OF ROLLER MARKS. CONTINUE ROLLING UNTIL ROLLER MARKS ARE ELIMINATED AND COURSE HAS ATTAINED MAXIMUM DENSITY. AFTER FINAL ROLLING, DO NOT PERMIT VEHICULAR TRAFFIC ON PAVEMENT UNTIL IT HAS COOLED AND HARDENED. ERECT BARRICADES TO PROTECT PAVING FROM TRAFFIC UNTIL MIXTURE HAS COOLED ENOUGH NOT TO BECOME MARKED.

TEST IN-PLACE ASPHALT CONCRETE COURSES FOR PAVING AS DIRECTED BY ENGINEER FOR SMOOTHNESS.

THICKNESS: IN-PLACE COMPACTED THICKNESS WILL NOT BE ACCEPTABLE IF EXCEEDING FOLLOWING ALLOWABLE VARIATION FROM REQUIRED THICKNESS:

BASE COURSE: 1/2" PLUS OR MINUS
SURFACE COURSE: 1/4" PLUS OR MINUS

SURFACE SMOOTHNESS: TEST FINISHED SURFACE OF EACH ASPHALT CONCRETE COURSE FOR SMOOTHNESS, USING 10' STRAIGHT EDGE APPLIED PARALLEL WITH, AND AT RIGHT ANGLES TO CENTER OF PAVED AREAS. SURFACES WILL NOT BE ACCEPTABLE IF EXCEEDING THE FOLLOWING TOLERANCES FOR SMOOTHNESS:

BASE COURSE SURFACE: 1/4"
WEARING COURSE SURFACE: 3/16"
CHECK SURFACED AREAS AT INTERVALS AS DIRECTED BY ENGINEER.

FIELD DENSITY AND SOIL BEARING CAPACITY TESTS SHALL BE PERFORMED BY THE GEOTECHNICAL ENGINEER. PROVIDE INSPECTION, CERTIFICATION OF PAVEMENT CONSTRUCTION, FIELD TESTS AND CORE SAMPLES OF THE COMPLETE PAVEMENT CONSTRUCTION.

MISCELLANEOUS PAVEMENT

WORK INCLUDED CONSISTS OF BUT IS NOT LIMITED TO THE FOLLOWING:

CONCRETE SIDEWALKS, CURBS, CURB AND GUTTER, INCLUDING POROUS FILL.

CONCRETE LIGHT POLE BASES.

SUBMIT A "LETTER OF INTENT" FOR THE FOLLOWING:

CONCRETE MIX DESIGN.

THIS SUBCONTRACTOR SHALL WARRANT ALL ASPHALT PAVING AGAINST DEFECTS IN MATERIALS AND WORKMANSHIP FOR A PERIOD OF TWO (2) YEARS.

POROUS FILL SHALL BE CLEAN COARSE SAND, FREE DRAINING GRAVEL, OR CRUSHED ROCK ALL AS APPROVED BY THE GEOTECHNICAL ENGINEER.

POROUS FILL UNDER SIDEWALKS, ETC., SHALL BE GRADED BETWEEN 3/8" AND NO. 200 SIEVE.

POROUS FILL SHALL BE CAPABLE OF BEING COMPACTED TO NOT LESS THAN 95% OF THE MAXIMUM DRY DENSITY AS DETERMINED BY ASTM-1557.

STEEL REINFORCING BARS SHALL CONFORM TO "SPECIFICATIONS FOR DEFORMED BILLET STEEL BARS FOR CONCRETE REINFORCEMENT", ASTM A-615 GRADE NO. 60, HAVING A MINIMUM YIELD STRENGTH OF 60,000 PSI.

TIE WIRE SHALL BE BLACK ANNEALED WIRE, 16 GAUGE MINIMUM. BAR SUPPORTS SHALL CONFORM TO THE "BAR SUPPORT SPECIFICATIONS" CONTAINED IN "MANUAL OF STANDARD PRACTICE" AS PUBLISHED BY CRSI AND WCRSI. BAR SUPPORTS AND ACCESSORIES WITHIN 1/2" OF SURFACE OF CONCRETE EXPOSED TO WEATHER SHALL BE NON-CORROSIVE.

CEMENT SHALL BE GRAY PORTLAND CEMENT, TYPE I OR II, CONFORMING TO ASTM C-150 OR ASTM C-175 FOR AIR-ENTRAINING PORTLAND CEMENT.

CONCRETE AGGREGATES SHALL CONFORM TO ASTM C-33.

FINE AND COARSE AGGREGATES SHALL BE REGARDED AS SEPARATE INGREDIENTS AND EACH SHALL CONFORM TO THE APPROPRIATE GRADING REQUIREMENTS OF ASTM C-33.

AIR-ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C-260. EXPANSION JOINTS SHALL BE 1/2" THICK CANE FIBER EXPANSION JOINTS, CONFORMING TO ASTM D-1751.

EXPANSION JOINT SEALANT SHALL BE TRAFFIC GRADE, SELF LEVELING TREMCO THC-900" OR PERCORA CORPORATION "NF-200". COLOR SHALL BE BLACK. SHALL BE AS RECOMMENDED BY SEALANT MANUFACTURER.

CURING COMPOUND SHALL BE CLEAR, CONFORMING TO ASTM C-309. CURING COMPOUND SHALL BE COMPATIBLE WITH PAINTS, ETC., SCHEDULED OR SPECIFIED FOR APPLICATION TO CONCRETE SURFACE.

ALL CONCRETE, UNLESS OTHERWISE NOTED, SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS. MIX DESIGN SHALL BE SO PROPORTIONED TO PROVIDE A MINIMUM OF 517 POUNDS OF CEMENT PER CUBIC YARD.

ALL CONCRETE SHALL BE PROPORTIONED TO HAVE A SLUMP OF 4" MAXIMUM. TOLERANCE IN SLUMP SHALL NOT EXCEED ACI RECOMMENDATIONS.

READY-MIXED CONCRETE SHALL CONFORM TO ASTM C-94 AND THE NATIONAL READY MIX CONCRETE ASSOCIATION.

POROUS FILL SHALL BE LAID AND COMPACTED TO A MINIMUM DEPTH OF 3", UNLESS OTHERWISE INDICATED, UNDER ALL SIDEWALKS, ETC.

POROUS FILL SHALL BE COMPACTED TO NOT LESS THAN 95% MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-1557.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO SUPPLY ALL MATERIALS NECESSARY TO COMPLETE PAVING.

SITE IMPROVEMENTS

ALL OFF-SITE WORK INCLUDED CONSISTS OF BUT IS NOT LIMITED TO THE FOLLOWING:

SITE RELATED FENCING.
GUARD POSTS, GUARD RAIL AND POSTS AND SIGN POSTS LOCATED ON THE SITE.
TRAFFIC CONTROL SIGNS. GUARD POSTS,

GUARD RAIL AND POSTS AND SIGN POSTS:

STEEL SHAPES SHALL CONFORM TO ASTM A-36.
STEEL PIPE SHALL CONFORM TO ASTM A-53, E OR S, TYPE B.
STEEL PIPE SHALL CONFORM TO ASTM A-501.

SHOP COAT SHALL BE RUST INHIBITING RED OXIDE, RED LEAD OR LEAD CHROMATE OR EQUAL. IT IS THE INTENT TO PERMIT THE USE OF THE FABRICATORS STANDARD PRIME COATING.

ASPHALT BASED COATING IS NOT PERMITTED.

CONCRETE FOR SETTING FENCE AND GUARD RAIL POSTS AND SETTING AND FILLING OF SIGN AND GUARD POSTS SHALL BE PORTLAND CEMENT COMPLYING WITH ASTM C-150, AGGREGATES COMPLYING WITH ASTM C-33, AND CLEAN WATER. MIX MATERIALS TO OBTAIN CONCRETE WITH A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI, USING AT LEAST 4 SACKS OF CEMENT PER CU. YD., 1" MAXIMUM SIZE AGGREGATES, MAXIMUM 3" SLUMP, AND 2% TO 4% ENTRAINED AIR. PREPARE TO CONFORM TO ASTM C-94

MISCELLANEOUS NOTES

THE CONTRACTOR IS CAUTIONED TO VISIT THE SITE AND FAMILIARIZE HIMSELF WITH THE PROJECT PRIOR TO BIDDING.

THE ENGINEER HAS ATTEMPTED TO LOCATE EXISTING STRUCTURES AND EXISTING UTILITIES IN THE PROJECT AREA. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE THE EXACT LOCATIONS OF THESE STRUCTURES OR UTILITIES AND TO DETERMINE IF OTHER STRUCTURES OR UTILITIES WILL BE ENCOUNTERED DURING THE COURSE OF THE WORK. THE CONTRACTOR SHALL TAKE WHATEVER STEPS NECESSARY TO PROVIDE FOR THEIR PROTECTION AND RELOCATION OF UTILITIES IN CONFLICT WITH NEW CONSTRUCTION BY APPROPRIATE UTILITY COMPANY.

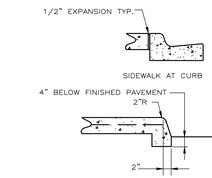
THE CONTRACTOR SHALL PLACE AND MAINTAIN ADEQUATE BARRICADES, CONSTRUCTION SIGNS, FLASHING LIGHTS, TORCHES, RED LANTERNS AND GUARDS DURING PROGRESS OF CONSTRUCTION WORK IN ACCORDANCE WITH STATE STANDARDS AND UNTIL IT IS SAFE FOR BOTH PEDESTRIAN AND VEHICULAR TRAFFIC.

CONTRACTOR IS RESPONSIBLE FOR REPLACING EXISTING SURROUNDINGS (I.E., ASPHALT, SIDEWALKS, CURBS, ETC.) THAT ARE DAMAGED DURING CONSTRUCTION. REPLACEMENT SHALL MATCH EXISTING.

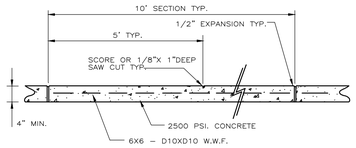
ALL SITE WORK MATERIALS AND CONSTRUCTION METHODS SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE FLORIDA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS.

CONTRACTOR SHALL HAVE ALL PERMITS PRIOR TO CONSTRUCTION IN WETLANDS, COUNTY RIGHT OF WAY, ETC.

CONSTRUCTION PLANS ARE BASED ON FIELD SURVEY AND OTHER DATA AS SHOWN. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY ALL LOCATIONS OF NEW AND EXISTING CONNECTIONS NECESSARY TO COMPLETE THE INTENT OF THE PLANS. IN THE EVENT THERE IS A CONFLICT DUE TO UNFORESEEN OBSTRUCTIONS OR SHORT FALLS TO CONNECTIONS (WHICH DOES NOT MEET THE INTENT OF THE CONSTRUCTION PLANS). THE CONTRACTOR SHALL CONTACT THE ENGINEER IMMEDIATELY FOR DIRECTION. THE CONTRACTOR SHALL RELOCATE OR REMOVE OBSTACLES AS DIRECTED BY OWNER.



SIDEWALK AT PAVEMENT



NOTE: MAX CROSS SLOPE OF 2.00%

CONCRETE SIDEWALK DETAIL
NOT TO SCALE

TESTING SCHEDULE

ITEM	TEST	TEST IDENTIFICATION	TEST REQUIREMENTS	TEST FREQUENCY
UTILITY TRENCH FILL & BACKFILL	MAXIMUM DENSITY OPTIMUM MOISTURE	AASHTO T-180 ASTM D-1557	N/A	PER SOIL TYPE
	FIELD DENSITY	AASHTO T-191, T-204 ASTM D-1556, D-2937	95% OF MAXIMUM DENSITY	ONE PER 500 LF HORIZONTAL OR ONE PER 750SY WITH A MINIMUM OF 3 TESTS, ALTERNATING LIFTS (12")
	GRADATION	AASHTO M-92	(15% PASSING NO. 200)	ONE PER SOIL TYPE
FILL & BACKFILL UNDER ROADWAYS AND STRUCTURES	MAXIMUM DENSITY OPTIMUM MOISTURE	AASHTO T-180 ASTM D-1557	N/A	PER SOIL TYPE
	FIELD DENSITY	AASHTO T-191, T-204, T-238 ASTM D-1556, D-2937, D-2922	98% OF MAXIMUM DENSITY	ONE PER 500 LF OR ONE PER 750SY WITH A MINIMUM OF 3 TESTS, ALTERNATING LIFTS (12")
	GRADATION	AASHTO M-92	(15% PASSING NO. 200)	ONE PER SOIL TYPE
SUBGRADE	BEARING VALUES	LBR-FDOT	40 (MIN.)	ONE PER SITE OR AT MATERIAL CHANGES
	MAXIMUM DENSITY OPTIMUM MOISTURE	AASHTO T-180 ASTM D-1557	N/A	PER SOIL TYPE
	FIELD DENSITY & THICKNESS	AASHTO T-191, T-204 ASTM D-1556, D-2937	98% OF MAXIMUM DENSITY	ONE PER 500 LF HORIZONTAL OR ONE PER 750SY WITH A MINIMUM OF 3 TESTS
BASE	MAXIMUM DENSITY OPTIMUM MOISTURE	AASHTO T-180 ASTM D-1557	N/A	ONE PER SOURCE OR AT MATERIAL CHANGES
	FIELD DENSITY & THICKNESS	AASHTO T-191, T-204 ASTM D-1556, D-2937	98% OF MAXIMUM DENSITY	ONE PER 500 LF HORIZONTAL OR ONE PER 750SY WITH A MINIMUM OF 3 TESTS
ASPHALT	MATERIALS QUALITY BITUMEN CONTENT & GRADATION	AASHTO T-164, T-30 ASTM D-2172	FDOT SPEC. 320, 330, 334 FDOT SPEC. 916	MIN. ONE PER DAY FOR GRADATION OR AS REQUIRED BY INSPECTOR
	FIELD DENSITY & THICKNESS	ASTM D-2950	95% OF LAB DENSITY	ONE PER 500 LF HORIZONTAL OR ONE PER 750SY WITH A MINIMUM OF 3 TESTS
CONCRETE (MISC. SITE WORK)	SLUMP TEST	AASHTO T-119 ASTM C-143	2" TO 3"	AS REQUIRED BY INSPECTOR OR ONE PER SET OF CYLINDERS
	COMPRESSIVE STRENGTH	AASHTO T-23 ASTM C-31	4000 PSI	ONE SET OF 3 CYLINDERS PER 50CY PER DAY
	AIR CONTENT	AASHTO T-199	3% TO 6%	ONE PER SET OF CYLINDERS

NOTE: 1. CONCRETE FOR SITE WORK INCLUDES BUT IS NOT LIMITED TO CURB, CURB & GUTTER, SIDEWALKS, CONCRETE PAVING, ETC.
2. THIS TEST SCHEDULE APPLIES TO SITE WORK ONLY. SEE ARCHITECT'S SPECIFICATIONS FOR FOUNDATION/BUILDING TESTING.

