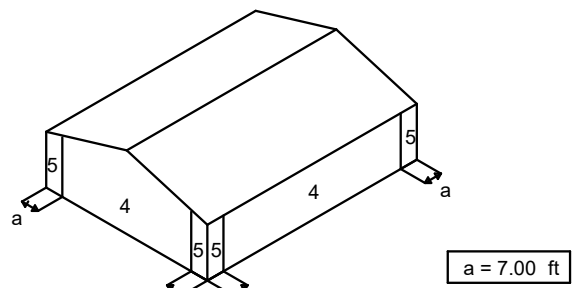


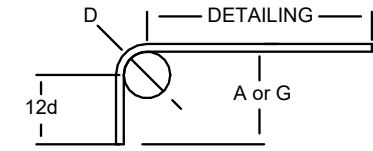
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ABBREVIATIONS	
AB	ANCHOR BOLT
ADDL	ADDITIONAL
ALT	ALTERNATE
ALUM	ALUMINUM
ARCH	ARCHITECTURAL or ARCHITECT
B	BOTTOM OF
BLDG	BUILDING
BLK	BLOCK
BLKG	BLOCKING
BM	BEAM
BOS	BOTTOM OF STEEL
BRS	BEARING
B/S	BOTTOM SIDE
BTM or (B)	BOTTOM, "BOT" SIM
BTW	BETWEEN
CB	CARRIAGE BOLT
CFS	COLD FORMED STEEL
CIP	CAST IN PLACE
CL	CONTROL SLAB
CL or (L)	CENTER LINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
C.O.	CASED OPENING
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONST	CONSTRUCTION
CONT	CONTINUE or CONTINUOUS
CSJ	CONSTRUCTION JOINT
CTR	CENTERED, "CNTR" SIM
DET	DETAIL
DIA	DIAMETER
DIA or (Ø)	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DIR	DIRECTION
D.L.	DEAD LOAD
DN	DOWN
DWG	DRAWING
DWL	DOWEL
E	EACH
E.E.	EACH END
E.F.	EACH FACE
EXP	EXPANSION JOINT
ELEV	ELEVATION
EMBED	EMBEDDED or EMBEDMENT
EDGE OF SLAB	EDGE OF SLAB
E.G.	EXISTING GRADE
E.S.	EACH SIDE
E.O.	EQUAL
E.W.	EACH WAY
EXT	EXISTING
EXPB	EXPANSION BOLT
EXT	EXTERIOR
F.F.	FAIR FACE
FIN	FINISHED FLOOR ELEVATION
F.G.	FINISHED GRADE
FIN	FINISH
FL	FLOOR
FLG	FLANGE
FND	FOUNDATION
FDC	FACE OF CONCRETE
FOM	FACE OF MASONRY
POS	FACE OF STUD
FS	FULL PENETRATION
FS	FAIR SIDE
FTG	FOOT or FEET
GA	GAUGE
GALV	GALVANIZED
GB	GRADE BEAM
GDR	GIRDER
GP	GUSSET PLATE
HAS	HEADED ANCHOR STUD
HD	HOLD DOWN
HDR	HOT DIP GALVANIZED
HOG	HEADER
HK	HOOK
HRZ	HORIZONTAL
HOB	HIGH STRENGTH BOLT
H.S.S.	HEADED SHEAR or "NELSON" STUD
HSS	HOLLOW STL. SECTION or TUBE
I.F.	INSIDE FACE
IN	INCH OR INCHES
INT	INTERIOR
INV	INVERT
JST	JOIST
JT	JOINT
K	KIP (1,000 POUNDS)
LB	POUNDS
L.E.	LEFT END
LL	LIVE LOAD
LL	LONG LEG DOWN
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
LVL	LAMINATED VENEER LUMBER
LWC	LIGHT WEIGHT CONCRETE
MAS	MASONRY
MAX	MAXIMUM
MB	MACHINE BOLT
MGR	MANUFACTURER
MID or (M)	MIDDLE
MIN	MINIMUM
MISC	MISCELLANEOUS
M.R.D.	METAL ROOF DECK
MTR	METAL
N.F.	NEAR FACE
NOM	NOMINAL
NO	NUMBER OR #
NS	NEAR SIDE
NS	NON-SHRINK
NS	NOT TO SCALE
NWC	NORMAL WEIGHT CONCRETE
O.C.	ON CENTER, "C.C." SIM
O.D.	OVERALL DIM. "OD" SIM
O.F.	OUTSIDE FACE
OPP	OPPOSITE
OVS	OVERSIZED
PCC	PRECAST CONCRETE
PEN	PENETRATION
PERP	PERPENDICULAR
P.L.	POINT LOAD
PL	PLATE
PSL	PARALLEL STRAND LUMBER
PT	POINT
P.T.	PRESSURE TREATED or POST-TENSION
P.U.	RIGHT END
REF	REINFORCING
REF	REFERENCE
RET	RETURN
REQ	REQUIRED
R.L.L.	ROOF LIVE LOAD
R.O.	ROUGH OPENING
SC	SLIP CRITICAL
SCHED	SCHEDULE
S.D.N.	STRUCTURAL DESIGN NOTES
SIM	SIMILAR
SOG	SLAB ON GRADE
S.E.	STUD STACK
SPECS	SPECIFICATIONS
SQ	SQUARE
SS	STAINLESS STEEL, "S.S." SIM
STAG	STAGGERED
STD	STANDARD
STIFF	STIFFENER
STL	STEEL
STR	STRUCTURAL, "STRUCT" SIMILAR
S.W.	SHEAR WALL
SYN	SYNTHETICAL
T or (T)	TOP
T&S	TOP AND BOTTOM
T&S	TEMPERATURE & SHRINKAGE REINF.
TOP	TOP OF
T/S	TOP SIDE
TR	THREADED ROD
TEMP.	"T&S" FOR REINF. ELSE TEMPORARY
THK	THICK or THICKNESS
TOB	TOP OF BEAM
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOS	TOP OF STEEL
TOW	TOP OF WALL
TRANS	TRANSVERSE
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE, "U.N.O." SIM
UWA	UNDER WALL ABOVE, "U.W.A." SIM
VERT	VERTICAL
W	WITH
WF	WIDE FLANGE
WP	WORKING POINT
WWF	WELDED WIRE FABRIC
WWM	WELDED WIRE MESH

COMPONENTS AND CLADDING (PER ASD)									
ROOF: (LOADING IN PSF)									
ZONE 1 (INTERIOR)			ZONE 2 (EDGE)			ZONE 3 (CORNER)			
TRIBUTARY AREA (SQFT)	(+)	(-)	(+)	(-)	(+)	(-)	(+)	(-)	
10	37.80	69.30	37.80	76.30	37.80	93.80			
20	34.56	58.77	34.56	68.20	34.56	81.65			
100	27.04	34.30	27.04	49.40	27.04	53.45			
WALLS: (LOADING IN PSF)									
ZONE 4 (INTERIOR)			ZONE 5 (EDGE/CORNER)			OVERHANG			
TRIBUTARY AREA (SQFT)	(+)	(-)	(+)	(-)	(+)	(-)	(+)	(-)	
10	41.30	44.80	41.30	55.30	41.30	105.00			
20	42.94	42.94	39.44	51.58					
100	35.12	38.62	35.12	42.94					
				9.60					