ROBERT L. CARROLL LICENSE No. 57988 STATE OF FLORIDA PROFESSIONAL ENGINEER

2/6/2026

Robert L. Carroll, P.E.
PROFESSIONAL ENGINEER
FL LC # 57988
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JRA ARCHITECTS
2211 NAVY BLVD. , STE 100
PANAMA CITY BEACH, FL
PHONE: (850) 236-9832
Commission Number: 25871

CONSULTANTS:

PROJECT:

BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD
HOUSE
MOSLEY HIGH SCHOOL

LYNN HAVEN, FLORIDA

SHEET TITLE:

CONSTRUCTION DETAILS

SHEET NUMBER:

C2.4

Addendum #2

Mosley Softball Fieldhouse

JRA Commission. 25871 CA/BC

Bay District Schools
1311 Balboa Avenue
Panama City, Florida 32401

JRA Architects, Inc.
2211 Navy Blvd- Suite 100
Panama City, Florida 32408

Date of Issue of addendum: **April 17, 2026**

The changes herewith form part of the Construction Documents and modify the original "Construction Documents" dated April 3, 2026.

This Addendum consists of **1** page(s), 1 attachment(s) and **2** revised plan sheet(s) attached and as referenced herein.

ITEM 2.1

Drawings: Refer to **Drawings sheet A1.1 Architectural Floor Plan** – The hollow cells of the 8" CMU walls are to be filled with spray foam insulation as noted on the **Architectural wall types** on the index sheet.

ITEM 2.2

Geotech Report: Refer to **Attachment "A" - NOVA Geotechnical Engineering Report** dated January 20, 2026.

ITEM 2.3

Drawings: Replace **Drawings sheets P1.1 Plumbing New work Floor Plans and P2.1 Plumbing Riser Diagrams** from the contract documents with the Attached Drawing Sheets P1.1 & P2.1with a revision date of 4/17/26.

END OF ADDENDUM #2

Attachment "A"

GEOTECHNICAL ENGINEERING REPORT

MOSLEY HS SOFTBALL FIELDHOUSE

Lynn Haven, Bay County, Florida

PREPARED FOR:

JRA ARCHITECTS, INC.
2211 Thomas Drive, Suite 100
Panama City Beach, Florida

NOVA Project Number: 10111-2026005

March 6, 2026

NOVA
ENGINEERING AND ENVIRONMENTAL

March 6, 2026

Mr. Dave Vincent, AIA, President
JRA ARCHITECTS, INC.
2211 Thomas Drive, Suite 100
Panama City Beach, Florida

Subject: Geotechnical Engineering Report
MOSLEY HS SOFTBALL FIELDHOUSE
Lynn Haven, Bay County, Florida
NOVA Project Number 10111-2026005

Dear Mr. Vincent:

NOVA ENGINEERING AND ENVIRONMENTAL LLC (NOVA) has completed the authorized subsurface exploration and geotechnical engineering evaluation for the proposed site improvements located in Lynn Haven, Bay County, Florida. Our services were performed in general accordance with NOVA Proposal Number 10111-2026005, dated January 26th, 2026. This report briefly discusses our understanding of the project at the time of the subsurface exploration, describes the geotechnical consulting services provided by NOVA, and presents our findings, conclusions, and recommendations.

We appreciate your selection of NOVA and the opportunity to be of service on this project. If you have any questions, or if we may be of further assistance, please do not hesitate to contact us.

Sincerely,
NOVA ENGINEERING AND ENVIRONMENTAL, LLC

Michael R. Keramidas
Staff Engineer

Daniel C. Brahana, P.E. (Georgia)
Geotechnical Service Line Leader

Jesse T. Strickland, P.E.
Senior Engineer
Florida Registration No. 74564

JESSE T. STRICKLAND LICENSE No 74564 STATE OF FLORIDA PROFESSIONAL ENGINEER

This item has been electronically signed and sealed by Jesse T. Strickland, P.E., using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Digitally signed by jesse t strickland
Date: 2026.03.06 15:28:55 -05'00'

Copies Submitted: Addressee (electronic)

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APPENDICES

1.0 INTRODUCTION

Our services for this project were performed as described in our Proposal Number 10111-2026005, authorized by the Client on January 28th, 2026. The primary objectives were to perform a geotechnical exploration within the proposed construction areas of the project site and to assess these findings as they relate to geotechnical aspects of the planned site development. The authorized geotechnical engineering services included a site reconnaissance, soil test borings and sampling, engineering evaluation of the field and laboratory data, and the preparation of this report. As authorized in the above-mentioned proposal, this geotechnical report includes:

- A description of the site, fieldwork, laboratory testing and general soil conditions encountered, together with a Boring Location Plan, and individual Test Boring Records.
- Site preparation considerations that include geotechnical discussions regarding site stripping and subgrade preparation and engineered fill/backfill placement.
- Recommendations for controlling groundwater and/or run-off during construction.
- Shallow foundation system recommendations for the proposed structures, including allowable foundation capacities, recommended bearing depths, and installation considerations
- Slab-on-grade construction considerations based on the geotechnical findings, including the need for a sub-slab vapor barrier or a capillary barrier.
- Geotechnical soil input parameters for SMS design.
- Suitability of on-site soils for re-use as structural fill and backfill and the criteria for suitable fill materials.
- Recommended quality control measures (i.e., sampling, testing, and inspection requirements) for site grading and foundation construction operations.

The assessment of the presence of wetlands, floodplains, or water classified as state waters was beyond the scope of this exploration. Additionally, the assessment of site environmental conditions, including the detection of pollutants in the soil, rock, or groundwater or a site-specific seismic and/or karst-related study are also beyond the scope of this geotechnical exploration and evaluation. If requested, NOVA can provide these services under a supplemental study at a later date.

2.0 PROJECT INFORMATION

Our understanding of this project is based on discussions with the Client, a review of the provided site plan, site reconnaissance during the boring layout and testing process, and our experience with similar projects.

2.1 NAME AND LOCATION OF PROJECT

The +/- ½-acre Subject Property (*Bay County Parcel ID# 11800-020-000*) is located at 501 Mosley Drive, in Lynn Haven, Bay County, Florida. The location of the Subject Property (project site) is indicated on the Site Location Map included in Appendix A.

2.2 PROJECT SITE

At the time of our services, the project site was an existing school facility, with the specific area of testing consisting of a sports field complex, with surface conditions consisting primarily of short grasses. The surrounding area is generally developed with a softball field to the southwest, a baseball field to the northeast and short grass fields to the northwest and southeast. The site topography was observed to be generally flat. Based on a review of ground surface elevation data obtained from publicly available GIS-database, the existing ground surface elevation is reported to be on the order of approximately EL +36 feet Earth Gravitational Model of 1996 (EGM96). EGM96 is a global ellipsoidal model used to approximate the shape of the Earth and should be considered as approximate.

2.3 PROPOSED DEVELOPMENT

NOVA understands the project includes the construction of an approximate 2,600 ft^2 single-story fieldhouse structure, and an associated stormwater management system (SMS) to treat and dispose of stormwater runoff related with the site improvements. The structure is anticipated to be of CMU construction with light gauge metal roofs trusses with a slab-on-grade and supported by a conventional shallow foundation system. The SMS is anticipated to be desired to consist of a conventional dry retention basin.

Maximium Loads & Site Grading

Final structural loading information was not available from the Design Team at the time of this report. We have therefore assumed that maximum loadings will not exceed 1 to 2 kips per linear foot for continuous load bearing wall footings and 40 kips for isolated column footings. Finished grades are anticipated to match existing site grades, with any cut or fill limited to 2 feet or less. The SMS basin is anticipated to be 3 feet or less in depth relative to existing site grades.

3.0 SUBSURFACE EXPLORATION

3.1 AREA GEOLOGY

The subject site is located in Bay County, and according to the United States Geological Survey (USGS), within the Gulf Coastal Plain which is separated from the Florida Platform by geologic structures known as the Gulf Trough and Apalachicola Embayment. These structures formed a bathymetric and environmental barrier from the earliest Eocene or earliest Oligocene periods into the Miocene. This physiographic region is generally underlain by undifferentiated sediments deposited during the Quaternary period. These sediments typically consist of siliciclastics (sand), organics and freshwater carbonates. These soils are highly permeable and form the Sand and Gravel Aquifer of the surficial aquifer system. Surficial soils in the region are primarily siliciclastic sediments deposited in response to the renewed uplift and erosion in the Appalachian highlands to the north and sea-level fluctuations. The extent and type of deposit is influenced by numerous factors, including mineral composition of the parent rock and meteorological events.

3.2 FIELD EXPLORATION

Our subsurface exploration was conducted on February 5th and February 6th, 2026, and included performing:

- One 50-foot-deep SPT boring and one 20-foot-deep SPT boring at the locations provided by the Client within the footprint of the proposed structure (designated as B-1 & B-2, respectively, in the Appendix).
- Two hand auger borings at the locations provided by the Client, and collection of one bulk sample from the top 3 feet within the footprint of the proposed SMS basin (S-1 & S-2).

The boring and test locations were established in the field by NOVA personnel using a handheld GPS device and by estimating distances and angles from site landmarks. Prior to initiating field testing, underground utilities were marked by the state utility locate service (811, or Sunshine One-Call). The approximate test locations are shown in the Boring Location Plan provided in Appendix B. If increased accuracy is desired by the Client, test locations and elevations may be surveyed.

The Test Boring Records in Appendix B show the Standard Penetration Test (SPT) resistances, or “N-values”, for the structure borings and present the soil conditions encountered in all of the borings. These records represent our interpretation of the subsurface conditions based on the field exploration data, visual examination of the split-barrel samples, laboratory test data, and generally accepted geotechnical

engineering practices. The stratification lines and depth designations represent approximate boundaries between various subsurface strata. Actual transitions between materials may be gradual.

The groundwater levels reported on the Test Boring Records represent measurements made at the completion of each test boring, following a suitable stabilization period. The test borings were subsequently backfilled with soil cuttings from the drilling process for safety concerns.

SPT Borings

The SPT borings were performed using the guidelines of ASTM Designation D-1586, "Penetration Test and Split-Barrel Sampling of Soils". A mud-rotary drilling process was used to advance the borings once groundwater was encountered. At regular intervals, soil samples were obtained with a standard 1.4-inch I.D., 2.0-inch O.D., split-tube sampler. The sampler was first seated six inches and then driven an additional foot with blows of a 140-pound *safety-hammer* falling 30 inches. The number of hammer blows required to drive the sampler the final foot is designated the "Penetration Resistance". Representative portions of the soil samples, obtained from the sampler, were placed in sealed containers and transported to our laboratory for further evaluation and laboratory testing.

Auger Borings

The auger borings were performed using the guidelines of ASTM Designation D-1452, “Soil Exploration and Sampling by Auger Borings". A manually advanced 3¼-inch diameter orchard-type auger was utilized with disturbed samples acquired continuously for the full depth of the boring. Representative portions of the soil samples, obtained from the sampler, were placed in sealed containers and transported to our laboratory for further evaluation and laboratory testing.

3.3 LABORATORY TESTING

Following completion of the field work, soil and rock samples obtained in the field were returned to our office for classification and laboratory testing assignment. These tests included the following:

- Moisture Content
- Fines Content
- Organic Content
- Re-molded Falling Head Permeability

The purpose of the testing program was to classify the subsurface materials relative to the Unified Classification System and to determine various physical characteristics of the soils tested. The results of the laboratory testing are in Appendix C.

3.4 SUBSURFACE CONDITIONS

The following paragraphs provide generalized descriptions of the subsurface profiles and soil conditions encountered within the test borings conducted during this exploration.

SPT Boring

The SPT test borings generally encountered loose to medium dense slightly silty fine sands to silty fine sands (USCS classifications of SP-SM and SM) to the maximum depths explored ranging from approximately 20 to 50 feet BEG. Standard Penetration Test (SPT) resistance values ("N"-values) generally ranged from 4 to 12 blows-per-foot (bpf) to a depth of 10 feet BEG and from 11 to 22 bpf until the maximum termination depth of approximately 50 feet BEG.

Hand Auger Borings

The hand auger test borings generally encountered mixed strata of slightly silty fine sands to silty fine sands (SP-SM and SM) to the maximum depths explored of approximately 4 feet BEG. We note that the hand auger borings were terminated prior to the planned termination depth due to groundwater intrusion causing cave-in of the boreholes.

Groundwater Conditions

Groundwater in the Bay County, Florida area typically occurs as an unconfined aquifer condition. Consequently, the groundwater table is expected to be a subdued replica of the original surface topography. Recharge is provided by the infiltration of rainfall and surface water through the soil overburden. More permeable zones in the soil matrix can affect groundwater conditions. Groundwater levels vary with changes in season and rainfall, construction activity, surface water runoff, and other site-specific factors. Groundwater levels in the Bay County area are typically lowest in the late fall to winter and highest in the early spring to mid-summer with annual groundwater fluctuations by seasonal rainfall; consequently, the water table may vary at times.

A stabilized groundwater table was encountered in the test borings at depths ranging from approximately 2 feet to 3 feet BEG at the time of our final field exploration, which occurred during a period of relatively normal seasonal rainfall. Based on comparisons of current annual monthly rainfall data to historical rainfall data extending back 50+ years in time, groundwater levels encountered during our subsurface exploration presented herein, and a review of published historical soils and groundwater in the project vicinity, we estimate that the normal permanent seasonal high groundwater (SHGW) table for this site will occur within 1-foot of the natural ground surface elevation during the wet season.

4.0 GEOTECHNICAL ASSESSMENT

The following assessment is based on our understanding of the proposed construction, our site observations, our evaluation and interpretation of the field data obtained during this exploration, our experience with similar subsurface conditions, and generally accepted geotechnical engineering principles and practices.

Based on the results of the test borings, the encountered subsurface conditions appear to be adaptable for supporting the proposed construction utilizing conventional shallow foundation systems, provided that the recommendations outlined in the following sections are appropriately implemented. However, due to the relatively near-surface groundwater levels reported at the site and encountered during the subsurface exploration presented herein, we anticipate a wet detention system will be required for the proposed SMS basin.

Subsurface conditions in unexplored locations can and will vary from those encountered at the boring locations considered and discussed herein. If such variations are noted during construction, or if project development plans are changed, we request the opportunity to review the changes and amend our recommendations, if necessary.

The following sections present our recommendations for site preparation and grading, and for the design of the proposed shallow foundation system, and the SMS.

5.0 RECOMMENDATIONS

5.1 SITE PREPARATION

Prior to proceeding with construction, all topsoil and vegetation, and any other deleterious non-soil materials found to be present should be stripped from the proposed construction areas). Clean topsoil may be stockpiled and subsequently re-used in landscaped areas to receive sod as a "finishing layer". Debris-laden materials should be excavated, transported, and disposed of off-site in accordance with appropriate solid waste rules and regulations. Any existing utility locations should be reviewed to assess their impact on the proposed construction and relocated/grouted in-place as appropriate.

The site should be graded during construction to maintain positive drainage away from the construction areas, to prevent ponding of stormwater on the site during and shortly following significant rain events. The construction areas should be sealed and crowned with a smooth roller to minimize ponding water from storm events at the end of each day of work.

The soils exposed at the stripped grade elevation should be compacted to a minimum soil density of at least 95 percent of the maximum dry density as determined by the Modified Proctor test method (ASTM D-1557). The upper 12 inches of soil beneath all footing excavation areas should be compacted to at least 98 percent.

NOVA should observe the compaction (i.e. proofrolling) of the subgrade to locate soft, weak, or excessively wet fill or existing soils present at the time of construction. Any unstable materials observed during the evaluation and compaction operations should be undercut and replaced with structural fill or stabilized in-place by scarifying and re-densifying. Actual remedial recommendations can best be determined by the geotechnical engineer in the field at the time of construction.

5.2 EXCAVATION

Excavations greater than five feet deep (such as for deeper foundations and underground utilities) should be sloped or shored in accordance with local, state, and federal regulations, including OSHA (29CFR Part 1926), excavation safety standards. It should be noted that the Contractor is solely responsible for site safety. This information is provided only as a service and under no circumstances should NOVA be assumed to be responsible for construction site safety. Each excavation should be observed and classified by an OSHA-competent person. All excavations below the groundwater level are classified as OSHA Class C soils for excavation purposes.

5.3 GROUNDWATER CONTROL

A stabilized groundwater table was encountered in the test borings at depths ranging from approximately 2 feet to 3 feet BEG at the time of our final field exploration, which occurred during a period of relatively normal seasonal rainfall. We recommend that measures be taken to reduce infiltration of water into the building foundation areas as recommended in *Section 5.4 – Drainage Considerations*.

Maintaining proper grades (i.e., positive drainage paths) during the construction phase of this project will be critical to avoid the development of "bird baths" within the proposed structure footprint area, which would degrade the underlying silty fine sand soils.

Additionally, the time frame that excavations needed to install building foundations and retaining walls remain open to inclement weather should be kept as short as practical, as water ponding in these excavations will degrade the underlying soil profile which could subsequently require remediation including possibly over-excavating pumping or otherwise overly wet soils. Sheet flowing stormwater exiting the overall site should be prevented from entering these excavations, and a pump system should be engaged immediately following the passing of rain events to minimize the time that standing water would be ponding on exposed soils.

Should elevated groundwater conditions be encountered during the earthwork phase of this development, most likely localized dewatering efforts (e.g., construction ditches, temporary sumps, etc.) will suffice to allow for earthwork operations to be performed in the dry. Given the anticipated fill heights of 3 feet above existing grades within the low-lying areas of the site, permanent dewatering measures are not anticipated as being necessary for this development

Temporary Dewatering: Design of a temporary dewatering system is usually the responsibility of the contractor. However, based on our experience with similar conditions, we anticipate that a well point dewatering system could potentially be necessary to properly control groundwater if deeper excavations are required. Since well point system construction is partially dependent on the technique and experience of the contractor, we believe the system should only be installed by a contractor with a minimum of 5 years of experience in dewatering system design and installation. Design flexibility should be allowed for the specialty contractor to choose the most appropriate dewatering system, including the well depths and spacing.

At the time of construction, groundwater must be lowered and continuously maintained at a minimum depth of 2 feet below the desired working elevation to permit subgrade preparation and foundation excavation and construction. The dewatering system should be installed and operational prior to excavation beneath the water table. The

dewatering system should also remain in continuous operation until the foundation concrete installation is complete.

Although dewatered, the subgrade will remain saturated and susceptible to damage caused by construction traffic. Undercutting, stabilization, and replacement of unstable soils should be assumed to be necessary. As previously noted, groundwater levels are subject to seasonal, climatic, tidal and other variations and may be different at other times and locations. The extent and nature of any dewatering required during construction will be dependent on the actual groundwater conditions prevalent at the time of construction and the effectiveness of construction drainage to prevent run-off into open excavations.

5.4 DRAINAGE CONSIDERATIONS

Soil strength and settlement potential is highly dependent upon the moisture condition of the supportive soil. Soil characteristics can change dramatically when moisture conditions change. As such, building pads, pavements, structures and surrounding grades should be properly designed and constructed to properly control water (surface and subsurface). Building pads should be designed to shed surface water prior to building construction. Grades surrounding structures should be adequately sloped away from the structure to promote positive drainage and prevent water from ponding near or against the structure. Swales and/or storm drainage structures should be constructed to collect and remove all surface water run-off. All roof drain downspouts should be connected to drain leaders that are properly daylighted or connected to storm drainage structures such that water is removed from structural areas. Interior and/or exterior foundation drains, if provided, should be installed to properly protect foundations from changing moisture conditions.

5.5 FILL PLACEMENT

Fill Suitability

Fill materials should be relatively clean sands with less than 20 percent fines (material passing the No. 200 sieve) and a Plasticity Index (PI) of less than 15, and free of non-soil materials and rock fragments larger than 3 inches in diameter. We note that soils with fines contents between approximately 15 and 20 percent will require strict moisture control at the time of placement and the Contractor should be prepared for difficulties during placement and compaction operations due to their moisture sensitivity.

Prior to construction, bulk samples of all proposed fill materials (both native and import) should be laboratory tested to confirm their suitability. Based on visual examination and limited laboratory soil testing results, the in-situ soils encountered

during this exploration typically consisted of slightly silty fine sands and silty fine sands (SP-SM and SM) that should be suitable for re-use as fill or backfill material, as laboratory testing indicated fines contents to range from approximately 7 to 18 percent.

We recommend that stockpiles of all materials planned for re-use be sealed as they are excavated to prevent (to the greatest extent practical) the intrusion of moisture into the core of the soil stockpile(s) during significant rain events prior to their potential re-use as fill and/or backfill soils.

Topsoil can be wasted in architectural areas to receive sod as a "finishing layer". Organic and/or debris-laden material is not suitable for re-use as structural fill and should be excavated, transported, and disposed of off-site in accordance with appropriate solid waste rules and regulations.

Soil Compaction

All structural fill should be placed in thin, horizontal loose lifts (maximum 12-inch) and compacted to a minimum soil density of at least 95 percent of the Modified Proctor maximum dry density (ASTM D-1557). The upper 12 inches of soil beneath all footing excavation areas should be compacted to at least 98 percent. In confined areas, such as utility trenches, portable compaction equipment and thinner fill lifts (3 to 4 inches) may be necessary.

Fill materials used in structural areas should have a target maximum dry density of at least 95 pounds per cubic foot (pcf). If lighter weight fill materials are used, the NOVA geotechnical engineer should be consulted to assess the impact on design recommendations.

Soil moisture content should be maintained within 3 percent of the optimum moisture content. We recommend that the grading contractor have equipment on site during earthwork for both drying and wetting fill soils. Moisture control may be difficult during rainy weather. Soils excavated from below the groundwater table will likely require significant efforts to achieve acceptable moisture contents prior to re-use as fill.

Filling operations should be observed by a NOVA soils technician, who can confirm suitability of material used and uniformity and appropriateness of compaction efforts. The technician can also document compliance with the specifications by performing field density tests using thin-walled tube, nuclear, or sand cone testing methods (ASTM D-2937, D-6938, or D-1556, respectively). One test per 2,500 square feet in structure areas should be performed at the stripped subgrade elevation as well as in each lift of fill, with test locations well distributed throughout construction footprint. When filling in small areas, at least one test per day per area should be performed. One test at

conventional spread foundations and one test per 50 linear feet of continuous strip foundations are also recommended.

5.6 SHALLOW FOUNDATIONS

Design: After the recommended site and subgrade preparation and fill placement has been completed, it is our opinion that conventional shallow foundations can be used to support the proposed structure. Foundations bearing on densified native in-situ soils, and/or properly placed and compacted structural fill, as recommended in this report, may be designed employing a maximum allowable soil bearing pressure of **2,000 pounds per square foot (psf)**.

We recommend a value of 0.35 can be employed as the coefficient of friction (sliding resistance) between foundations and the underlying residual or fill soils. Footings should be a minimum of 24 inches in width for ease of construction and to reduce the possibility of localized shear failures. Isolated exterior and interior footing bottoms should be established at least 18 inches below finished surrounding exterior grades. When utilizing a post-tensioned monolithic slab-on-grade design, exterior and interior footing bottoms should be established a minimum of 12 inches below adjacent finished grades.

Settlement: Settlements for conventional foundations bearing on improved subgrade soils were assessed using SPT values to estimate elastic modulus, based on published correlations and previous NOVA experience. We note that the settlements presented are based on a subsoil profile deemed representative of the subsurface conditions encountered in the test borings. Conditions may be better or worse in other areas; however, we believe the estimated settlements stated herein are reasonable, but if subsurface conditions at the time of construction are determined to be different from those described herein, then NOVA should be retained to evaluate the differing conditions.

Based on the assumed structural loadings, the soil bearing capacity provided above, the presumed foundation elevations as discussed above, and successful completion of the site preparations provided herein, we expect residual primary total settlement beneath individual foundations to be on the order of 1-inch or less.

The amount of differential settlement is difficult to predict because the subsurface and foundation loading conditions can vary considerably across the site. However, we anticipate residual differential settlement between adjacent foundations will be less than ½-inch. The final deflected shape of the structure will be dependent on actual foundation locations and loading.

Construction: Foundation excavations should be evaluated by the NOVA geotechnical engineer prior to reinforcing steel placement to observe foundation subgrade preparation and confirm bearing pressure capacity. Foundation excavations should be level and free of debris, ponded water, mud, and loose, frozen, or water-softened soils. Concrete should be placed as soon as is practical after the foundation is excavated and the subgrade evaluated. Foundation concrete should not be placed on frozen or saturated soil.

If a foundation excavation remains open overnight, or if precipitation is imminent, a 3 to 4-inch thick "mud mat" of lean concrete should be placed in the bottom of the excavation to protect the bearing soils until reinforcing steel and concrete can be placed.

5.7 SLABS-ON-GRADE

General

The conditions exposed at subgrade levels will vary across the site and may include structural fill or densified in-situ soils. Slab(s)-on-grade may be adequately supported on these subgrade conditions subject to the recommendations in this report. The slab-on-grade should be jointed around columns and along walls to reduce cracking due to differential movement. An underdrain system is not necessary beneath the slab, provided that the slab is installed at least 2 feet above the seasonal high groundwater level. An impermeable vapor barrier is recommended beneath finished spaces to reduce dampness.

A coefficient of subgrade reaction (***k***) of 125 pci (psi per inch) may be used for conventional slab design where slabs bear upon subgrades prepared in accordance with previous recommendations. Once grading is completed, the subgrade can be exposed to adverse construction activities and weather conditions during the period of sub-slab utility installation. The subgrade should be well drained to prevent the accumulation of water. If the exposed subgrade becomes unstable, excessively wet or exhibits excessive rutting or pumping, the geotechnical engineer should be consulted.

5.8 STORMWATER MANAGEMENT SYSTEM

Based on the results of the SMS test borings, the subsurface conditions encountered beneath the proposed SMS site appear poorly suited for the construction of the desired dry retention basin. Due to the relatively near-surface groundwater levels encountered during our subsurface exploration presented herein and a review of published historical soils and groundwater in the project vicinity, we recommend a wet retention basin alternative is utilized for the site improvements. Recommended soil input parameters for the SMS basin design are presented in the table below:

SMS Soil Design Parameters	
Corresponding Soil Boring Depths from Boring S-2	0' – 3'
Estimated Depth to Confining Stratum, BEG	Below 10 feet
Measured Vertical Permeability Rate (k_v)*	1 feet/day
Calculated Horizontal Permeability Rate (k_h)*	2 feet/day
Measured In-Situ Infiltration Rate*	<0.1 inches/hr.
Measured Depth to Apparent Groundwater Table, BEG	2 feet
Estimated Depth to Normal Permanent SHGW table, BEG	1-foot

** Factors of Safety have not been applied to the hydraulic conductivity and infiltration values.*

General Considerations

The seasonal high groundwater level estimate provided in the table above is based on our experience with projects in this locale, the soil strata and groundwater levels encountered in our test borings, and a review of publicly available historical data. Should these estimates become critical to design, then as previously stated, we recommend the locations and associated ground surface elevations are determined by a licensed and registered land surveyor.