3	2	3	2	3
2	1	2	1	2
3	2	3	2	3



STRUCTURAL GENERAL NOTES

- GENERAL NOTES:
 - STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH PROJECT SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR OPENINGS, DEPRESSIONS, EQUIPMENT WEIGHTS AND LOCATIONS, EMBEDDED ITEMS AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.
 - DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.
 - NO STRUCTURAL MEMBER OR COMPONENT SHALL BE CUT, NOTCHED, OR OTHERWISE ALTERED UNLESS APPROVED IN WRITING BY THE ENGINEER OF RECORD. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL COSTS INCURRED BY THE ENGINEER OF RECORD FOR REVIEW OF ANY SUCH DEVIATIONS.
 - DO NOT SCALE DRAWINGS.
 - THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, AND TIEDOWNS.
 - DETAILS LABELED "TYPICAL DETAILS" ON THE DRAWINGS SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. THE APPLICABILITY OF THE DETAIL TO ITS LOCATION ON THE DRAWINGS CAN BE DETERMINED BY THE TITLE OF DETAIL. SUCH DETAILS SHALL APPLY WHETHER OR NOT THEY ARE REFERENCED AT EACH LOCATION. QUESTIONS REGARDING APPLICABILITY OF TYPICAL DETAILS SHALL BE DETERMINED BY THE ENGINEER OF RECORD.
 - THE GENERAL CONTRACTOR SHALL COMPARE THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, CIVIL AND STRUCTURAL DRAWINGS AND REPORT ANY DISCREPANCIES BETWEEN EACH SET OF DRAWINGS AND WITHIN EACH SET OF DRAWINGS TO THE ARCHITECT AND ENGINEER OF RECORD PRIOR TO THE FABRICATION AND INSTALLATION OF ANY STRUCTURAL MEMBERS.
 - THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, AND DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, SEQUENCE AND SCHEDULING. THE ENGINEER DOES NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, FOR THE FACTS OR CONDUCT OF THE CONTRACTOR, SUBCONTRACTOR OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
 - THE REVIEW OF SUBMITTALS AND/OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. THE CONTRACTOR ALSO SHALL BE RESPONSIBLE FOR MEANS, METHOD, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
 - PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF ATLAS ENGINEERING AND CONSULTING, LLC IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN GENERAL ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHALL NOT BE CONSIDERED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK.
 - ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE TO EXCEED LIFE SPAN AND TO ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO THE ENVIRONMENT. A PLANNED PROGRAM OF MAINTENANCE SHALL BE ESTABLISHED BY THE OWNER. THIS PROGRAM SHALL INCLUDE ITEMS SUCH AS, BUT NOT LIMITED TO, PAINTING OF STRUCTURAL STEEL, PROTECTIVE COATINGS FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS EXPOSED TO SALT ENVIRONMENT OR OTHER HARSH CHEMICALS.
 - STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR THE DESIGN OF STEEL STAIRS, HANDRAILS, CURTAIN WALL/WINDOW WALL SYSTEMS, COLD-FORMED STEEL FRAMING, OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. SUCH SYSTEMS SHALL BE DESIGNED, FURNISHED, AND INSTALLED AS REQUIRED BY OTHER PORTIONS OF THE CONTRACT DOCUMENTS.
 - ALL CONSTRUCTION SHALL CONFORM TO THE FLORIDA BUILDING CODE 8TH EDITION (2023). REFERENCE TO OTHER CODES OR STANDARD SPECIFICATIONS REFER TO THE LATEST EDITION OF SUCH CODES OR SPECIFICATIONS, UNLESS STATED OTHERWISE.
 - NO PROVISIONS HAVE BEEN MADE FOR VERTICAL OR HORIZONTAL EXPANSION EXCEPT AS SHOWN ON CONTRACT DOCUMENTS.
 - FINISH FLOOR ELEVATION (FIRST FLOOR) OF "0'-0'" IS USED AS A REFERENCE ELEVATION. SEE CIVIL DRAWINGS FOR ACTUAL ELEVATION.
 - THE USE OF REPRODUCTIONS OF THESE CONTRACT DOCUMENTS AND USE OF CAD FILES BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS IS PROHIBITED UNLESS PRIOR WRITTEN APPROVAL IS OBTAINED FROM ENGINEER OF RECORD.
 - IN THE EVENT THAT THE STRUCTURAL CONTRACTS DRAWINGS AND SPECIFICATIONS CONFLICT ON INFORMATION, THE STRUCTURAL CONTRACT DRAWINGS SHALL SUPERSEDE THE SPECIFICATIONS.
 - B) CODE DESIGN:
 - STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE.
 - GRAVITY LOADS
 - UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):
 - UNIFORM ROOF LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):
 - WIND LOADS (ASCE 7-22):
 - BASIC WIND SPEED = 153 MPH (INTERPOLATED VALUE)
 - STRUCTURAL RISK CATEGORY = II
 - WIND EXPOSURE CATEGORY = D
 - ENCLOSURE CLASSIFICATION = ENCLOSED
 - INTERNAL PRESSURE COEFFICIENT = 0.18 +/-
 - FLOOD DESIGN DATA:
 - FLOOD ZONE = VE
 - BFE = 12'
 - PANEL NO = 12081C0404J
 - ROOF RAIN LOAD DESIGN DATA:
 - RAIN LOAD = 5 PSF
 - RAIN INTENSITY (i) = 4.5 IN/HR
 - FOUNDATIONS:
 - THE FOUNDATIONS ARE DESIGNED FOR AN ANTICIPATED ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF ON COMPACTED FILL IF GEOTECHNICAL INVESTIGATION HAS NOT BEEN CONDUCTED.
 - REGARDLESS OF WHETHER OR NOT A GEOTECHNICAL INVESTIGATION IS PERFORMED, NO WARRANTIES, EXPRESSED OR IMPLIED, ARE MADE BY ATLAS ENGINEERING AND CONSULTING, LLC FOR THE PERFORMANCE OF THE FOUNDATION.
 - AT A MINIMUM, SITE PREPARATION WORK SHALL INCLUDE:
 - STRIPPING AND GRUBBING OF THE BUILDING FOOTPRINT PLUS A MARGIN OF 5 FEET AROUND THE BUILDING, REMOVING ALL ORGANIC MATERIALS.
 - PROOF ROLLING THE BUILDING SITE TO LOCATE ANY UNFORESEEN SOFT AREAS. ANY SOFT AREAS SHALL BE EXCAVATED AND REPLACED WITH CLEAN FILL. A DENSITY OF AT LEAST 95% FOR A DEPTH OF 2 FEET IS REQUIRED UNDER THE BUILDING FOOTPRINT.
 - ALL FILL SHALL BE CLEAN SAND AND FREE OF ORGANIC MATERIALS. COMPACT FILL IN 12 INCH (UNCOMPACTED THICKNESS) LIFTS TO A MINIMUM OF 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY VALUE.
 - EXCAVATIONS FOR FOUNDATIONS SHALL BE COMPACTED TO 95% FOR A DEPTH OF AT LEAST 2 FEET BELOW THE BOTTOM OF THE FOUNDATION.
 - DEWATERING MAY BE REQUIRED TO ACHIEVE THE REQUIRED COMPACTNESS VALUES, AND IF USED, SHOULD DRAW DOWN THE WATER LEVEL TO AT LEAST 2 FEET BELOW THE BOTTOM OF THE EXCAVATION.
 - SLABS ON GRADE SHALL BE PLACED OVER A 15 MIL, CLASS "B" VAPOR RETARDER. VAPOR RETARDER SHALL BE LAPPED A MINIMUM OF 6", OR AS RECOMMENDED BY THE MANUFACTURER (WHICHEVER IS GREATER) AND TAPED AT ALL JOINTS. ALL PUNCTURES IN THE VAPOR RETARDER SHALL BE REPAIRED PER MANUFACTURER'S WRITTEN INSTRUCTIONS. ALL PENETRATIONS THROUGH THE VAPOR RETARDER (COLUMNS, PLUMBING, CONDUITS, ETC) SHALL BE SEALED PER THE MANUFACTURER'S WRITTEN INSTRUCTIONS. VAPOR RETARDER SHALL BE CONTINUOUS UNDER WALL FOUNDATIONS OR SEALED TO EXTERIOR WALLS PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
 - CONCRETE:
 - CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ACI STANDARDS AND SPECIFICATIONS.
 - UNLESS NOTED OTHERWISE, ALL CONCRETE SHALL BE NORMAL WEIGHT AND HAVE THE FOLLOWING MINIMUM 28 DAY COMPRESSIVE STRENGTHS:
 - FOUNDATIONS & SLAB-ON-GRADE: 3,000 PSI
 - STRUCTURAL SLABS: 4,000 PSI
 - CIP BEAMS, COLUMNS & WALLS: 4,000 PSI
 - EXTERIOR RETAINING WALLS: 4,000 PSI
 - SUBMIT PROPOSED MIX DESIGN WITH RECENT FIELD CYLINDER OR LAB TESTS FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE.
 - CONCRETE SHALL COMPLY WITH THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED.
 - THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE.
 - CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.
 - CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE.
 - CONDUITS, PIPES AND SLEEVES SHALL BE PLACED AND SPACED IN ACCORDANCE WITH ACI 318, 20.6.
 - CONCRETE DESIGN MIX SUBMITTALS SHALL INCLUDE TESTED, STATISTICAL BACK-UP DATA AS PER 26.4.4 OF ACI 318.
 - CONCRETE SLABS ON GRADE SHALL BE REINFORCED WITH 6x6 W1.4xw1.4 STEEL MESH OR SYNTHETIC FIBERS AT A MINIMUM RATE OF 3.0 LBS/CY, OR AS RECOMMENDED BY THE FIBER MANUFACTURER FOR CONTROL OF TEMPERATURE AND SHRINKAGE/CRACKING, WHICHEVER IS GREATER, UNLESS NOTED OTHERWISE.
 - WHEN WATER-BASED ADHESIVE ARE BEING USED ON CONCRETE SURFACES, THE CONTRACTOR SHALL VERIFY THAT THE WATER CONTENT OF THE CONCRETE IS WITHIN THE ALLOWABLE RANGE BEFORE INSTALLATION.
 - CHAMFER OR ROUND ALL EXPOSED CORNERS A MINIMUM OF 3/4".
 - REINFORCING STEEL:
 - SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS.
 - REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE CLEAR COVER UNO (PER ACI 318-09 PART 7.7.1):
 - CONCRETE CAST AGAINST EARTH: 3"
 - FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: 2" (#6 BARS & LARGER) 1-1/2" (#5 BARS & SMALLER)
 - CONCRETE NOT EXPOSED TO EARTH OR WEATHER: 3/4" (SLABS) 1" (WALLS) 1-1/2" (STIRRUPS & TIES)
 - SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.
 - PROVIDE STANDARD HOOKS AT DISCONTINUOUS ENDS OF ALL TOP BARS.
 - WHERE REINFORCING IS SHOWN CONTINUOUS, SPLICE BOTTOM BARS OVER SUPPORTS AND TOP BARS AT CENTER OF SPAN. ALL OTHER LAP SPLICES SHALL BE IN ACCORDANCE WITH SPLICE TABLES AND DETAILS SHOWN ON DRAWINGS.
 - PROVIDE DOWELS INTO FOOTINGS, PILE CAPS, SUPPORT BEAMS, ETC. TO MATCH VERTICAL BARS WITH CLASS B TENSION LAP SPLICES, U.N.O.
 - WHERE HOOKS ARE SHOWN ON THE PLANS OR DETAILS, HOOKS SHALL BE DETAILED TO EXTEND DEEP ENOUGH INTO SUPPORTING STRUCTURE TO DEVELOP THE FULL STRENGTH OF THE HOOKED BAR. PROVIDE ADDITIONAL TIES OR STIRRUPS IN SUPPORTING STRUCTURE AS REQUIRED TO SATISFY ACI 318 HOOK DEVELOPMENT, CONFINEMENT, AND ANCHORAGE CRITERIA.
 - ALL REINFORCEMENT SHALL BE BENT COLD, UNLESS OTHERWISE APPROVED BY THE STRUCTURAL EOR.
 - SHOP DRAWINGS SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING REINFORCING STEEL IN MAT SLABS, CAST-IN-PLACE WALLS, AND STRUCTURAL SLABS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION.
 - ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED AND SPACED IN FORMS AND SECURED IN PLACE IN ACCORDANCE WITH THE PROCEDURES AND REQUIREMENTS FOR REINFORCED CONCRETE, "ACI 318" AND THE "MANUALS OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES," ACI 315 (LATEST EDITION).
 - ALL REINFORCING STEEL AND EMBEDMENTS SHALL BE SECURELY TIED AND SUFFICIENTLY SUPPORTED TO MAINTAIN THE POSITION WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES. "WET STICKING" DOWELS INTO CONCRETE IS NOT PERMITTED.
 - CONTINUOUS REINFORCEMENT SHALL BE PROVIDED WHEREVER POSSIBLE. REINFORCEMENT SHALL BE SPLICED ONLY AS SHOWN OR NOTED IN THE STRUCTURAL CONTRACT DOCUMENTS. WHERE POSSIBLE, USE FULL TENSION SPLICE (CLASS "B") UNLESS NOTED OTHERWISE. DOWELS SHALL MATCH THE SIZE AND SPACING OF THE SPECIFIED REINFORCEMENT AND SHALL BE LAPPED WITH FULL TENSION SPLICES (CLASS "B") UNLESS NOTED OTHERWISE. TERMINATE BARS WITH STANDARD HOOKS.
 - REINFORCING STEEL SHALL NOT BE WELDED OR TACK WELDED UNLESS APPROVED BY THE STRUCTURAL EOR.
 - ALL STEEL REINFORCING USED IN SLAB-ON-GRADE CONSTRUCTION IS REQUIRED TO BE SUPPORTED IN THE CENTER TO UPPER ONE THIRD OF THE SLAB.
- WELDED WIRE FABRIC:
 - SHALL CONFORM TO ASTM A-185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS.
 - MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES.
 - ALL WELD WIRE MESH USED IN SLAB-ON-GRADE CONSTRUCTION IS REQUIRED TO BE SUPPORTED IN THE CENTER TO UPPER ONE THIRD OF THE SLAB.
- WOOD:
 - STRUCTURAL GLUED LAMINATED TIMBER SHALL BE PRODUCED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC) MINIMUM ALLOWABLE BENDING STRESS SHALL BE 2,400 PSI (DRY CONDITIONS).
 - PROVIDE DRESSED SEASONED LUMBER, S4S, WITH A MAXIMUM MOISTURE CONTENT OF 19% AT TIME OF DRESSING AS LISTED BELOW.
 - INTERIOR AND EXTERIOR LOAD-BEARING WALLS: SOUTHERN PINE, NO. 2 GRADE.
 - LINTELS, FLOOR JOISTS AND BEAMS: SOUTHERN PINE, NO. 2 GRADE.
 - WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE FOUNDATION GRADE PRESSURE-TREATED. USE GALVANIZED NAILS IN PRESSURE-TREATED WOOD. THE PROTECTIVE COATING ON LIGHT GAUGE STEEL CONNECTIONS IN CONTACT W/ PRESSURE-TREATED WOOD SHALL BE IN ACCORDANCE WITH THE CONNECTOR MANUFACTURERS RECOMMENDATIONS.
 - ENGINEERED LUMBER PRODUCTS
 - PARALLEL STRAND LUMBER (PSL) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE STRESSES AND PROPERTIES:

ALLOWABLE BENDING STRESS	F/B = 2900 PSI
COMPRESSION PERPENDICULAR TO GRAIN	F/C _⊥ = 750 PSI
COMPRESSION PARALLEL TO GRAIN	F/C = 2500 PSI
HORIZONTAL SHEAR	F/V = 290 PSI
MODULUS OF ELASTICITY	E = 2,000,000 PSI
 - LAMINATED VENEER LUMBER (LVL) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE STRESSES AND PROPERTIES:

ALLOWABLE BENDING STRESS	F/B = 2600 PSI
COMPRESSION PERPENDICULAR TO GRAIN	F/C _⊥ = 750 PSI
COMPRESSION PARALLEL TO GRAIN	F/C = 2510 PSI
HORIZONTAL SHEAR	F/V = 285 PSI
MODULUS OF ELASTICITY	E = 2,000,000 PSI
 - LAMINATED STRAND LUMBER (LSL) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE STRESSES AND PROPERTIES:

ALLOWABLE BENDING STRESS	F/B = 1700 PSI
COMPRESSION PERPENDICULAR TO GRAIN	F/C _⊥ = 710 PSI
COMPRESSION PARALLEL TO GRAIN	F/C = 1635 PSI
HORIZONTAL SHEAR	F/V = 425 PSI
MODULUS OF ELASTICITY	E = 1,300,000 PSI
 - GLULAM BEAMS SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE STRESSES AND PROPERTIES:

ALLOWABLE BENDING STRESS	F/B = 3000 PSI
COMPRESSION PERPENDICULAR TO GRAIN	F/C _⊥ = 805 PSI
TENSION PARALLEL TO GRAIN	F/T = 1350 PSI
HORIZONTAL SHEAR	F/V = 300 PSI
MODULUS OF ELASTICITY	E = 2,100,000 PSI
 - PRESERVED GLULAM BEAMS SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE STRESSES AND PROPERTIES:

ALLOWABLE BENDING STRESS	F/B = 2400 PSI
COMPRESSION PERPENDICULAR TO GRAIN	F/C _⊥ = 740 PSI
HORIZONTAL SHEAR	F/V = 300 PSI
MODULUS OF ELASTICITY	E = 1,800,000 PSI
- STRUCTURAL PANELS
 - FLOOR PANELS SHALL BE CONSTRUCTED WITH TONGUE AND GROOVE APA RATED, EXPOSURE 1, 23/32" PLYWOOD. FLOOR PANELS SHALL BE GLUED AND NAILED W/ 10d RING SHANK NAILS @ 4" O/C AT PANEL EDGES AND AT 6" O/C IN THE FIELD.
 - WALL PANELS SHALL BE CONSTRUCTED WITH APA RATED, EXPOSURE 1, 1/2" SHEATHING. SHEATHING SHALL BE ATTACHED WITH 10d COMMON NAILS @ 3" O/C AT PANEL EDGES AND 6" O/C IN THE FIELD. ALL PANEL EDGES SHALL BE BLOCKED.
 - ROOF PANELS SHALL BE CONSTRUCTED WITH APA RATED, EXPOSURE 1, 19/32" SHEATHING. SHEATHING SHALL BE ATTACHED WITH 10d RING SHANK NAILS @ 4" O/C AT PANEL EDGES AND AT 6" O/C IN THE FIELD. ALL PANEL EDGES SHALL BE BLOCKED OR ATTACHED WITH SIMPSON PSCA PANEL SHEATHING CLIPS. NAIL HEADS SHALL NOT PENETRATE THE OUTER SURFACE OF SHEATHING.

- FABRICATED WOOD TRUSSES
 - DESIGN OF WOOD TRUSSES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS, DESIGN LOAD DATA, AND SUPPORT REACTIONS SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE, REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS WITH REGARD TO TRUSS CONFIGURATION, AND THE CONTRACTOR'S INTERPRETATION OF DESIGN LOADS AND DETAILS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN OF THE TRUSSES OR TRUSS CONNECTIONS NOT SPECIFICALLY DETAILED IN THE CONTRACT DOCUMENTS.
 - ERECTION AND TEMPORARY BRACING OF PREFABRICATED WOOD TRUSSES SHALL BE IN CONFORMANCE WITH THE RECOMMENDATIONS OF THE TRUSS MANUFACTURER AND THE TRUSS PLATE INSTITUTE'S BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS.
 - SECURE EACH COMMON ROOF TRUSS/RAFTER TO TOP PLATE WITH SIMPSON H-10 OR H-7 HURRICANE CLIP AT ALL BEARING POINTS. USE SIMPSON H-7 AT GIRDER TRUSSES. PROVIDE A MINIMUM OF TWO STUDS UNDER GIRDER TRUSS END BEARING.
 - TRUSSES ON SITE PRIOR TO INSTALLATION SHALL BE STORED IN A VERTICAL POSITION WITH SUPPORT POINTS PROVIDED AT FINAL BEARING POINTS AND BRACED TO AVOID TIPPING.
 - INSTALLATION OF ALL TRUSSES SHALL BE DONE USING A SPREADER BAR WITH A THREE POINT VERTICAL PICK AND CARE IS TO BE USED IN LIFTING TO MINIMIZE HORIZONTAL BENDING.
 - IMPROPER HANDLING OF THE TRUSSES AS NOTED ABOVE AND IN THE SPECIFICATIONS SHALL MEAN REMOVAL OF THE TRUSSES FROM THE JOB SITE.
 - DOUBLE TRUSSES SHALL BE NAILED TOGETHER W/ 10d @ 12" O.C. EACH SIDE, TOP AND BOTTOM CHORDS & WEBS.
 - TRUSS TO TRUSS CONNECTIONS SHALL BE VERIFIED BY THE TRUSS DESIGNER.
 - CONTRACTOR TO REFER TO "STANDARD FOR HURRICANE RESISTANT CONSTRUCTION SSTD 10-99 FOR FRAMING REQUIREMENTS OF WOOD FRAMED WALL SYSTEMS.
- FLOOR TRUSS LOADS:
 - FLOOR TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS UNLESS SPECIAL LOADING PATTERN IS PROVIDED BY THE STRUCTURAL ENGINEER OF RECORD.

TOP CHORD LIVE LOAD	40 PSF
TOP CHORD DEAD LOAD	20 PSF
BOTTOM CHORD DEAD LOAD	10 PSF
TOTAL	70 PSF
- ROOF TRUSS LOADS:
 - ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS UNLESS A SPECIAL LOADING PATTERN IS PROVIDED BY THE STRUCTURAL ENGINEER OF RECORD.

TOP CHORD LIVE LOAD	20 PSF
TOP CHORD DEAD LOAD	20 PSF
BOTTOM CHORD DEAD LOAD	10 PSF
TOTAL	40 PSF

NOT FOR CONSTRUCTION

Digital Signature

PRICING SET

OKALOOSA ISLAND
PRIME PROP LLC

OK ISLAND
MARINA
RESTAURANT

1345 E Highway 98 Fort
Walton Beach, FL 32548

REVISIONS
NO. DESCRIPTION DATE

Project Number 25001
Date 03 JULY 2026

WALTON COUNTY FLOOD ZONE CERTIFICATION

I CERTIFY, TO THE BEST OF MY KNOWLEDGE, THAT THE FOUNDATION AND STRUCTURE IS DESIGNED AS ADEQUATELY ANCHORED TO PREVENT FLOATAION, COLLAPSE, AND LATERAL MOVEMENT OF THE STRUCTURE RESULTING FROM HYDRODYNAMIC AND HYDROSTATIC LOADS, INCLUDING THE EFFECT OF BUOYANCY.