The actual exfiltration rate from the pond may be influenced by basin geometry, natural soil variability, in-situ depositional characteristics and soil density, retention volume, and groundwater mounding effects. Appropriate factors of safety should be incorporated into the design process. We note that NOVA performs remolded laboratory permeability testing using generally accepted practices of the local engineering community. These types of tests are the quickest and most economical for stormwater retention basin design. However, the user of this information is cautioned that the potential variability of results of these types of tests can be significant, and the reproducibility of results can vary by factors of up to 100 percent.

Also, the permeability rate measured by such tests may not be representative of the total effective aquifer thickness. Factors of safety can compensate for part of the inherent test limitations, but the designer must exercise judgment regarding final selection and applicability of provided soil design input parameters. Should the modeling analysis indicate marginally acceptable compliance with Water Management District design criteria, it may be advisable to perform more extensive and representative in-situ permeability testing by collecting "undisturbed" horizontal and vertical soil samples and/or installing grouted piezometers or wells for slug testing. NOVA can perform these field tests under separate scope & fee, if desired at a later date.

6.0 LIMITATIONS

The findings, conclusions and recommendations presented in this report represent our professional opinions concerning subsurface conditions at the site. The opinions presented are relative to the dates of our site work and should not be relied on to represent conditions at significantly later dates or at locations not explored. The opinions included herein are based on information provided to us, the data obtained at specific locations during the study and our experience. If additional information becomes available that might impact our geotechnical opinions, it will be necessary for NOVA to review the information, reassess the potential concerns, and re-evaluate our conclusions and recommendations.

Regardless of the thoroughness of a geotechnical exploration, there is the possibility that conditions between test locations will differ from those encountered at specific test locations, that conditions are not as anticipated by the designers and/or the contractors, or that either natural events or the construction process have altered the subsurface conditions. These variations are an inherent risk associated with subsurface conditions in this region and the approximate methods used to obtain the data. These variations may not be apparent until construction.

This report is intended for the sole use of **JRA ARCHITECTS, INC.** for the above noted project. The scope of work performed during this study may not satisfy other user's requirements. Use of this report or the findings, conclusions or recommendations by others will be at the sole risk of the user. NOVA is not responsible or liable for the interpretation by others of the data in this report, nor their conclusions, recommendations or opinions.

Our professional services have been performed, our findings obtained, our conclusions derived and our recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices in the State of Florida. This warranty is in lieu of all other statements or warranties, either expressed or implied.

APPENDIX A
Figures and Maps

Base map provided by ***Google Earth***

Scale: Not To Scale	
Date Drawn: February 10, 2026	
Drawn By: B. Hardtke	
Checked By: J. Strickland	

NOVA

17612 Ashley Drive
Panama City Beach, Florida 32413
850.249.6682 ♦ 850.249.6683

PROJECT LOCATION MAP

Mosley HS Softball Fieldhouse

Lynn Haven, Bay County, Florida

NOVA Project Number 10111-2026005

Soil Map—Bay County, Florida
(Mosley HS Softball Field House)

Map Scale: 1:362 if printed on A portrait (8.5" x 11") sheet.

N

Meters
0 5 10 20 30

Feet
0 15 30 60 90

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 16N WGS84

MAP LEGEND

Area of Interest (AOI)
- Area of Interest (AOI)

Soils
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features
- Streams and Canals

Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background
- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Bay County, Florida
Survey Area Data: Version 25, Aug 27, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jan 10, 2024—Jan 20, 2024

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
22	Pamlico-Dorovan complex	0.2	31.5%
32	Plummer sand	0.5	68.5%
Totals for Area of Interest		**0.7**	**100.0%**

Bay County, Florida

32—Plummer sand

Map Unit Setting

National map unit symbol: brv2
Landscape: Coastal plains
Elevation: 0 to 450 feet
Mean annual precipitation: 61 to 69 inches
Mean annual air temperature: 63 to 70 degrees F
Frost-free period: 252 to 282 days
Farmland classification: Not prime farmland

Map Unit Composition

Plummer and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Plummer

Setting

Landscape: Coastal plains
Landform: Flats on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Sandy and loamy marine deposits

Typical profile

A - 0 to 7 inches: sand
Eg - 7 to 48 inches: sand
Btg1 - 48 to 59 inches: sandy loam
Btg2 - 59 to 80 inches: sandy clay loam

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: About 0 to 6 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum: 4.0
Available water supply, 0 to 60 inches: Moderate (about 6.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4w
Hydrologic Soil Group: A/D
Ecological site: F152AY320FL - East Central Sandy Flat
Forage suitability group: Sandy soils on flats of mesic or hydric lowlands (G152AA141FL)
Other vegetative classification: Sandy soils on flats of mesic or hydric lowlands (G152AA141FL)
Hydric soil rating: Yes

Minor Components

Pelham, hydric

Percent of map unit: 4 percent
Landscape: Coastal plains
Landform: Flats on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: F152AY335FL - East Central Sandy Flooded Flat
Other vegetative classification: Sandy over loamy soils on stream terraces, flood plains, or in depressions (G152AA245FL)
Hydric soil rating: Yes

Albany

Percent of map unit: 3 percent
Landscape: Coastal plains
Landform: Knolls on marine terraces, Rises on marine terraces, Flats on marine terraces
Landform position (three-dimensional): Interfluve
Down-slope shape: Convex
Across-slope shape: Linear
Ecological site: F152AY320FL - East Central Sandy Flat
Other vegetative classification: Sandy soils on rises and knolls of mesic uplands (G152AA131FL)
Hydric soil rating: No

Rutlege

Percent of map unit: 3 percent
Landscape: Coastal plains
Landform: Depressions on marine terraces
Landform position (three-dimensional): Dip
Down-slope shape: Concave
Across-slope shape: Concave
Ecological site: F152AY350FL - East Central Sandy Lowland
Other vegetative classification: Sandy soils on stream terraces, flood plains, or in depressions (G152AA145FL)
Hydric soil rating: Yes

Pottsburg, hydric

Percent of map unit: 3 percent
Landscape: Coastal plains

Landform: Flats on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: F152AY320FL - East Central Sandy Flat
Other vegetative classification: Sandy soils on flats of mesic or hydric lowlands (G152AA141FL)
Hydric soil rating: Yes

Rains

Percent of map unit: 2 percent
Landscape: Coastal plains
Landform: Flats on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: F152AY320FL - East Central Sandy Flat
Other vegetative classification: Loamy and clayey soils on flats of hydric or mesic lowlands (G152AA341FL)
Hydric soil rating: Yes

Data Source Information

Soil Survey Area: Bay County, Florida
Survey Area Data: Version 25, Aug 27, 2025

Bay County, Florida

22—Pamlico-Dorovan complex

Map Unit Setting

National map unit symbol: brtq
Landscape: Coastal plains
Elevation: 0 to 450 feet
Mean annual precipitation: 61 to 69 inches
Mean annual air temperature: 63 to 70 degrees F
Frost-free period: 252 to 282 days
Farmland classification: Not prime farmland

Map Unit Composition

Pamlico and similar soils: 40 percent
Dorovan and similar soils: 35 percent
Minor components: 25 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Pamlico

Setting

Landscape: Coastal plains
Landform: Flood plains on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Concave
Parent material: Herbaceous organic material over sandy marine deposits

Typical profile

Oa - 0 to 32 inches: muck
2Cg - 32 to 72 inches: sand

Properties and qualities

Slope: 0 to 1 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Very poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 6.00 in/hr)
Depth to water table: About 0 to 12 inches
Frequency of flooding: Frequent
Frequency of ponding: Frequent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum: 4.0
Available water supply, 0 to 60 inches: High (about 9.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7w
Hydrologic Soil Group: A/D
Forage suitability group: Organic soils in depressions and on flood plains (G152AA645FL)
Other vegetative classification: Organic soils in depressions and on flood plains (G152AA645FL)
Hydric soil rating: Yes

Description of Dorovan

Setting

Landscape: Coastal plains
Landform: Flood plains on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Organic material

Typical profile

Oa - 0 to 60 inches: muck
2Cg - 60 to 80 inches: sand

Properties and qualities

Slope: 0 to 1 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Very poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: About 0 to 6 inches
Frequency of flooding: Frequent
Frequency of ponding: Frequent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum: 4.0
Available water supply, 0 to 60 inches: Very high (about 22.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7w
Hydrologic Soil Group: B/D
Forage suitability group: Organic soils in depressions and on flood plains (G152AA645FL)
Other vegetative classification: Organic soils in depressions and on flood plains (G152AA645FL)
Hydric soil rating: Yes

Minor Components

Rutlege

Percent of map unit: 10 percent
Landscape: Coastal plains
Landform: Depressions on marine terraces

Landform position (three-dimensional): Dip
Down-slope shape: Concave
Across-slope shape: Concave
Other vegetative classification: Sandy soils on stream terraces, flood plains, or in depressions (G152AA145FL)
Hydric soil rating: Yes

Pantego

Percent of map unit: 3 percent
Landscape: Coastal plains
Landform: Depressions on marine terraces
Landform position (three-dimensional): Dip
Down-slope shape: Concave
Across-slope shape: Concave
Other vegetative classification: Loamy and clayey soils on stream terraces, flood plains, or in depressions (G152AA345FL)
Hydric soil rating: Yes

Plummer

Percent of map unit: 3 percent
Landscape: Coastal plains
Landform: Flats on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Other vegetative classification: Sandy soils on flats of mesic or hydric lowlands (G152AA141FL)
Hydric soil rating: Yes

Pottsburg, hydric

Percent of map unit: 3 percent
Landscape: Coastal plains
Landform: Flats on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Other vegetative classification: Sandy soils on flats of mesic or hydric lowlands (G152AA141FL)
Hydric soil rating: Yes

Alapaha

Percent of map unit: 2 percent
Landscape: Coastal plains
Landform: Flats on marine terraces, Drainageways on marine terraces, Depressions on marine terraces
Landform position (three-dimensional): Talf, dip
Down-slope shape: Linear
Across-slope shape: Concave
Other vegetative classification: Sandy over loamy soils on stream terraces, flood plains, or in depressions (G152AA245FL)
Hydric soil rating: Yes

Rains

Percent of map unit: 2 percent

Landscape: Coastal plains
Landform: Flats on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Other vegetative classification: Loamy and clayey soils on flats of hydric or mesic lowlands (G152AA341FL)
Hydric soil rating: Yes

Pansey

Percent of map unit: 2 percent
Landscape: Coastal plains
Landform: Flats on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Other vegetative classification: Loamy and clayey soils on flats of hydric or mesic lowlands (G152AA341FL)
Hydric soil rating: Yes

Data Source Information

Soil Survey Area: Bay County, Florida
Survey Area Data: Version 25, Aug 27, 2025

APPENDIX B
Subsurface Data

BORING LOCATION PLAN

NOVA
ENGINEERING AND ENVIRONMENTAL

PREPARED BY
NOVA Engineering and Environmental, LLC
Kennesaw, GA

PROJECT
Name: Mosley HS Softball Fieldhouse
Number: 10111-2026005

LOCATION
30.206815, -85.643291
Lynn Haven, FL

SYMBOL KEY
Test Boring
Soil Boring

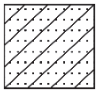
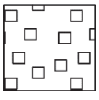
KEY TO SYMBOLS AND CLASSIFICATIONS

DRILLING SYMBOLS

	Standard Penetration Testing Sample
	Undisturbed Sample (UD)
	Auger without Sampling
	Rock Core Sample
	Standard Penetration Resistance (ASTM D1586)
	Dynamic Cone Penetrometer (DCP) Resistance
	Water Table at least 24 Hours after drilling
	Water Table 1 Hour or less after drilling
50/2"	Number of Blows (50) to Drive the Spoon a Number of Inches (2)
NX, NQ	Core Barrel Sizes: 2⅛- and 2-Inch Diameter Rock Core, Respectively
REC	Percentage of Rock Core Recovered
RQD	Rock Quality Designation - Percentage of Recovered Core Segments 4 or more Inches Long
	Loss of Drilling Fluid
N/E	Not Encountered
N/M	Not Measured
C	Boring Cave-in Depth
WOH	Weight of Hammer

DRILLING PROCEDURES

Soil sampling and standard penetration testing performed in general accordance with ASTM D1586-18[e1]. The standard penetration resistance (N-value) is the number of blows of a 140-pound hammer falling 30 inches to drive a 2-inch O.D., 1.375-inch I.D. split-barrel sampler one foot. Core drilling performed in general accordance with ASTM D2113-14. The undisturbed sampling procedure is described by ASTM D1587-15. Unless other arrangements are made, NOVA will dispose of all soil and rock samples at the time of report submission.

	Paving		Well Graded Sand - SW		Silt - ML
	Gravel / Graded Aggregate Base		Silty Sand - SM		Elastic Silt - MH
	Fill		Clayey Sand - SC		Low Plasticity Clay - CL
	Topsoil		Poorly graded silty, clayey sand - SM/SC		High Plasticity Clay - CH
	Alluvium		Clayey Sand and Gravel - SC/GC		Partially Weathered Rock (PWR)
	Poorly Graded Sand - SP		Silty Sand and Gravel - SM/GM		Rock

NOVA

CORRELATION OF PENETRATION RESISTANCE WITH RELATIVE DENSITY AND CONSISTENCY

	Number of Blows, "N"	Approximate Relative Density
	0 – 4	Very Loose
	5 – 10	Loose
SANDS	11 – 30	Medium Dense
	31 – 50	Dense
	Over 50	Very Dense

	Number of Blows, "N"	Approximate Consistency
	0 – 2	Very Soft
	3 – 4	Soft
SILTS	5 – 8	Firm
and	9 – 15	Stiff
CLAYS	16 – 30	Very Stiff
	31 – 50	Hard
	Over 50	Very Hard

SOIL CLASSIFICATION CHART

COARSE GRAINED SOILS	**GRAVELS**	Clean Gravel less than 5% fines	GW	Well graded gravel
			GP	Poorly graded gravel
		Gravels with Fines more than 12% fines	GM	Silty gravel
			GC	Clayey gravel
	SANDS	Clean Sand less than 5% fines	SW	Well graded sand
			SP	Poorly graded sand
		Sands with Fines more than 12% fines	SM	Silty sand
			SC	Clayey sand
FINE GRAINED SOILS	**SILTS AND CLAYS** Liquid Limit less than 50	Inorganic	CL	Lean clay
			ML	Silt
		Organic	OL	Organic clay and silt
	SILTS AND CLAYS Liquid Limit 50 or more	Inorganic	CH	Fat clay
			MH	Elastic silt
		Organic	OH	Organic clay and silt
HIGHLY ORGANIC SOILS		Organic matter, dark color, organic odor	PT	Peat

PARTICLE SIZE IDENTIFICATION

GRAVELS	Coarse	¾ inch to 3 inches
	Fine	No. 4 to ¾ inch
SANDS	Coarse	No. 10 to No. 4
	Medium	No. 40 to No. 10
	Fine	No. 200 to No. 40
SILTS AND CLAYS		Passing No. 200

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TEST BORING RECORD B-1

PROJECT NAME Mosley HS Softball Fieldhouse | **PROJECT NO.** 10111-2026005

CLIENT JRA Architects, Inc | **LATITUDE** 30.20682

PROJECT LOCATION Lynn Haven, Bay County, Florida | **LONGITUDE** -85.64300

LOCATION North side of building footprint | **ELEVATION** 36' (EGM96)

DRILLER Watson's Drilling LLC | **LOGGED BY** B. Hardtke

DRILLING METHOD Mud Rotary - Manual **% ENERGY** 60% | **DATE** 02/05/2026

DEPTH TO-WATER ▽ **INITIAL** 3' ▼ **AFTER 24 HOURS** N/M ⌒ **CAVING** N/M

ELEVATION (ft)	DEPTH (ft)	MATERIAL DESCRIPTION	BLOW COUNTS (UNCORRECTED N-VALUE)
30	5	Brown silty SAND (SM)	3-4-4-3 (8)
		Loose to medium dense light brown poorly graded SAND (SP-SM) with silt	2-3-3-4 (6)
25	10		5-6-6-5 (12)
		Medium dense dark brown silty SAND (SM)	
20	15		6-8-8 (16)
		Medium dense brown poorly graded SAND (SP-SM) with silt	
15	20		9-10-11 (21)
		Medium dense gray poorly graded SAND (SP-SM) with silt	
10	25		10-10-12 (22)
5	30		5-6-6 (12)
0	35		5-5-6 (11)
		Medium dense brown poorly graded SAND (SP-SM) with silt	
-5	40		8-7-8 (15)
-10	45		6-6-6 (12)
-15	50		7-6-7 (13)
		End of boring at 50'	

GRAPHIC LOG | WATER LEVELS (ft) | SAMPLE TYPE

● N-Value
Plastic/Liquid Limit (%)
▲ Moisture Content (%)
▫ Fines Content (%)

10 20 30 40 50 60 70 80 90

Notes: 1) Ground Surface Elevations were interpolated through Google Earth imagery and should be noted as approximate. 2) A manual hammer with rope and cathead was used for the SPT sampling and testing. 3) Hammer energy efficiency of 60% is a published reference value for manual safety hammers.

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TEST BORING RECORD B-2

PROJECT NAME Mosley HS Softball Fieldhouse | **PROJECT NO.** 10111-2026005
CLIENT JRA Architects, Inc | **LATITUDE** 30.20671
PROJECT LOCATION Lynn Haven, Bay County, Florida | **LONGITUDE** -85.64299
LOCATION South side of building footprint | **ELEVATION** 36' (EGM96)
DRILLER Watson's Drilling LLC | **LOGGED BY** B. Hardtke
DRILLING METHOD Mud Rotary - Manual **% ENERGY** 60% | **DATE** 02/05/2026
DEPTH TO-WATER ▽ **INITIAL** 3' ▼ **AFTER 24 HOURS** N/M ⌒ **CAVING** N/M

ELEVATION (ft)	DEPTH (ft)	MATERIAL DESCRIPTION	BLOW COUNTS (UNCORRECTED N-VALUE)
30	5	Brown silty SAND (SM)	2-2-2-3 (4)
		Loose light brown poorly graded SAND (SP-SM) with silt	3-2-3-3 (5)
25	10		4-5-5-4 (10)
		Loose to medium dense brown poorly graded SAND (SP-SM) with silt	
20	15		7-7-9 (16)
15	20	End of boring at 20'	7-8-8 (16)
10	25		
5	30		
0	35		
-5	40		
-10	45		
-15	50		

GRAPHIC LOG | WATER LEVELS (ft) | SAMPLE TYPE

● N-Value
Plastic/Liquid Limit (%)
▲ Moisture Content (%)
▫ Fines Content (%)

10 20 30 40 50 60 70 80 90

Notes: 1) Ground Surface Elevations were interpolated through Google Earth imagery and should be noted as approximate. 2) A manual hammer with rope and cathead was used for the SPT sampling and testing. 3) Hammer energy efficiency of 60% is a published reference value for manual safety hammers.

NOVA
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SOIL BORING RECORD
S-1

PROJECT NAME Mosley HS Softball Fieldhouse | **PROJECT NO.** 10111-2026005
CLIENT JRA Architects, Inc | **LATITUDE** 30.20676
PROJECT LOCATION Lynn Haven, Bay County, Florida | **LONGITUDE** -85.64293
LOCATION South side of pond footprint | **ELEVATION** 36' (EGM96)
DRILLER B. Hardtke | **LOGGED BY** B. Hardtke
DRILLING METHOD Hand Auger | **DATE** 02/06/2026
DEPTH TO-WATER ▽ **INITIAL** 2' ▼ **AFTER 24 HOURS** N/M ⊂ **CAVING** 3'

ELEVATION (ft)	DEPTH (ft)	MATERIAL DESCRIPTION	GRAPHIC LOG	WATER LEVELS (ft)	SAMPLE TYPE	BLOW COUNTS (UNCORRECTED N-VALUE)
		Brown poorly graded SAND (SP-SM) with silt				
35		Dark gray silty SAND (SM)				
		Light brown poorly graded SAND (SP-SM) with silt		▽		
		Collapsed at 3'				

● N-Value
Plastic/Liquid Limit (%)
▲ Moisture Content (%)
▫ Fines Content (%)
10 20 30 40 50 60 70 80 90

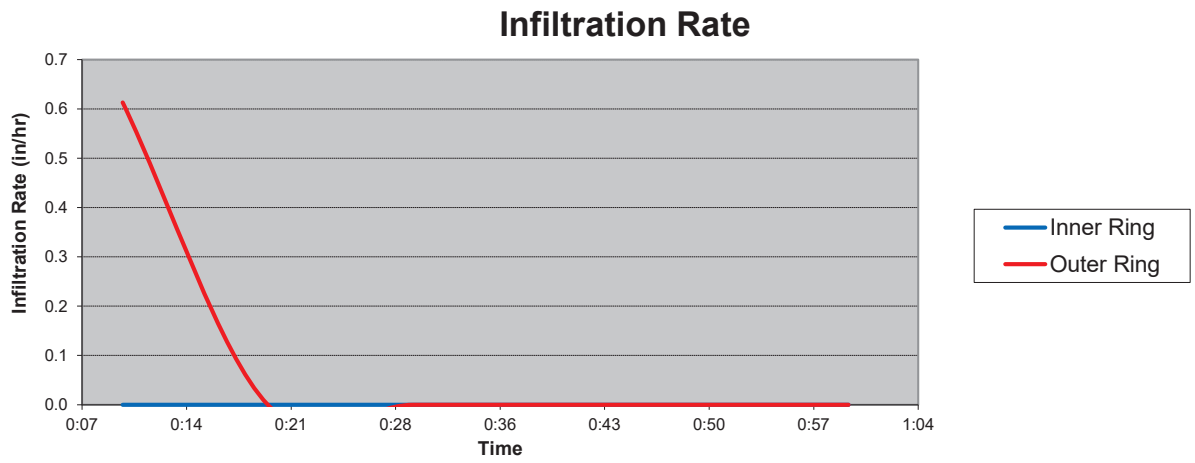
Notes: 1) Ground Surface Elevations were interpolated through Google Earth imagery and should be noted as approximate.

Project:	**Mosley HS Softball Fieldhouse**	**Report of DRI - 1 (Boring S-1)**
Project Location:	**Lynn Haven, Bay County, Florida**	
Project Number:	**10111-2025005**	

Date(s) of Test	**February 6, 2026**	Tested by	**B. Hardtke**	Weather	**Sunny**
Test Method	**ASTM D 3385**	Logged by	**B. Hardtke**	Type of liquid	**tap water**
Area Inner Ring	**110.75 sq. in.**	Checked by	**J. Strickland**	Liquid Temperature	**80 °F**
Area Outer Ring	**447.69 sq. in.**	See Auger Boring Record for Soil Profile		Soil Temperature	**83 °F**
Area Annular Space	**334.59 sq. in.**	Approx. Elevation	**12 in. below grade**	Location	**Per Boring Location Plan**

INCREMENTAL INFILTRATION RATE vs. TOTAL ELAPSED TIME

		Inner Ring		Outer Ring		
Time	**Elapsed Time (minutes)**	**Volume (gal)**	**Infiltration Rate (In/hr)**	**Volume (gal)**	**Infiltration Rate (In/hr)**	**Comments**
0:10	10	0.0	0.0	0.2	0.6	
0:20	10	0.0	0.0	0.0	0.0	
0:30	10	0.0	0.0	0.0	0.0	
0:40	10	0.0	0.0	0.0	0.0	
0:50	10	0.0	0.0	0.0	0.0	
1:00	10	0.0	0.0	0.0	0.0	



INFILTRATION RATE = 0 in/hr

Figure DRI-1

NOVA ENGINEERING AND ENVIRONMENTAL

SOIL BORING RECORD S-2

PROJECT NAME Mosley HS Softball Fieldhouse | **PROJECT NO.** 10111-2026005

CLIENT JRA Architects, Inc | **LATITUDE** 30.20688

PROJECT LOCATION Lynn Haven, Bay County, Florida | **LONGITUDE** -85.64292

LOCATION North side of pond footprint | **ELEVATION** 36' (EGM96)

DRILLER B. Hardtke | **LOGGED BY** B. Hardtke

DRILLING METHOD Hand Auger | **DATE** 02/06/2026

DEPTH TO-WATER ▽ **INITIAL** 3' ▼ **AFTER 24 HOURS** N/M ▽ **CAVING** 4'

ELEVATION (ft)	DEPTH (ft)	MATERIAL DESCRIPTION	GRAPHIC LOG	WATER LEVELS (ft)	SAMPLE TYPE	BLOW COUNTS (UNCORRECTED N-VALUE)
35	0	Brown poorly graded SAND (SP-SM) with silt				
	1	Dark gray silty SAND (SM)				
	3	Light brown poorly graded SAND (SP-SM) with silt		▽		
	4	Collapsed at 4'				

● N-Value
Plastic/Liquid Limit (%)
▲ Moisture Content (%)
▫ Fines Content (%)

10 20 30 40 50 60 70 80 90

Notes: 1) Ground Surface Elevations were interpolated through Google Earth imagery and should be noted as approximate.

This information pertains only to this boring and should not be interpreted as being indicative of the site.

APPENDIX C
Laboratory Data

Laboratory Testing

Mosley HS Softball Fieldhouse
Lynn Haven, Bay County, Florida
NOVA Project Number 10111-2026005

Moisture Content

The moisture content is the ratio expressed as a percentage of the weight of water in a given mass of soil to the weight of the solid particles. This testing was conducted in general accordance with ASTM Designation D-2216. Six moisture content tests were performed in this study.

Fines Content

The percentage of fines passing through the No. 200 sieve is generally considered to represent the amount of silt and clay of the tested soil sample. The sieve analysis testing was conducted in general accordance with ASTM Designations D-6913 and D-1140. Six fines content tests were performed in this study.

Organic Content

The organic content is the ratio expressed as a percentage of the weight of organic material in a given mass of soil to the weight of the solid particles. This testing was conducted in general accordance with ASTM D-2974. Five organic content tests were performed in this study.

Remolded Falling Head Permeability Testing

A remolded falling head permeability test (ASTM D-5084) is a common laboratory test used to determine the hydraulic conductivity of fine-grained soils. The test involves the flow of water through a re-molded, fully saturated soil sample inside a rigid-wall permeameter connected to a standpipe of constant diameter. Before beginning the flow measurements, the soil sample is saturated, and the standpipe is filled with water to a given level. The test then starts by allowing the water to flow through the sample until the water in the standpipe reaches a lower limit. The time required for the water to flow from the upper to lower limit is recorded. One remolded permeability test was performed in this study.