THE DESIGN HAS TAKEN INTO ACCOUNT THE PROVISIONS OF CHAPTER 16 OF THE FLORIDA BUILDING CODE, CHAPTER 3 OF THE FLORIDA BUILDING CODE (RESIDENTIAL), ASCE 24, AND THE FLOOD LOADS IMPOSED BY A BASE FLOOD EVENT OF A 100 YEAR FLOOD EVENT AS SHOWN ON THE CURRENT WALTON COUNTY FLOOD INSURANCE MAP AND NFIP REGULATIONS.

WALTON COUNTY DESIGN CERTIFICATION

I CERTIFY

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ABBREVIATIONS	
AB	ANCHOR BOLT
ADDL	ADDITIONAL
ALT	ALTERNATE
ALUM	ALUMINUM
ARCH	ARCHITECTURAL or ARCHITECT
B/	BOTTOM OF
BLDG	BUILDING
BLK	BLOCK
BLKG	BLOCKING
BM	BEAM
BOS	BOTTOM OF STEEL
BRG	BEARING
B/S	BOTTOM SIDE
BTM or (B)	BOTTOM, "BOT" SIM
BTW	BETWEEN
CB	CARRIAGE BOLT
CFS	COLD FORMED STEEL
CIP	CAST IN PLACE
C-J	CONTROL JOINT
CL or (L)	CENTER LINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
C.O.	CASED OPENING
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONST	CONSTRUCTION
CONT	CONTINUE or CONTINUOUS
CSJ	CONSTRUCTION JOINT
CTR	CENTERED, "CNTRD" SIM
DET	DETAIL
DBL	DOUBLE
DIA or (Ø)	DIAMETER
DIAG	DIAGONAL
DIR	DIRECTION
DIR	DIRECTION
D.L.	DEAD LOAD
DN	DOWN
DWG	DRAWING
DWL	DOWEL
EA	EACH
E.E.	EACH END
E.F.	EACH FACE
EJ	EXPANSION JOINT
ELEV	ELEVATION
EMBED	EMBEDDED or EMBEDMENT
EDGE	EDGE OF SLAB
E.G.	EXISTING GRADE
E.S.	EACH SIDE
EQ	EQUAL
E.W.	EACH WAY
EX	EXISTING
EXPB	EXPANSION BOLT
EXT	EXTERIOR
F.F.	FAR FACE
FFE	FINISHED FLOOR ELEVATION
F.G.	FINISHED GRADE
FIN	FINISH
FL	FLOOR
FLG	FLANGE
FND	FOUNDATION
FOC	FACE OF CONCRETE
FOM	FACE OF MASONRY
FOS	FACE OF STUD
FP	FULL PENETRATION
F/S	FAR SIDE
FT	FOOT OR FEET
FTS	FOOTING
GA	GAUGE
GALV	GALVANIZED
GB	GRADE BEAM
GDR	GIRDER
GP	GUSSET PLATE
HAS	HEADED ANCHOR STUD
HD	HOLD DOWN
HOG	HOT DIP GALVANIZED
HDR	HEADER
HK	HOOK
HORIZ	HORIZONTAL
HSB	HIGH STRENGTH BOLT
H.S.S.	HEADED SHEAR or "HEI-SON" STUD
HSS	HOLLOW STL. SECTION or TUBE
I.F.S.	INSIDE FACE
IN	INCH OR INCHES
INT.	INTERIOR
INV	INVERT
JST	JOIST
K	KIP (1,000 POUNDS)
LB	POUNDS
L.E.	LEFT END
LL	LIVE LOAD
LLD	LONG LEG DOWN
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
LVL	LAMINATED VENEER LUMBER
LWC	LIGHT WEIGHT CONCRETE
MAS	MASONRY
MAX	MAXIMUM
MB	MACHINE BOLT
MGFR	MANUFACTURER
MID or (M)	MIDDLE
MIN	MINIMUM
MISC	MISCELLANEOUS
M.R.D.	METAL ROOF DECK
MTL	METAL
MTRL	MATERIAL
N.F.	NEAR FACE
NOM	NOMINAL
NO.	NUMBER OR #
N/S	NEAR SIDE
NS	NON-SHRINK
N.T.S.	NOT TO SCALE
NWC	NORMAL WEIGHT CONCRETE
O.C.	ON CENTER, "O.C." SIM
O.D.	OVERALL DIA. "OD" SIM
O.F.	OUTSIDE FACE
OPP	OPPOSITE
OVRSZ	OVERSIZED
PCC	PRECAST CONCRETE
PEN	PENETRATION
PERP	PERPENDICULAR
P.L.	POINT LOAD
PL	PLATE
PSL	PARALLEL STRAND LUMBER
PT	POINT
P.T.	PRESSURE TREATED or POST-TENSION
P.U.	RIGHT END
REINF	REINFORCING
REF	REFERENCE
RET	RETURN
REQ	REQUIRED
R.L.L.	ROOF LIVE LOAD
R.O.	ROUGH OPENING
SC	SLIP CRITICAL
SCHED	SCHEDULE
S.D.N.	STRUCTURAL DESIGN NOTES
SIM	SIMILAR
SOG	SLAB ON GRADE
S.E.	STUD PACK
SPECS	SPECIFICATIONS
SQ	SQUARE
SS	STAINLESS STEEL, "S.S." SIM
STAG	STAGGERED
STD	STANDARD
STIFF	STIFFENER
STL	STEEL
STR	STRUCTURAL, "STRUCT" SIMILAR
S.W.	SHEAR WALL
SYM	SYMMETRICAL
T or (T)	TOP
T&S	TOP AND BOTTOM
T&S	TEMPERATURE & SHRINKAGE REINF.
T/	TOP OF
T/S	TOP SIDE
TDR	THREADED ROD
TEMP.	"T&S" FOR REINF. ELSE TEMPORARY
THK	THICK or THICKNESS
TOB	TOP OF BEAM
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOS	TOP OF STEEL
TOW	TOP OF WALL
TRANS	TRANSVERSE
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE, "U.N.O." SIM
UWA	UNDER WALL ABOVE, "U.W.A." SIM
VERT	VERTICAL
W/	WITH
WF	WIDE FLANGE
WP	WORKING POINT
WWF	WELDED WIRE FABRIC
WWM	WELDED WIRE MESH