SUMMARY OF CLASSIFICATION & INDEX TESTING

Mosley HS Softball Fieldhouse
Lynn Haven, Bay County, Florida
NOVA Project Number 10111-2026005

Boring Number	Sample Depth (ft)	Natural Moisture (%)	Percent (%) Passing Sieve #200	Percent Organic (%)	USCS Soil Classification
B-2	0 – 2	14	18	4	SM
S-1	1 – 2	19	15	2	SM
B-1	2 – 4	19	7	N/A	SP-SM
S-2	3 – 4	19	10	1	SP-SM
B-2	4 – 6	16	8	0	SP-SM
B-1	28.5 – 30	28	11	1	SP-SM

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PERMEABILITY, -200 SIEVE WASH, AND MOISTURE CONTENT

PROJECT: Mosley HS Softball Fieldhouse **NOVA PROJECT #:** 10111-2026005

DATE: 2/12/2026 **ASSIGNED BY:** J. Strickland **TESTED BY:** M. Keramidas

Sample LOCATION / BORING NO.	S-2
Sample NUMBER / DEPTH	0 - 3

PERMEABILITY TESTING SUMMARY			
PERMEABILITY (K_V)	→	1	ft/day
Corresponding K_h	→	2	ft/day
DRY DENSITY	→	100	lbs/ft^3
MOISTURE CONTENT	→	4	%
-200 FINES CONTENT	→	11	%

FALLING HEAD PERMEABILITY (ASTM D 5084)

No. of LAYERS:	3	Wt. of MOLD (lbs):	9.31
BLOWS/LAYER:	15	Wt. of MOLD/SOIL (lbs):	12.77

HEIGHT (FT)	TRIAL #1 (SEC)	TRIAL #2 (SEC)	PERMEABILITY	ASTM Check
5	0.0	0.0		
4	86.1	96.1	4.45E-04	Passed
3	120.0	132.6	4.14E-04	Passed
2	187.1	187.2	3.94E-04	Passed
1	317.0	320.4	3.95E-04	Passed
				Below Standard: Rerun Test
				Below Standard: Rerun Test
Average Permeability		4.1E-04		cm/sec

NUMBER OF INCHES MOLD WAS SHORT? 0.000 INCHES (ZERO INCHES IS DEFAULT)

PERMEABILITY CONSTANT USED WAS → 0.07 (Includes 1/2" ID tubing)

MOISTURE CONTENT (ASTM D 2216)

Pan NUMBER	J-72
Wt. of WET SOIL &	225.6
Wt. of DRY SOIL &	218.7
Wt. of PAN (g)	50.3
Wt. of Water (g)	6.9
Wt. of Dry Soil (g)	168.4
MOISTURE CONTE	4.1

-200 SIEVE WASH (ASTM D 1140)

Pan NUMBER	J-72
Wt. of DRY SOIL & P	156.3
Wt. of WASH SOIL &	144.6
Wt. of PAN (g)	50.3
Wt. of Original Dry S	106.0
Wt. of -200 Material (	11.7
Wt. of Washed Dry S	94.3
-200 FINES CONTEN	11.0

APPENDIX D
Support Documents

Important Information about This

Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

Geotechnical Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a civil engineer may not fulfill the needs of a constructor — a construction contractor — or even another civil engineer. Because each geotechnical- engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client. No one except you should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one — not even you* — should apply this report for any purpose or project except the one originally contemplated.

Read the Full Report

Serious problems have occurred because those relying on a geotechnical-engineering report did not read it all. Do not rely on an executive summary. Do not read selected elements only.

Geotechnical Engineers Base Each Report on a Unique Set of Project-Specific Factors

Geotechnical engineers consider many unique, project-specific factors when establishing the scope of a study. Typical factors include: the client's goals, objectives, and risk-management preferences; the general nature of the structure involved, its size, and configuration; the location of the structure on the site; and other planned or existing site improvements, such as access roads, parking lots, and underground utilities. Unless the geotechnical engineer who conducted the study specifically indicates otherwise, do not rely on a geotechnical-engineering report that was:

- not prepared for you;
- not prepared for your project;
- not prepared for the specific site explored; or
- completed before important project changes were made.

Typical changes that can erode the reliability of an existing geotechnical-engineering report include those that affect:

- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes—even minor ones—and request an assessment of their impact. *Geotechnical engineers cannot accept responsibility or liability for problems that occur because their reports do not consider developments of which they were not informed.*

Subsurface Conditions Can Change

A geotechnical-engineering report is based on conditions that existed at the time the geotechnical engineer performed the study. *Do not rely on a geotechnical-engineering report whose adequacy may have been affected by*: the passage of time; man-made events, such as construction on or adjacent to the site; or natural events, such as floods, droughts, earthquakes, or groundwater fluctuations. *Contact the geotechnical engineer before applying this report to determine if it is still reliable.* A minor amount of additional testing or analysis could prevent major problems.

Most Geotechnical Findings Are Professional Opinions

Site exploration identifies subsurface conditions only at those points where subsurface tests are conducted or samples are taken. Geotechnical engineers review field and laboratory data and then apply their professional judgment to render an opinion about subsurface conditions throughout the site. Actual subsurface conditions may differ — sometimes significantly — from those indicated in your report. Retaining the geotechnical engineer who developed your report to provide geotechnical-construction observation is the most effective method of managing the risks associated with unanticipated conditions.

A Report's Recommendations Are Not Final

Do not overrely on the confirmation-dependent recommendations included in your report. *Confirmation-dependent recommendations are not final,* because geotechnical engineers develop them principally from judgment and opinion. Geotechnical engineers can finalize their recommendations *only* by observing actual subsurface conditions revealed during construction. *The geotechnical engineer who developed your report cannot assume responsibility or liability for the report's confirmation-dependent recommendations if that engineer does not perform the geotechnical-construction observation required to confirm the recommendations' applicability.*

A Geotechnical-Engineering Report Is Subject to Misinterpretation

Other design-team members' misinterpretation of geotechnical-engineering reports has resulted in costly

problems. Confront that risk by having your geotechnical engineer confer with appropriate members of the design team after submitting the report. Also retain your geotechnical engineer to review pertinent elements of the design team's plans and specifications. Constructors can also misinterpret a geotechnical-engineering report. Confront that risk by having your geotechnical engineer participate in prebid and preconstruction conferences, and by providing geotechnical construction observation.

Do Not Redraw the Engineer's Logs

Geotechnical engineers prepare final boring and testing logs based upon their interpretation of field logs and laboratory data. To prevent errors or omissions, the logs included in a geotechnical-engineering report should *never* be redrawn for inclusion in architectural or other design drawings. Only photographic or electronic reproduction is acceptable, *but recognize that separating logs from the report can elevate risk.*

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can make constructors liable for unanticipated subsurface conditions by limiting what they provide for bid preparation. To help prevent costly problems, give constructors the complete geotechnical-engineering report, *but* preface it with a clearly written letter of transmittal. In that letter, advise constructors that the report was not prepared for purposes of bid development and that the report's accuracy is limited; encourage them to confer with the geotechnical engineer who prepared the report (a modest fee may be required) and/or to conduct additional study to obtain the specific types of information they need or prefer. A prebid conference can also be valuable. *Be sure constructors have sufficient time* to perform additional study. Only then might you be in a position to give constructors the best information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions.

Read Responsibility Provisions Closely

Some clients, design professionals, and constructors fail to recognize that geotechnical engineering is far less exact than other engineering disciplines. This lack of understanding has created unrealistic expectations that have led to disappointments, claims, and disputes. To help reduce the risk of such outcomes, geotechnical engineers commonly include a variety of explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

Environmental Concerns Are Not Covered

The equipment, techniques, and personnel used to perform an *environmental* study differ significantly from those used to perform a *geotechnical* study. For that reason, a geotechnical-engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated environmental problems have led to numerous project failures.* If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. *Do not rely on an environmental report prepared for someone else.*

Obtain Professional Assistance To Deal with Mold

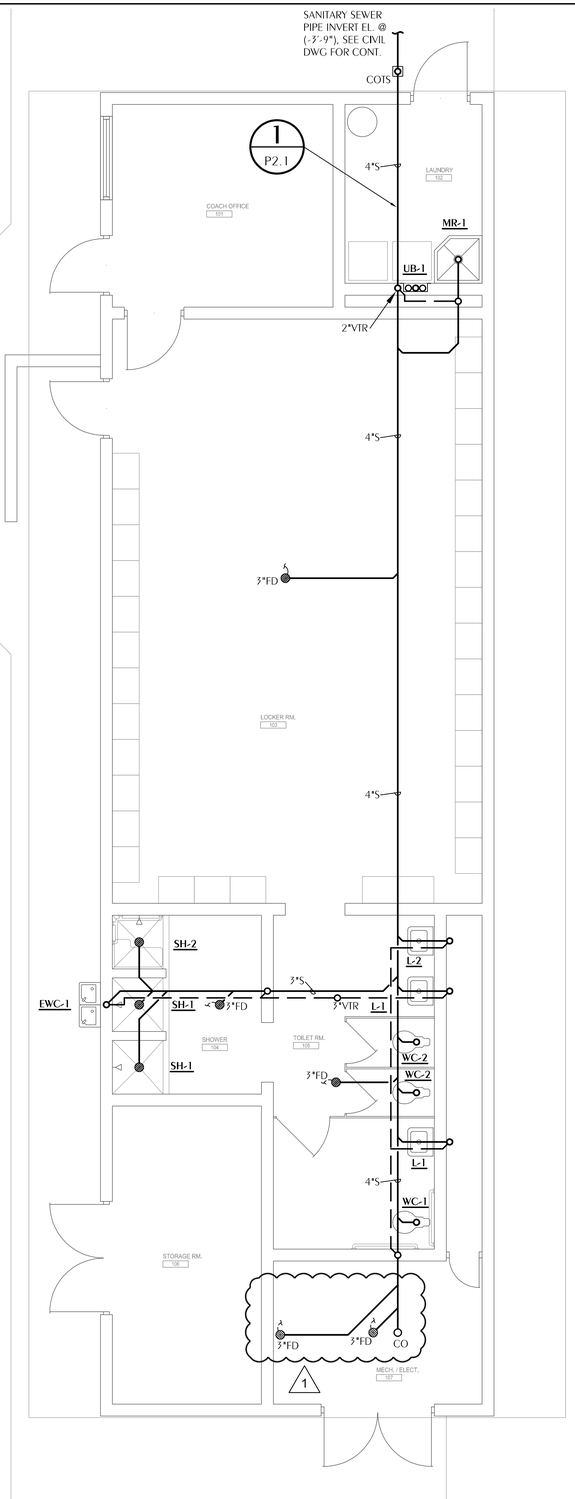
Diverse strategies can be applied during building design, construction, operation, and maintenance to prevent significant amounts of mold from growing on indoor surfaces. To be effective, all such strategies should be devised for the *express purpose* of mold prevention, integrated into a comprehensive plan, and executed with diligent oversight by a professional mold-prevention consultant. Because just a small amount of water or moisture can lead to the development of severe mold infestations, many mold- prevention strategies focus on keeping building surfaces dry. While groundwater, water infiltration, and similar issues may have been addressed as part of the geotechnical- engineering study whose findings are conveyed in this report, the geotechnical engineer in charge of this project is not a mold prevention consultant; *none of the services performed in connection with the geotechnical engineer's study were designed or conducted for the purpose of mold prevention. Proper implementation of the recommendations conveyed in this report will not of itself be sufficient to prevent mold from growing in or on the structure involved.*

Rely, on Your GBC-Member Geotechnical Engineer for Additional Assistance

Membership in the Geotechnical Business Council of the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project. Confer with you GBC-Member geotechnical engineer for more information.

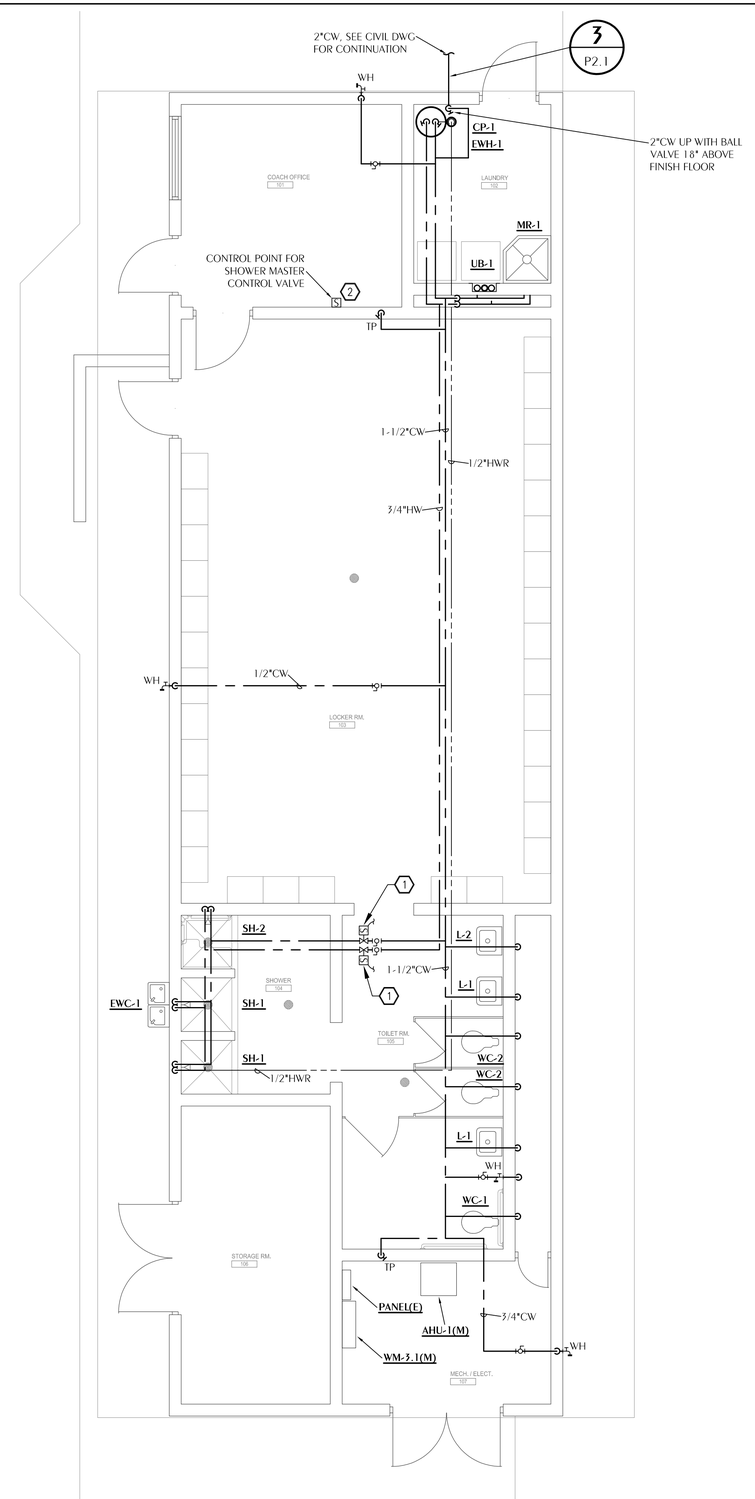
GEOTECHNICAL BUSINESS COUNCIL
of the Geoprofessional Business Association

8811 Colesville Road/Suite G106, Silver Spring, MD 20910
Telephone: 301/565-2733 Facsimile: 301/589-2017
e-mail: info@geoprofessional.org www.geoprofessional.org



1 SANITARY NEW WORK FLOOR PLAN

SCALE: 1/4" = 1'-0"



2 DOMESTIC WATER NEW WORK FLOOR PLAN

SCALE: 1/4" = 1'-0"

REFERENCE:
FINISHED FLOOR ELEVATION = 0'-0"

GENERAL NOTES

1. DUE TO PROXIMITY BETWEEN DOMESTIC WATER AND SANITARY BEING CLOSE WHILE ENTERING AND EXITING THE BUILDING, MAINTAIN MINIMUM 12" VERTICAL CLEARANCE BETWEEN THE TWO SYSTEMS PER SECTION 603.2 OF THE 2023 FLORIDA BUILDING CODE, PLUMBING, 8TH EDITION.