- CONNECTIONS
- CONNECTIONS FOR STRUCTURAL TIMBER SHALL BE GALVANIZED STRONG TIE CONNECTORS BY THE SIMPSON COMPANY OR APPROVED EQUAL. CONNECTORS SHALL FOLLOW MANUF. CORROSION PROTECTION RECOMMENDATIONS.
- THE NUMBER OF FASTENERS PER CONNECTION SHALL BE THE MAX. ALLOWED FOR THAT PARTICULAR FASTENER.
- 8) TIE-DOWN NOTES:
- ANCHOR TIE-DOWN RODS SHALL HAVE A MINIMUM SHAFT DIAMETER OF 1/2" (ONE HALF INCH) U.N.O.
- TIE-DOWN RODS MAY BE CONNECTED DIRECTLY TO THE PLAN SPECIFIED 5/8" ANCHOR BOLTS USING A SIMPSON CNW5/8"-1/2" TRANSITION COUPLER NUT ABOVE THE REQUIRED BP3-5/8" AND 5/8" NUT ATOP THE SILL PLATE.
- TIE-DOWN DIRECT CONNECTION MAY BE USED BY UTILIZING SIMPSON SET ANCHORAGE ADHESIVE WITH MINIMUM ROD EMBEDMENT OF 6" WHEREVER CAST-IN-PLACE ANCHOR BOLTS ARE SPECIFIED FOR COUPLED EXTENSION TO ABOVE.
- ANY PLACE WHERE A TIE-DOWN LOCATION IS FILLED WITH STUDS; USE A SIMPSON LTT131 OR HDUS HOLD-DOWN CONNECTED AT THE TOP AND BOTTOM OF A MINIMUM 2PLY STUD PACK TO PRESERVE TENSION LOAD PATH TO FLOORS ABOVE AND/OR BELOW. RESUME INDICATED TIE-DOWN ROD PATH AT ENDS OF STUD PACK IF POSSIBLE.
- SIMPSON LTT131 & HDU5 HOLD-DOWNS SHALL BE ANCHORED INTO GROUT FILLED CMU WALLS OR PIERS WITH A SIMPSON SB56 OR SSTB ANCHOR BOLT WITH EMBEDMENT PER MFR. RECOMMENDATION.
- RODS SHALL INSTALLED PLUMB. THE MAXIMUM ALLOWABLE ROD DRIFT FROM PLUMB SHOULD NOT EXCEED 1.35 DEGREES OR MAXIMUM CENTERLINE OFFSET DISTANCE OF 1.75" PER FLOOR.
- TIE-DOWN RODS SHALL HAVE A SIMPSON BP-3 WITH NUT ATOP THE TERMINAL TOP PLATE. RODS AT SLOPED OR RAKED PLATES SHALL REQUIRE HILLSIDE WASHERS BETWEEN BP-3 AND NUT TO LEVEL.
- ALL RODS ARE TO BE TIGHTENED AT EACH FLOOR AND THE TOP PLATE CONNECTION AFTER ROOF IS FULLY LOADED. JUST PRIOR TO GYPSUM BOARD INSTALLATION. ALL RODS WITH BP AND NUT CONNECTIONS TO THE BOTTOM OF THE FIRST FLOOR FRAMING SHALL BE TIGHTENED AT THE END OF THE CONSTRUCTION PROCESS PRIOR TO C.O. OPTIONAL: CONTRACTOR MAY INSTALL A SIMPSON BPTUD AND RTUD TAKE-UP DEVICE AT THE TERMINAL TOP PLATE CONNECTION TO REMOVED UNWANTED SYSTEM SLACK DUE TO STRUCTURE SETTLING OR CREEP.
- 9) MASONRY WALLS:
- ALL MASONRY CONSTRUCTION SHALL CONFORM TO ACI 530/ASCE 5/TMS 402 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" AND ACI 530.1/ASCE 6/TMS 602 "SPECIFICATION FOR MASONRY STRUCTURES", LATEST EDITION.
- MASONRY UNITS SHALL MEET ASTM C-90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 1900 PSI ON THE NET AREA (Pn = 1500 PSI). MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C-270.
- GROUT SHALL BE 3000 PSI MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C-476 AND HAVE A SLUMP BETWEEN 8" AND 11" WITH WATER CM RATIO OF 0.55 MAXIMUM AND WITH 3/8" MAXIMUM AGGREGATE.
- PROVIDE HOOKED DOWELS IN FOUNDATIONS FOR VERTICAL REINFORCING ABOVE. REFER TO TABLE FOR SPLICE LENGTH.
- BLOCK CELLS SHALL BE GROUT FILLED WITH VERTICAL REINFORCING BARS AT CORNERS, INTERSECTIONS, EACH SIDE OF OPENINGS AND AS SHOWN ON THE DRAWINGS.
- DOWELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW, UNLESS NOTED OTHERWISE.
- USE METAL LATH, MORTAR OR SPECIAL UNITS TO CONFINE CONCRETE AND GROUT TO AREA AS REQUIRED.
- MASONRY SHALL BE LAID IN RUNNING BOND PATTERN UNLESS NOTED OTHERWISE. AT FILLED CELLS LAY UNITS WITH FULL BED JOINTS AROUND CELLS.
- PROVIDE 9 GAUGE GALVANIZED HORIZONTAL JOINT REINFORCING AT ALTERNATE BLOCK COURSES. LADDER TYPE IS RECOMMENDED WITH REINFORCED FILLED CELLS. PROVIDE PREFABRICATED "TEE" OR CORNER SECTIONS AT WALL INTERSECTIONS.
- CONTROL JOINTS SHALL BE CONSTRUCTED IN CONCRETE MASONRY CONSTRUCTION AT A MAXIMUM HORIZONTAL SPACING BETWEEN JOINTS OF 25'-0" AND NOT MORE THAN 12" FROM CORNERS. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS. CONSTRUCT INTERIOR CONTROL JOINTS AT A MAXIMUM HORIZONTAL SPACING OF 32'-0" OR 16'-0" FROM CORNERS. NO JOINTS SHALL BE LOCATED WITHIN 2'-0" OF STEEL BEAM BEARINGS. HORIZONTAL WALL REINFORCING SHALL BE STOPPED EACH SIDE OF CONTROL JOINTS. SEE ARCHITECTURAL DRAWINGS FOR SEALANT REQUIREMENTS AT CONTROL JOINTS.
- SUBMIT PROPOSED GROUT MIX DESIGNS FOR REVIEW PRIOR TO USE. MIX NUMBER OR OTHER POSITIVE IDENTIFICATION SHALL UNIQUELY IDENTIFY MIX.
- USE OF SUPERPLASTICIZER IS PROHIBITED.
- CELLS TO BE GROUT FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL GROUT SPACE.
- CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF CELLS TO BE GROUT FILLED IN EACH POUR IN EXCESS OF 5 FEET IN HEIGHT. AFTER INSPECTION AND BEFORE GROUTING, THE REBAR SHALL BE TIED AT THE CLEANOUTS AND THE CLEANOUTS SHALL BE SEALED.
- ANY OVERHANGING MORTAR OR OTHER OBSTRUCTION OR DEBRIS SHALL BE REMOVED FROM THE INSIDES OF SUCH CELL WALLS.
- VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 192 BAR DIAMETERS.
- ALL CELLS SHALL BE FILLED SOLIDLY WITH GROUT (U.N.O.). SAMPLE AND TEST GROUT PER ASTM C1019.
- GROUT SHALL BE POURED IN LIFTS OF 4 FEET MAXIMUM HEIGHT. GROUT SHALL BE CONSOLIDATED AT TIME OF PLACING BY VIBRATING AND RECONSOLIDATED LATER BY VIBRATING BEFORE PLASTICITY IS LOST.
- WHEN TOTAL GROUT POUR EXCEEDS 5 FEET IN HEIGHT, (HIGH LIFT GROUTING), THE GROUT SHALL BE PLACED IN 4-FOOT LIFTS WITH A MINIMUM OF A 30 MINUTE DELAY BETWEEN LIFTS. MINIMUM CELL DIMENSION SHALL BE IN ACCORDANCE WITH TABLE 5 OF ACI 530.1 (2' X 3' FOR COARSE GROUT, 12 FT. MAXIMUM POUR HEIGHT).
- WHEN THE GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZONTAL CONSTRUCTION JOINTS SHALL BE MADE BY STOPPING THE POUR OF GROUT NOT LESS THAN 1-1/2 INCH BELOW THE TOP OF THE UPPERMOST UNIT GROUTED.
- MASONRY WALLS MARKED AS "LOAD BEARING" ARE DESIGNED TO CARRY FLOOR GRAVITY LOADS AND MUST BE CONSTRUCTED TO SUPPORT THE CONCRETE FLOOR SLAB CONCURRENTLY WITH CONCRETE COLUMN CONSTRUCTION.
- MASONRY WALLS INDICATED AS "INFILL" ARE DESIGNED TO RESIST LATERAL LOADS AND MUST BE CONSTRUCTED AFTER THE CONCRETE SLAB IS CAST AND POST TENSIONING OPERATION IS COMPLETED. INFILL WALLS SHALL BE CONSTRUCTED STARTING AT THE FOUNDATION LEVEL AND WORKING UPWARD ONE LEVEL AT A TIME. DO NOT START NEXT HIGHER LEVEL OF WALL PRIOR TO COMPLETION OF WALL BELOW. ALLOW A MINIMUM OF 3 DAYS CURING FOR GROUT OF WALL BELOW PRIOR TO STARTING WALL ABOVE.
- SINGLE STORY MASONRY WALLS INDICATED AS "PARTITION WALLS" SHALL BE CAST ON THICKENED SLAB FOUNDATIONS AND ARE NOT DESIGNED TO CARRY ANY LOADS FROM THE MAIN BUILDING STRUCTURES. ISOLATE TOP OF PARTITION WALLS FROM UNDERSIDE OF CONCRETE SLAB WITH A MINIMUM 1/2" THICK COMPRESSIBLE MATERIAL.
- SUBMIT WRITTEN CONSTRUCTION SEQUENCES AND PROCEDURES PRIOR TO THE START OF MASONRY CONSTRUCTION.
- 10) CHEMICAL (ADHESIVE) ANCHORS:
- SHALL BE A TWO PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS HILTI HIT HY150, HILTI RE500, OR SIMPSON SET ADHESIVE SYSTEM, OR ENGINEER APPROVED SUBSTITUTION.
- EPOXY TYPES AND BRANDS VARY IN THEIR BOND STRENGTH AND SUITABILITY OF USE, DEPENDING ON TYPE OF LOADING, ANCHOR SPACING, ETC. WHEN A PARTICULAR TYPE OF EPOXY IS SPECIFIED IN THESE DRAWINGS, A UNIQUE CALCULATION HAS BEEN MADE BASED ON THE PROPERTIES OF THAT SPECIFIC TYPE OF EPOXY FOR THE SPECIFIC CONDITION SHOWN IN THE DETAIL. SUBSTITUTION OF EPOXY TYPE IS NOT ALLOWED WHERE DETAIL SPECIFIES ONLY ONE TYPE OF EPOXY. WITHOUT PRIOR WRITTEN APPROVAL BY THE ENGINEER OF RECORD, NOT ALL EPOXY BRANDS OR TYPES WILL BE ALLOWED AS SUBSTITUTES.
- SUBSTITUTION OF EPOXIES IN ONE CONDITION SHALL NOT BE CONSTRUED AS APPROVAL TO MAKE SIMILAR SUBSTITUTION OF EPOXIES IN OTHER DIFFERING CONDITIONS. EACH SUBSTITUTION MUST RECEIVE PRIOR WRITTEN APPROVAL BY THE ENGINEER OF RECORD.
- INSTALL IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS.
- THE MANUFACTURER'S REPRESENTATIVE SHALL TRAIN INSTALLERS.
- THE CONTRACTOR IS RESPONSIBLE TO ENSURE THAT ALL HOLE CLEAN-OUT REQUIREMENTS ARE FULLY COMPLETED BY THE INSTALLERS PRIOR TO INJECTING EPOXY INTO THE HOLES.
- NO LOAD SHALL BE APPLIED TO THE EPOXY ANCHORS UNTIL THE EPOXY HAS FULLY CURED AND HAS ACHIEVED ITS SPECIFIED STRENGTH.