SHEET NOTES

1. PROVIDE 120 VOLT NORMALLY CLOSED SOLENOID VALVE FOR ON/OFF MASTER VALVE SERVING SHOWER AREAS. COORDINATE WITH ELECTRICAL POWER DRAWINGS. VALVE CONTROLS LOCATED IN COACHES' OFFICE.
2. VALVE CONTROLS FOR ON/OFF MASTER VALVE. INTERLOCK WITH OTHER CONTROLLED AREAS, COORDINATE WITH ELECTRICAL DRAWINGS. MOUNT AT 54" ABOVE FLOOR. PROVIDE ENGRAVED PLASTIC SIGNAGE IDENTIFYING CONTROL AS "MASTER SHOWER CONTROL VALVE".

REVISIONS

NO.	DESCRIPTION	DRAWN	CHECKED	DATE
1	ADDENDUM 2	JDD	KAJ	4/17/26

PHASE	DRAWN	CHECKED	DATE
SCHEMATIC DESIGN	JDD	KAJ	10/17/25
DESIGN DEVELOPMENT	JDD	KAJ	1/9/26
CONSTRUCTION DOCUMENTS	JDD	KAJ	4/3/26

JRA ARCHITECTS
2211 NAVY BLVD., STE 100
PANAMA CITY BEACH, FL
PHONE: (850) 236-9832
Commission Number: 25811

CONSULTANTS:

WATFORD ENGINEERING
4452 Clinton Street, Marianna, Florida 32446 Florida Certificate of Authorization: 27825
850.526.3447 Project Number: 2025-095 Keith A. Johnson, PE Florida License 86407

PROJECT:

BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD HOUSE
MOSLEY HIGH SCHOOL

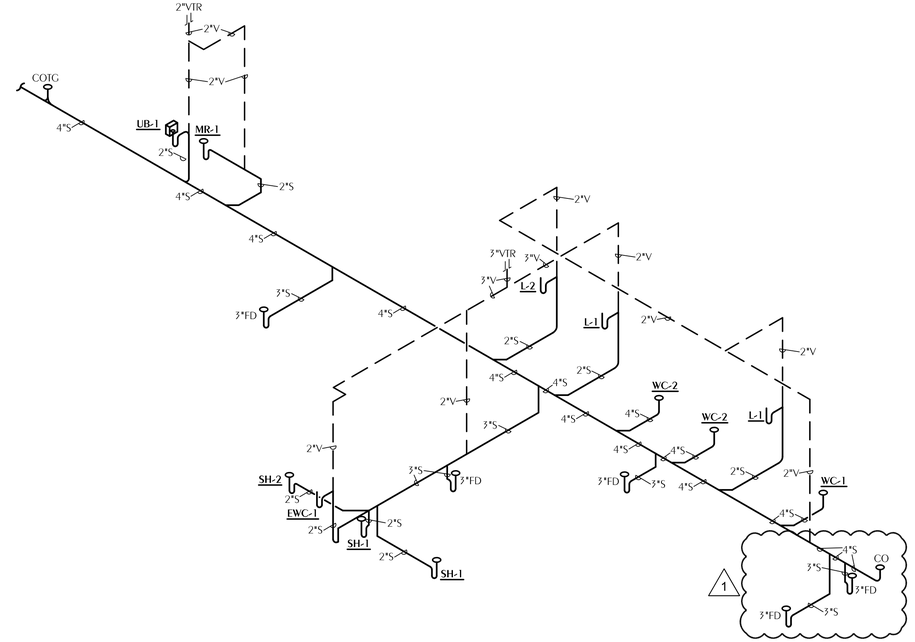
LYNN HAVEN, FLORIDA

SHEET TITLE:

PLUMBING NEW WORK FLOOR PLANS

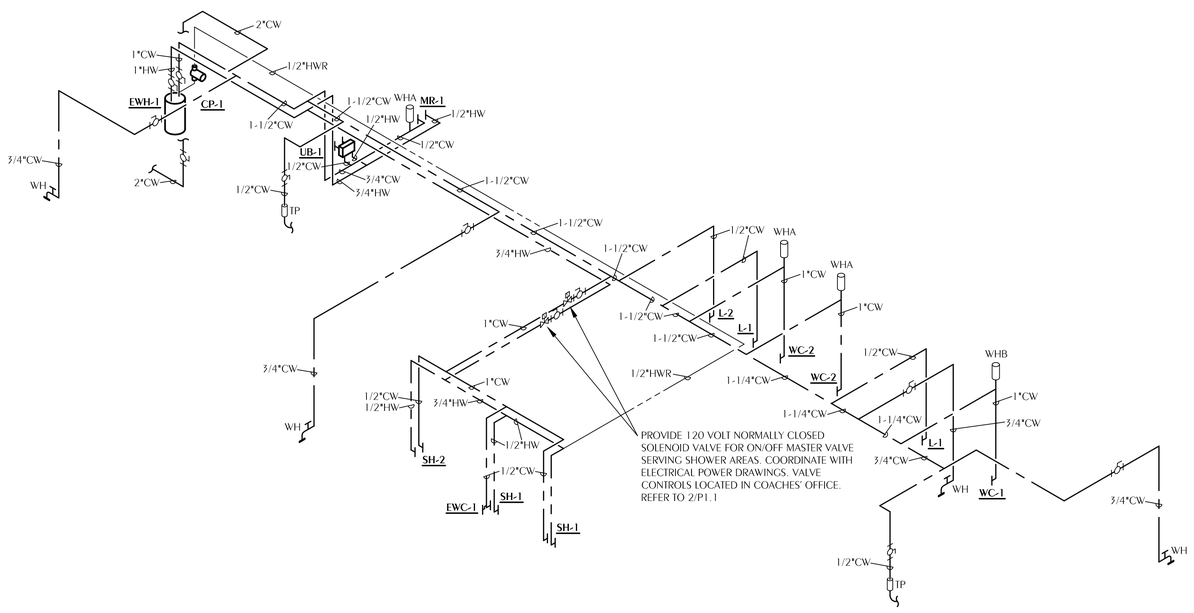
SHEET NUMBER:

P1.1



1 SANITARY RISER DIAGRAM

P2.1 SCALE: NONE



2 DOMESTIC WATER RISER DIAGRAM

P2.1 SCALE: NONE

REVISIONS				
NO.	DESCRIPTION	DRAWN	CHECKED	DATE
1	ADDENDUM 2	JDD	KAJ	4/17/26

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JRA ARCHITECTS
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4452 Clinton Street, Marianna, Florida 32446 Florida Certificate of Authorization: 27825
850.526.3447 Project Number: 2025-095 Keith A. Johnson, PE Florida License 89457

PROJECT:

BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD HOUSE
MOSLEY HIGH SCHOOL

LYNN HAVEN, FLORIDA

SHEET TITLE:

PLUMBING RISER DIAGRAMS

SHEET NUMBER:

P2.1

Addendum #3

Mosley Softball Fieldhouse

JRA Commission. 25871 CA/BC

Bay District Schools
1311 Balboa Avenue
Panama City, Florida 32401

JRA Architects, Inc.
2211 Navy Blvd- Suite 100
Panama City, Florida 32408

Date of Issue of addendum: **April 23, 2026**

The changes herewith form part of the Construction Documents and modify the original "Construction Documents" dated April 3, 2026.

This Addendum consists of **1** page(s), 0 attachment(s) and **2** revised plan sheet(s) attached and as referenced herein.

ITEM 3.1

Drawings: **Clarification to Construction Documents** – See Note 1 on sheet M1.1 the dryer exhaust goes to a gooseneck on the roof.

ITEM 3.2

Drawings: **Clarification to Construction Documents**

- The condensate drain for CC-2.1 should be drained to the mop sink in 102.
- The condensate drain for WM-1.1 and WM-3.1 should be drained to the new floor drain that was added in Mech. / Electrical 107.

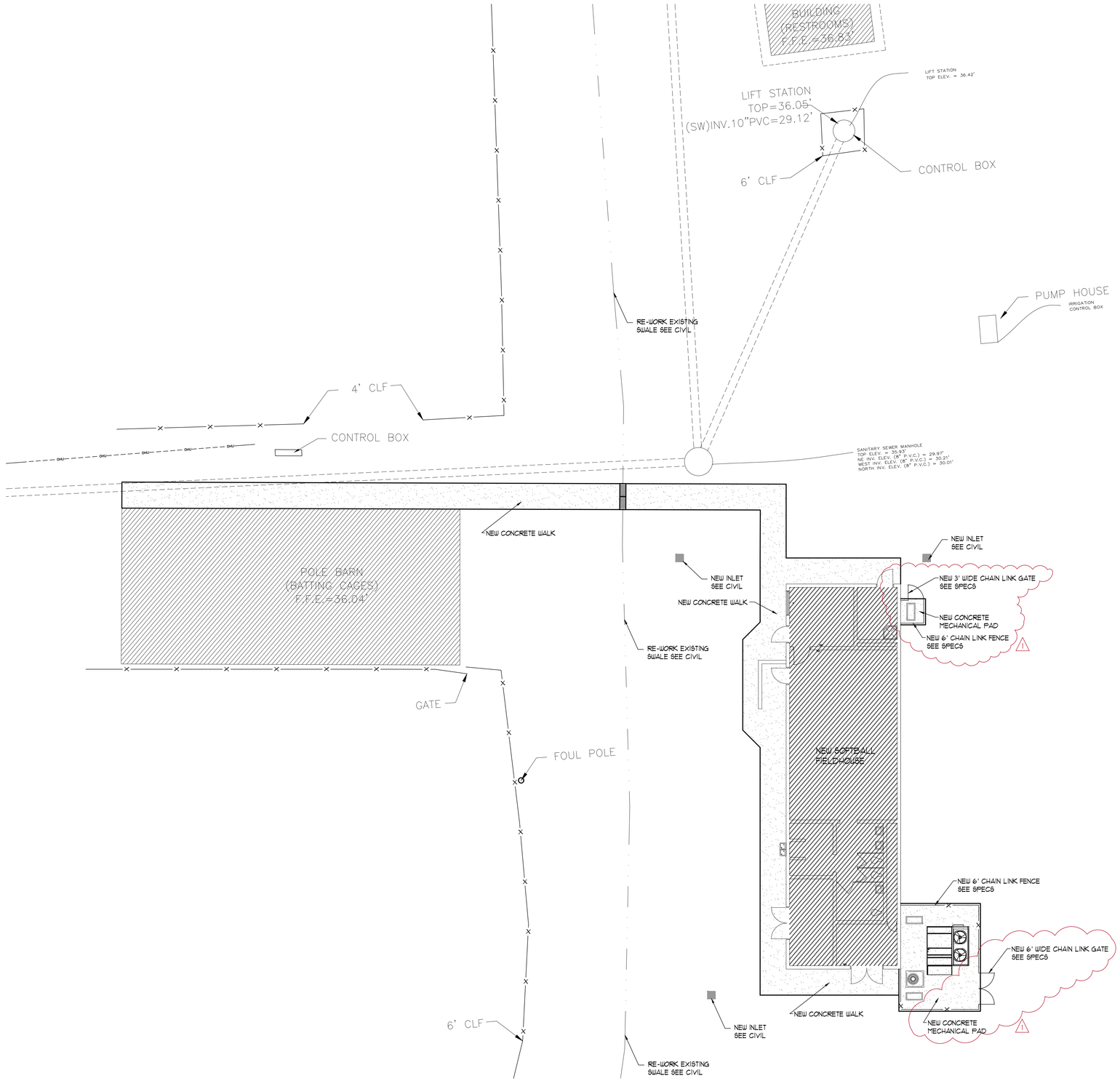
ITEM 3.3

Drawings: Replace **Drawings sheet A0.1 Site Plan** with the Attached Drawing Sheet A0.1 with a revision date of 4/23/26.

ITEM 3.4

Drawings: Replace **Drawings sheet A1.1 Floor Plan** with the Attached Drawing Sheet A1.1 with a revision date of 4/23/26.