STRUCTURAL GENERAL NOTES

- IF DETAIL SHOWS EPOXY ANCHORS IN SLOTTED HOLES, IT IS IMPERATIVE THAT ANY EXCESS EPOXY IS CLEANED UP FROM AROUND THE ANCHOR ROD, SO THAT IT DOES NOT INTERFERE WITH ADJUSTABILITY OF ANCHOR ROD IN SLOTTED HOLE.
- 11) MECHANICAL ANCHORS:
- SHALL BE EITHER HEAVY DUTY CONCRETE SCREW ANCHOR (SUCH AS POWERS WEDGE-BOLT, SIMPSON TITEN HD, OR HILTI HUS-H) OR WEDGE TYPE EXPANSION ANCHOR (SUCH AS POWERS POWER-STUD, SIMPSON WEDGE-ALL, OR HILTI KWIK BOLT 3).
- TYPE OF ANCHOR SHALL BE AS SPECIFIED ON THE DRAWINGS, WHILE BRAND AND MODEL OF ANCHOR MAY BE SELECTED FROM THE ABOVE LISTED ANCHORS. SUBSTITUTION ANCHORS MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVED IN WRITING BY THE ENGINEER OF RECORD PRIOR TO USE.
- IN SOME CASES OF CRITICAL LOADING OR GEOMETRIC CONDITIONS, ONLY SPECIFIC ANCHORS WILL BE ALLOWED, AS NOTED ON THE DRAWINGS. IN THESE CASES, THE SPECIFIED BRAND AND MODEL OF ANCHOR MUST BE USED.
- INSTALL IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS.
- THE MANUFACTURER'S REPRESENTATIVE SHALL TRAIN INSTALLERS.
- MINIMUM EMBEDMENT DEPTH OF 1/4" TAPCONS INSTALLED IN CONCRETE SHALL BE 1.25" AND INSTALLED INTO MASONRY SHALL BE 1.5". SELECT ANCHOR LENGTH AS REQUIRED TO ACHIEVE THE SPECIFIED MINIMUM EMBEDMENT DEPTH.
- TAPCON SCREWS MAY BE REPLACED W/ 0.157" SHANK DIAMETER PAF ANCHORS (HILTI X-U OR EQUAL) ON A 1:1 SUBSTITUTION BASIS. MINIMUM EMBEDMENT DEPTH SHALL BE 1.25" WHEN INSTALLED INTO CONCRETE OR GROUTED MASONRY. FOLLOW MANUFACTURERS' INSTALLATION RECOMMENDATIONS, MINIMUM EDGE DISTANCES, AND PLACEMENT LIMITATIONS (RELATIVE TO MORTAR JOINTS IN MASONRY).
- 12) STRUCTURAL STEEL:
- STEEL WORK SHALL BE NEW AND CONFORM TO THE ANSIAISC 360-05 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS.
- MATERIAL SHALL CONFORM TO THE FOLLOWING, EXCEPT AS NOTED:
- | | |
|-----------------------------|-------------------------------|
| WIDE FLANGE SHAPES | ASTM A992 (Fy=50 KSI) |
| ANGLES, CHANNELS AND PLATES | ASTM A36 (Fy=36 KSI) |
| RECTANGULAR HSS ASTM | A500, GRADE B (Fy=46 KSI) |
| HIGH STRENGTH BOLTS | ASTM A325 OR A490 |
| THREADED RODS | ASTM A36 (Fy=36 KSI) |
| HEAVY HEX NUTS | ASTM A563 |
| HARDENED STEEL WASHERS | ASTM F436 |
| ANCHOR RODS | ASTM F1554 GR. 36 (Fy=36 KSI) |
- CONNECTIONS:
- BOLTS SHALL BE HIGH-STRENGTH, BEARING TYPE IN SNUG TIGHT CONDITION, U.N.O. TIGHTEN BY AN AISC APPROVED METHOD.
- WELDING ELECTRODES SHALL BE PER AWS D1.1. RETURN FILLET WELDS FOR FRAMED CONNECTIONS 1/2" AT EACH END.
- FIELD CONNECTIONS SHALL BE BOLTED, EXCEPT AS NOTED OTHERWISE.
- DETAIL FLOOR AND ROOF FRAMING CONNECTIONS FOLLOWING THE REQUIREMENTS SHOWN IN THE TYPICAL CONNECTION SCHEDULES SHOWN IN THESE DRAWINGS, BASED ON THE BEAM OR GIRDER SIZE.
- FOR THE PURPOSE OF CORRECTLY INTERPRETING THE CONNECTION SCHEDULES, GIRDERS SHALL BE CONSIDERED AS ANY FLOOR OR ROOF BEAM WHICH CARRIES OTHER FLOOR OR ROOF BEAMS, OR ANY FLOOR OR ROOF BEAM WHICH CARRIES STEEL COLUMNS.
- DETAIL DIAGONAL BRACING CONNECTIONS AS SHOWN IN THE DETAILS. IF NO DETAIL IS PROVIDED, DETAIL CONNECTION TO DEVELOP THE FULL TENSION CAPACITY OF THE DIAGONAL BRACING MEMBER.
- HIGH STRENGTH BOLTS IN BEARING CONDITION SUPPORTING SIMPLE SPAN BEAMS NOT SUBJECT TO AXIAL LOADS MAY BE INSTALLED TO "SNUG TIGHT" CONDITION IF NORMAL, OR SHORT SLOTTED HOLES ARE USED. THE ENGINEER OF RECORD WILL BE THE ULTIMATE AUTHORITY IN THE USE OF "SNUG TIGHT" BOLTS. IF LONG SLOTTED OR OVERSIZED HOLES ARE USED, BOLTS MUST BE FULLY PRETENSIONED AND SLIP CRITICAL. PROPER SURFACE PREPARATION IS REQUIRED FOR SLIP CRITICAL BOLTS, INCLUDING OMISSION OF PRIMER OR FIRE PROOFING, AS APPROPRIATE.
- BOLTS SHARING LOAD WITH WELDS IN A CONNECTION SHALL BE FULLY PRETENSIONED AND SLIP CRITICAL.
- WHERE FULLY PRETENSIONED OR SLIP CRITICAL BOLTS ARE REQUIRED, TIGHTENING SHALL BE ACHIEVED USING EITHER TWIST-OFF TENSION CONTROL BOLTS OR DIRECT TENSION INDICATING WASHERS.
- ALL STRUCTURAL STEEL EXPOSED TO EXTERIOR CONDITIONS SHALL BE HOT DIPPED GALVANIZED PER ASTM A123 AND ALL FASTENERS AND HARDWARE SHALL BE HOT DIPPED GALVANIZED PER ASTM A153.
- GROUT UNDER BEARING PLATES SHALL BE NON-METALLIC, NON-SHRINK TYPE WITH A COMPRESSIVE STRENGTH OF AT LEAST 5,000 PSI IN 28 DAYS.
- THE CAMBER OF STEEL MEMBERS SHALL BE VERIFIED IN THE SHOP AND THE FIELD. WHEN NO CAMBER IS INDICATED, TURN THE MEMBER NATURAL CAMBER UP.
- APPLY FIREPROOFING TO STEEL STRUCTURE CALCULATING THE THICKNESS OF FIREPROOFING BY COMPARING THE ACTUAL MEMBER SIZE TO THE MEMBER SIZE USED IN THE DESIGNATED UL RATING AND ADJUSTING APPROPRIATELY.
- 13) RAILINGS AND HANDRAILS:
- ENGINEERED RAILING SYSTEM AND CONNECTION OF SAME TO THIS STRUCTURE SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA.
- THE CONFIGURATION OF THE RAILING SYSTEM SHALL BE AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- RAILING SYSTEM AND CONNECTIONS SHALL BE DESIGNED FOR APPLICABLE LOADS AS INDICATED ON THE DRAWINGS AND IN THE BUILDING CODE.
- THE LOADS SHALL BE CLEARLY INDICATED ON SHOP DRAWINGS AND SHALL COMPLY WITH ALL APPLICABLE CODES.
- SHOP DRAWINGS SHALL SHOW AND SPECIFY CONNECTIONS UTILIZED WITHIN THE RAILING SYSTEM AS WELL AS CONNECTIONS TO AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM SHOWN ON THESE DRAWINGS.
- SHOP DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA.
- 14) STRUCTURAL SUBMITTALS
- SHOP DRAWINGS OR REPORTS FOR THE FOLLOWING ITEMS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD PRIOR TO FABRICATION OR CONSTRUCTION (AS APPLICABLE) U.N.O.
- PILE INSTALLATION QUALITY CONTROL
 - PILE INSTALLATION MONITORING LOG
 - CONCRETE ADMIXTURES
 - CONCRETE STRENGTH TESTS
 - CONCRETE DETAILING
 - ENGINEERED FILL COMPACTION TESTING
 - ENGINEERED ROOF AND FLOOR TRUSSES
 - STEEL JOISTS
 - STRUCTURAL STEEL
 - HANDRAILS AND STAIRS

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PRICING SET

OKALOOSA ISLAND
PRIME PROP LLC

OK ISLAND
MARINA
RESTAURANT

1345 E Highway 98 Fort
Walton Beach, FL 32548

REVISIONS
NO. DESCRIPTION DATE

Project Number 25001
Dated 03 JULY 2026

STRUCTURAL
GENERAL NOTES

S002

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FOUNDATION LEGEND

- DENOTES 10" DIA. TIP ROUND TIMBER PILE w/ PILE CAP PER PLAN & 8"x16" CMU PIER
- DENOTES 10" DIA. TIP ROUND TIMBER PILE.
EMBEDMENT DEPTH = 25 FT (MIN.)
REQUIRED CAPACITY =
COMPRESSIVE = 12.0 TONS
TENSION = 4.9 TONS
QUANTITY = 103 (CONTRACTOR TO CONFIRM)