END OF ADDENDUM #3



ARCHITECTURAL SITE PLAN

SCALE: 1" = 10'-0"

REVISIONS

NO.	DESCRIPTION	DRAWN	CHECKED	DATE
1	ADDENDUM 3			4/23/26

PHASE	DRAWN	CHECKED	DATE
SCHEMATIC DESIGN	MAY	SLB	10/17/25
DESIGN DEVELOPMENT	MAY	SLB	1/9/26
CONSTRUCTION DOCUMENTS	MAY	SLB	4/3/26

JRA ARCHITECTS
2211 NAVY BLVD., STE 100
PANAMA CITY BEACH, FL
PHONE: (850) 236-9832
Commission Number: 25811

CONSULTANTS:

PROJECT:

BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD HOUSE
MOSLEY HIGH SCHOOL

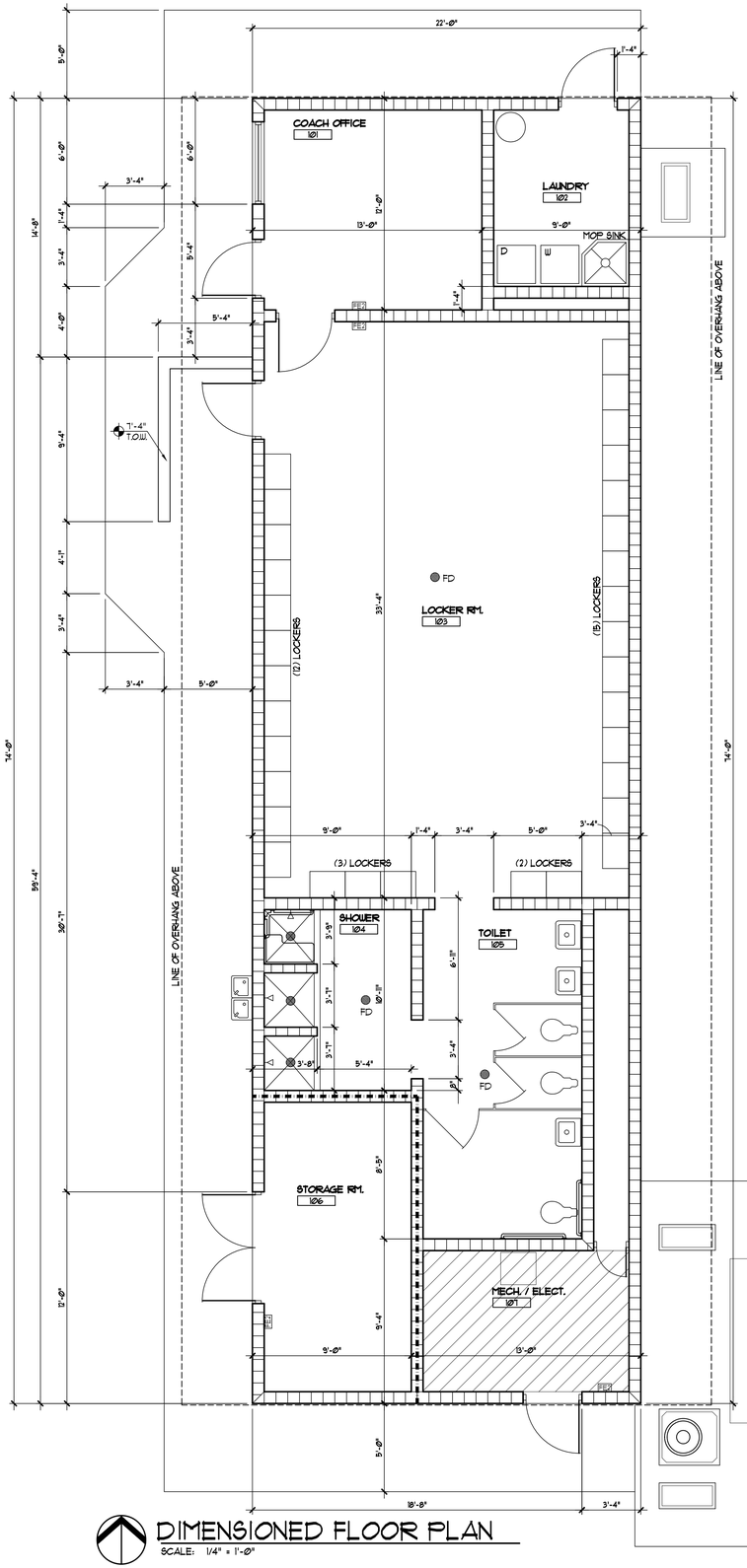
LYNN HAVEN, FLORIDA

SHEET TITLE:

SITE PLAN

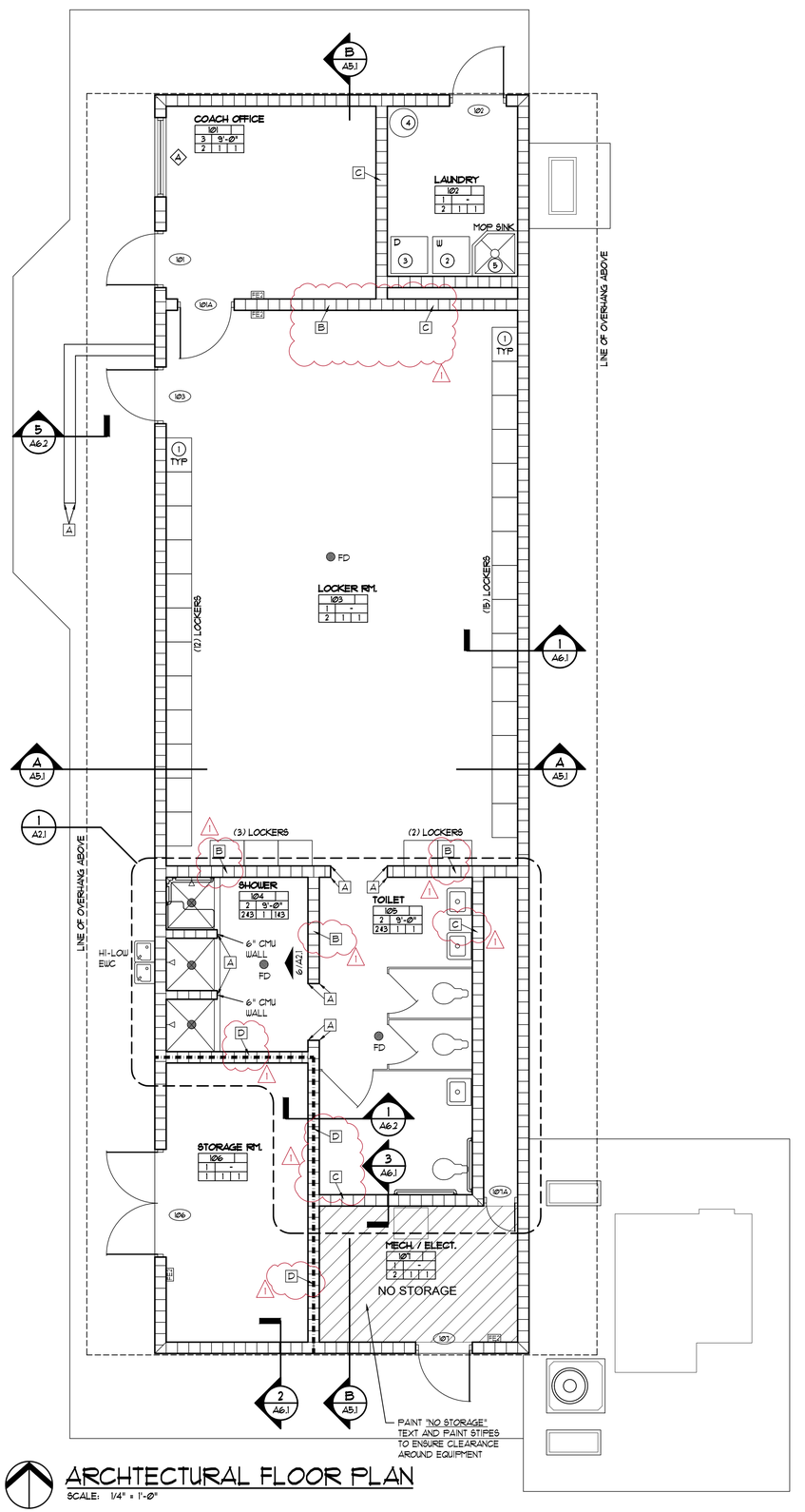
SHEET NUMBER:

A0.1



DIMENSIONED FLOOR PLAN

SCALE: 1/4" = 1'-0"



ARCHITECTURAL FLOOR PLAN

SCALE: 1/4" = 1'-0"

FINISH SCHEDULE

FLOORS
1. SEALED CONCRETE
2. EPOXY PAINT
3. RESINIOUS EPOXY FLOORING

BASE
1. VINYL
2. NOT USED
3. NOT USED
4. NOT USED

WALLS
1. EPOXY PAINTED CMU
2. NOT USED
3. APPLIED EPOXY
4. NOT USED

CEILING
1. EXPOSED TRUSSES
2. MR GYP BD
3. ACOUSTICAL LAY IN
4. NOT USED

REMARKS

SPACE NAME
SPACE NUMBER — REMARKS
CLG HT
WALL
BASE
FLOOR
CEILING

LEGEND

FE - SURFACE MOUNTED FIRE EXTINGUISHER
FE1 - SEMI-RECESSED FIRE EXTINGUISHER
FE2 - BRACKET HUNG FIRE EXTINGUISHER

EQUIPMENT SCHEDULE

1. 24"W x 74 3/4"H x 18" D - SINGLE TIER LOCKERS, N.I.C.
2. WASHER, N.I.C.
3. DRYER, N.I.C.
4. ELECTRIC WATER HEATER - SEE PLUMBING
5. JANITOR'S MOP SINK - SEE PLUMBING

CONSTRUCTION KEYNOTES

A. BULL NOSE - CMU BLOCK

B. METAL FRAMED WALL W/ MOISTURE RESISTANT GWB FINISH @ EA. SIDE (CONT.) FROM TOP OF CMU WALL TO ROOF DECK - SEE SECTIONS.

C. METAL FRAMED WALL W/ MOISTURE RESISTANT GWB FINISH @ EA. SIDE (CONT.) W/ (R-19) OPEN CELL SPRAYED INSULATION (CONT.) FROM TOP OF CMU WALL TO ROOF DECK - SEE SECTIONS.

D. MTL FRAMED WALL W/ 5/8" TYPE "X" FIRE RATED GYP BD BOTH SIDES (CONT. FROM TOP OF CMU PORTION OF THE WALL TO UNDERSIDE OF MTL ROOF DECK W/ MINERAL WOOL BETWEEN TOP OF MTL TRACK & MTL ROOF DECK. ABOVE & FIRE CAULKED @ TOP OF GYP BD TO DECKING ABOVE - ENTIRE WALL TO BE FIRE RATED - METAL FRAMED PORTION OF THE WALL TO MEET (UL #U465) CMU PORTION OF THE WALL TO MEET (UL #306)

REVISIONS

NO.	DESCRIPTION	DRAWN	CHECKED	DATE
1	ADDENDUM 3			4/23/26

PHASE	DRAWN	CHECKED	DATE
SCHEMATIC DESIGN	MAY	SLB	10/17/25
DESIGN DEVELOPMENT	MAY	SLB	1/9/26
CONSTRUCTION DOCUMENTS	MAY	SLB	4/3/26

JRA ARCHITECTS
2211 NAVY BLVD., STE 100
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PHONE: (850) 236-9832
Commission Number: 25871

CONSULTANTS:

PROJECT:

BAY DISTRICT SCHOOLS
NEW SOFTBALL FIELD HOUSE
MOSLEY HIGH SCHOOL

LYNN HAVEN, FLORIDA

SHEET TITLE:

FLOOR PLAN

SHEET NUMBER:

A1.1

BDS Mosley HS New Softball Field House

RFI #	Question	Bid Package	Architect/School District Response
1	Confirm there are no requirements for access control, fire alarm or telecom for the Mesley Softball Field House Project? 4.20.26 9AM	16A	There are no requirements for access control, fire alarm or telecom for the Mosely Softball Field House 4.20.26 10:25AM
2	Confirm that Asphaltic Concrete Paving is not needed Sheet C1.10 There is no drawing detail. 4.20.26 11:08AM	02A	This is correct, there is no asphalt paving on this project. 4.20.26 11:08AM
3	Opening dimension needed for the window A4.1 4.20.26 11:08AM	10A	The size of the window is provided on sheet A9.1 and in the specifications 08 43 13 paragraph 2.2 performance requirements item C states that wind loads are to be as indicated on the structural drawings. Wind Velocity is given on sheet S0.2 and component and cladding pressures are given there as well. 4.20.26 11:08AM
4	Is there a specific Waterproof Material needed for Sheet E1.2 Keynote 7 4.20.26 11:08AM	16A	This note is referencing an in-use weatherproof switch cover.
5	Regarding the window Type; the specs call for both fixed and operable window systems. Which window system will be used? 4.20.26 12:34PM	07A	Fixed 4.20.26 12:41PM
6	Confirm no gutter and downspout required.	07A	That Is Correct there are now gutters and downspouts
7	1)Sheet A0.1 a.The plan does not show the exterior mechanical pad/fencing that is shown on the civil and mechanical plans.	03A 15C 10A	See Addendum 3
8	b.Need information on the fencing for the mechanical areas (height and gate width) Sheet A0.1	10A	See Addendum 3
9	c.Confirm both areas will have an exterior concrete pad and if the HVAC units will have bases or if concrete bases will be needed. Sheet A0.1	02A 03A	Yes both will have concrete pads See Addendum 3

10	d.There is a note to rework the swale and what appears to be some type of culvert?? Please confirm what that is and provide a detail.Sheet A0.1	02A	Answered in Addendum 2 C1.5 DS1, DS2, DS3
11	2)Sheet A1.1 a.Need designation for the type of CMU wall for the exterior (Shane said it will need spray foam in the CMU but it's not called out and the wall sections don't show it either).	04A 05A	Answerd in Addendum 2 - Refer to Drawings Sheet A1.1 Architectural Floor Plan
12	b.Many interior walls are showing metal stud framing/sheathing with spray foam above the CMU and ceiling. Shane and I discussed we may not need to have all the walls shown with spray foam and he is supposed to correct some. Sheet A1.1	09A 05A	See Addendum 3
13	c.The plumbing chase wall is shown as type "E" which is fire rated but it doesn't need to be. Sheet A.1	09A	See Addendum 3
14	d.Need wall designation for the laundry room chase wall. Sheet A1.1	09A	See Addendum 3
15	e.Is door 107A for the entrance into the needed? Structural plans show a lintel but both areas are exposed to the deck. A1.1	08A Doors	Yes this is needed and the wall is continuous over the door as well
16	3)Sheet A5.1 a.Are the lockers that are N.I.C. coming with an integral base or will concrete bases be needed so the resinous flooring can be rolled up?	09B Flooring	The lockers have no base and simply sit on the floor
17	4)Sheet M1.1 a.Confirm if the dryer exhaust vents through the roof. Note says it goes to a gooseneck but doesn't show penetration.	15C HVAC	See addendum 3
18	b.Where will all the mini-split condensate drains be routed? Storage room has no drains shown in the plumbing. The mechanical room split is on the west wall but the only drain is in the center of the room. Where will the ceiling cassette in the coach's office be routed?	15C HVAC	Answered in Addendum #2. (pg 47-48) New drain is to go into the mechanical room per P1.1 and P2.1
19			
20			
21			
22			
23			