FOUNDATION TIMBER & HARDWARE NOTES

1. PROVIDE DRESSED SEASONED LUMBER, S4S, 21TH A MAXIMUM MOISTURE CONTENT OF 19% AT TIME OF DRESSING AS LISTED BELOW.

2. ALL DIMENSIONAL LUMBER TO BE #2 SOUTHERN YELLOW PINE OR EQUAL.

3. ALL PILES TO BE TREATED TO CCA 2.5 SPECIFICATIONS OR BETTER.

4. ALL DIMENSIONAL LUMBER TO BE TREATED TO CCA 0.80 SPECIFICATIONS OR BETTER.

5. ALL HARDWARE TO CONSIST OF HOT-DIPPED GALVANIZED OR STAINLESS STEEL.

6. CONTRACTOR TO INSTALL WASHER UNDER HEAD OF BOLTS & NUTS (EXCLUDING CARRIAGE BOLTS).

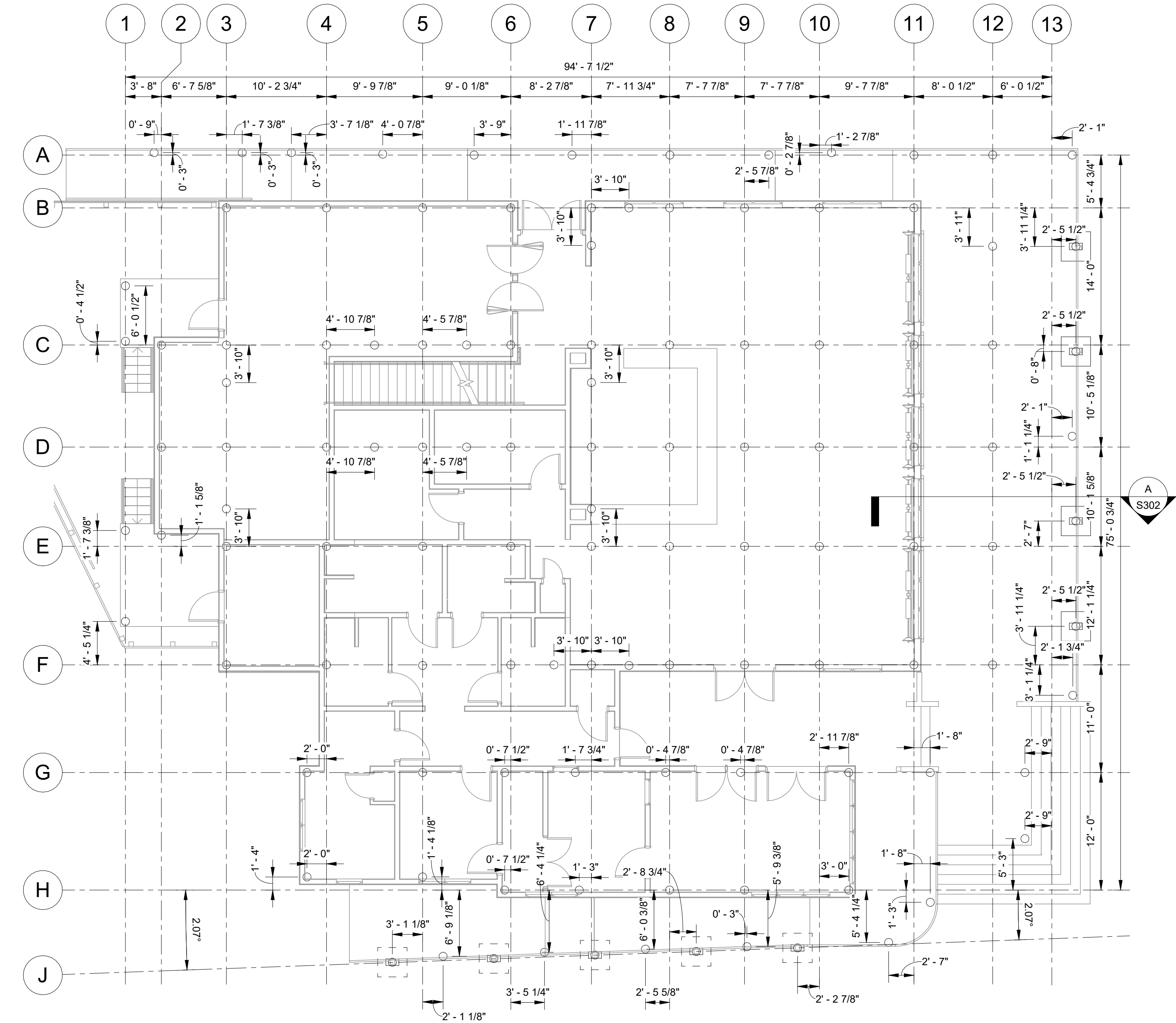
7. GRADES:

7.1 HIGH STRENGTH BOLTS: ASTM A325

7.2 HEAVY HEX NUTS: ASTM A563

7.3 HARDENED STEEL WASHERS: ASTM F436

7.4 STAINLESS STEEL SUBSTITUTION: 316



A PILING LAYOUT PLAN
1/8" = 1'-0"

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OKALOOSA ISLAND
PRIME PROP LLC

OK ISLAND
MARINA
RESTAURANT

1345 E Highway 98 Fort
Walton Beach, FL 32548

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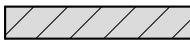
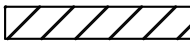
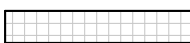
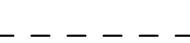
Project Number 25001
Dated 03 JULY 2026

PILING LAYOUT
PLAN

S101

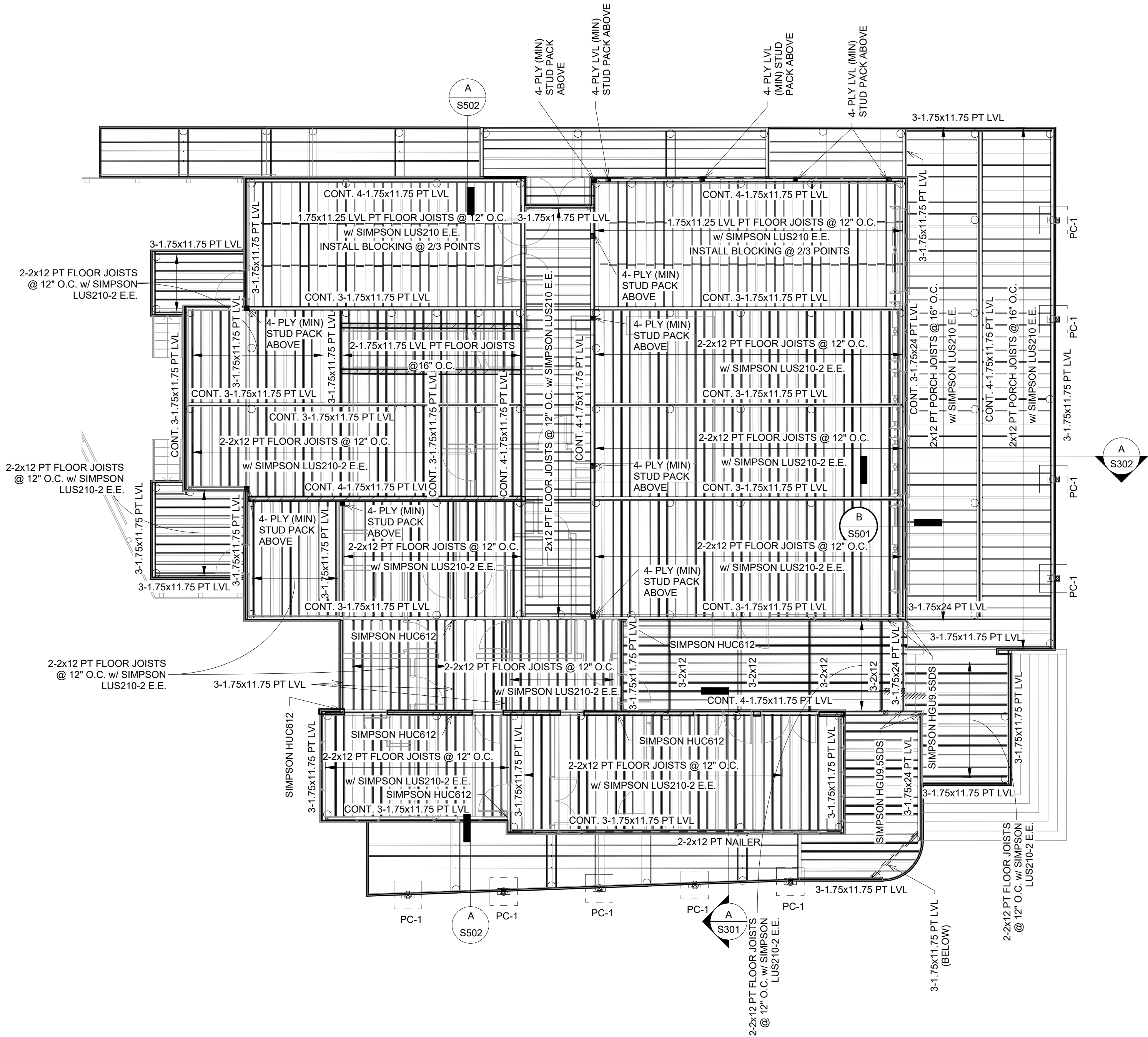
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FRAMING LEGEND

-  - DENOTES LOAD BEARING ELEMENT BELOW.
-  - DENOTES LINTEL OR HDR AS NOTED ON PLAN.
-  - DENOTES AREA OF 2x10 SISTERED RAFTERS.
-  - DENOTES 1/2" DIA. ALL-THREAD HOLDOWN @ EA. SIDE OF ALL OPENINGS. 1'-0" OR LESS FROM END WALLS, & 6'-0" O.C. THEREAFTER
-  - DENOTES EDGE OF FOUNDATION ELEMENT BELOW.
-  - DENOTES 10" DIA. TIP ROUND TIMBER PILE. EMBEDMENT DEPTH = 25 FT (MIN.) REQUIRED CAPACITY = COMPRESSIVE = 12.0 TONS TENSION = 4.9 TONS QUANTITY = 103 (CONTRACTOR TO CONFIRM)

FLOOR FRAMING NOTES

- SEE ARCH. FOR ALL DIMENSIONS & CONDITIONS NOT SHOWN.
- CONTRACTOR SHALL COORDINATE ALL MECHANICAL UNIT LOCATIONS, SIZES, OPENINGS, ETC. WITH MECHANICAL DRAWINGS.
- EACH MEMBER OF MULTI-PLY BEAMS AND GIRDERS SHALL BE LAMINATED TOGETHER WITH 10d NAILS @ 12" O.C. (MIN.)
- BEAM PLY SPLICES SHALL OCCUR ATOP THE CENTER LINE OF SUPPORT.
- ALL LUMBER EXPOSED TO WEATHER TO BE PRESSURE TREATED (P.T.), (U.N.O.).
- ALL COMPONENTS AND FASTENERS SHALL BE SIMPSON STRONG TIE OR EQUIVALENT PRE-APPROVED BY EOR.
- INSTALLATION OF EACH COMPONENT SHALL COMPLY WITH THE MFGR RECOMMENDATIONS. DO NOT MODIFY ANY COMPONENT.
- ALL FASTENER TYPE FOR EACH COMPONENT SHALL COMPLY WITH THE MFGR MINIMUM FASTENING REQUIREMENTS UNLESS SPECIFIED OTHERWISE ON THE PLANS. ALL ROUND, OBROUND AND HEXAGONAL HOLES IN EACH COMPONENT MUST BE FILLED WITH THE APPROPRIATE FASTENER. U.N.O. ALL NAILS SPECIFIED ARE COMMON NAILS.
- ALL SIMPSON HANGERS, STRAPS AND FASTENERS ON FIRST FLOOR FRAMING MUST BE STAINLESS STEEL (TYPE 316L UNLESS PRE-APPROVED BY EOR). USE STAINLESS STEEL NAILS WITH STAINLESS STEEL HANGERS AND STRAPS.
- ALL SPECIFIED POINT LOADS ARE DERIVED FROM FRAMING ABOVE ONLY & SHOULD BE USED IN CONJUNCTION WITH STANDARD LOADING OF CURRENT FLOOR.



A FIRST FLOOR FRAMING PLAN
1/8" = 1'-0"

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OKALOOSA ISLAND
PRIME PROP LLC

**OK ISLAND
MARINA
RESTAURANT**

1345 E Highway 98 Fort
Walton Beach, FL 32548

REVISIONS		
NO.	DESCRIPTION	DATE

Project Number	25001
Dated	03 JULY 2026

FIRST FLOOR
FRAMING PLAN

S102

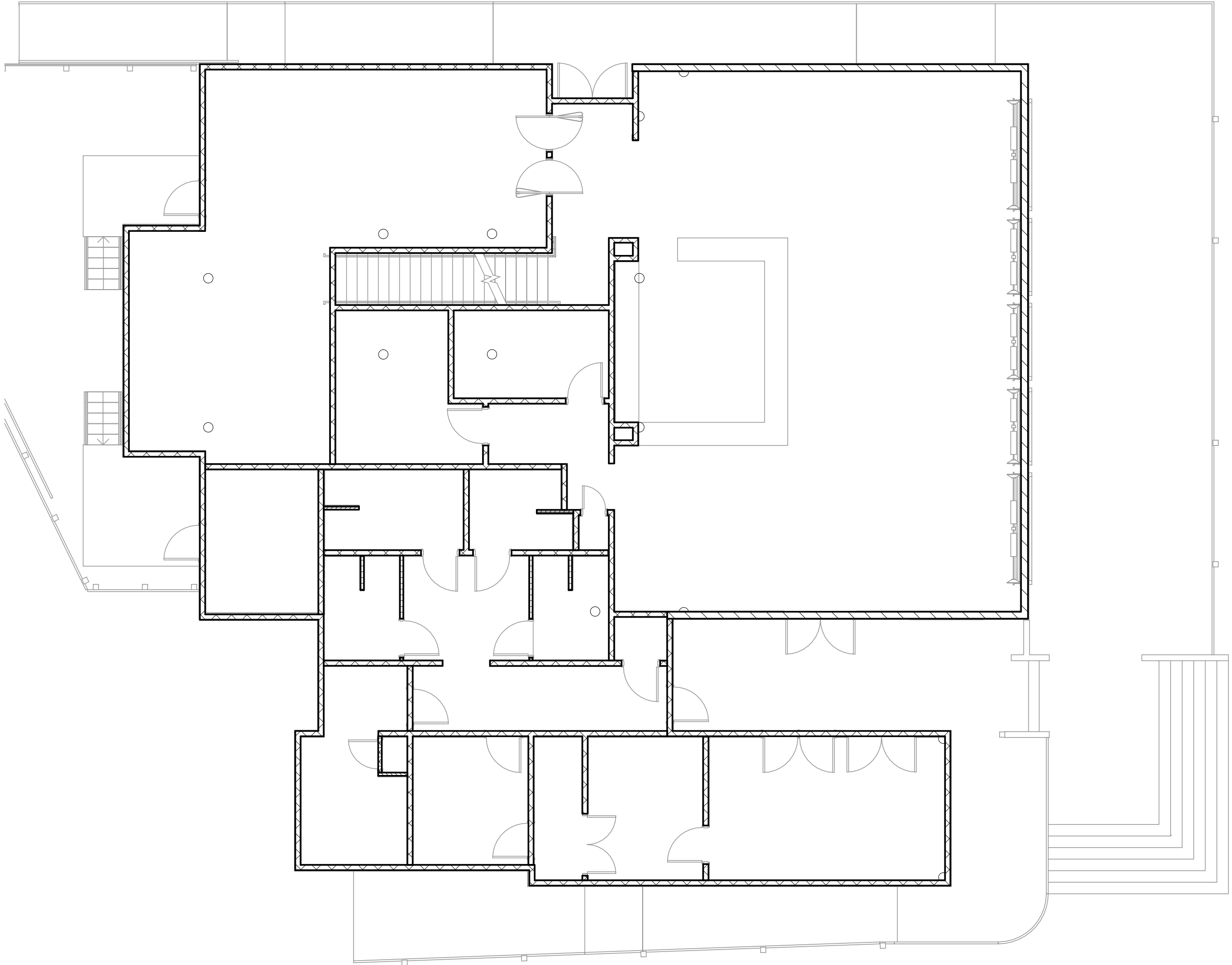
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WALL LEGEND

- DENOTES 4" STUD FRAMING.
- DENOTES 6" STUD FRAMING.
- DENOTES 6" STUD FRAMING w/ 1 HR. FIRE RATING.
- DENOTES 8" STUD FRAMING.

JAMB STUD FRAMING SCHEDULE - 2x6 WALL				
	UP TO 3'-0" R.O.	3'-0" TO 6'-0" R.O.	6'-1" TO 10'-0" R.O.	10'-1" TO 13'-0" R.O.
FIRST FLOOR	2 KING, 1 JACK	3 KING, 2 JACK	4 KING, 3 JACK	5 KING, 4 JACK

JAMB STUDS FRAMING SCHEDULE - 2x8 WALL				
	UP TO 3'-0" R.O.	3'-1" TO 6'-0" R.O.	6'-1" TO 10'-0" R.O.	10'-1" TO 13'-0" R.O.
FIRST FLOOR	2 KING, 2 JACK	3 KING, 2 JACK	4 KING, 3 JACK	5 KING, 4 JACK



A WALL FRAMING PLAN
1/8" = 1'-0"

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OKALOOSA ISLAND
PRIME PROP LLC

**OK ISLAND
MARINA
RESTAURANT**

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Project Number	25001
Dated	03 JULY 2026

WALL FRAMING
PLAN

S103

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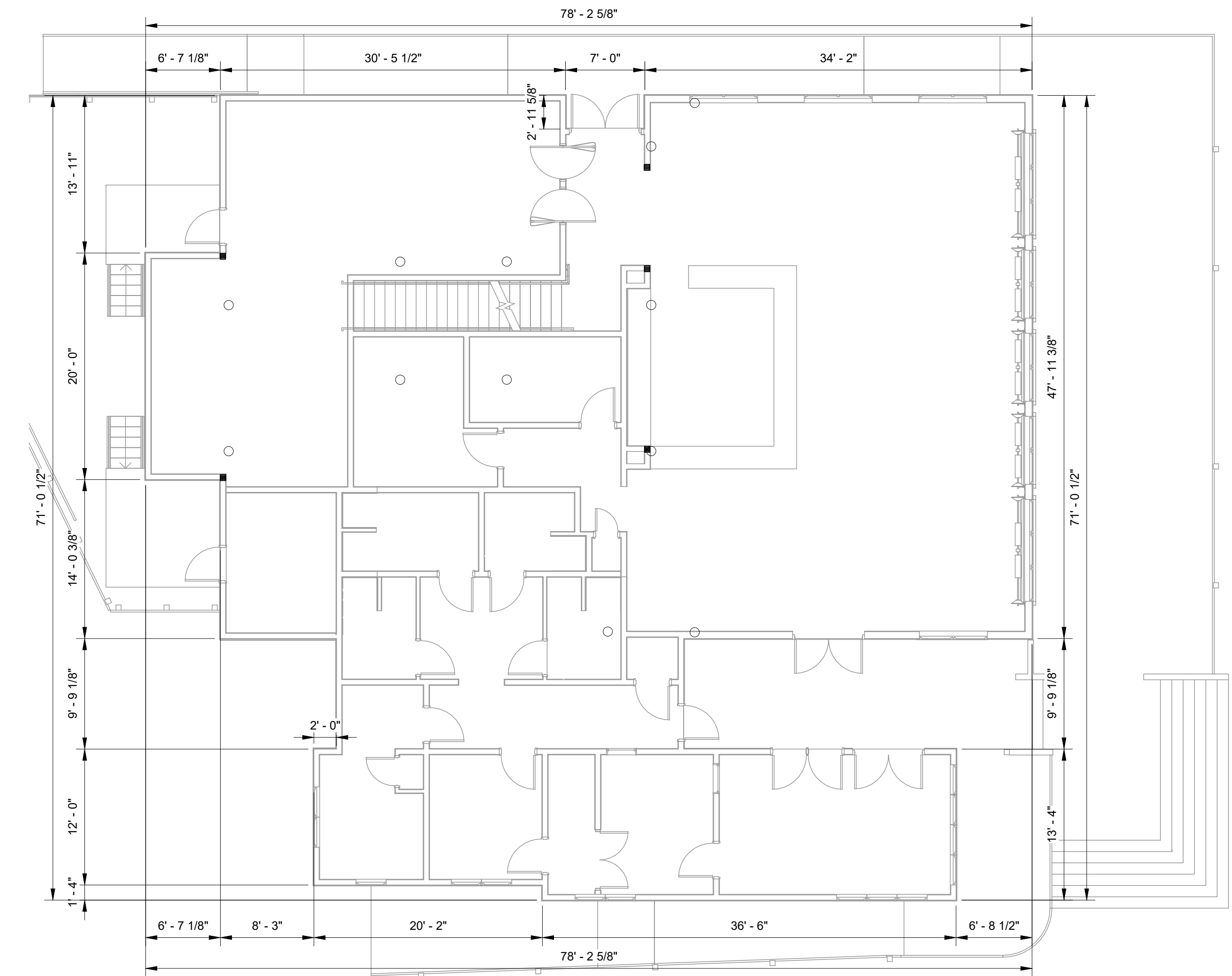
RICING SET

345 E Highway 98 Fort
Walton Beach, FL 32548

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Dated 03 JULY 2026

S104

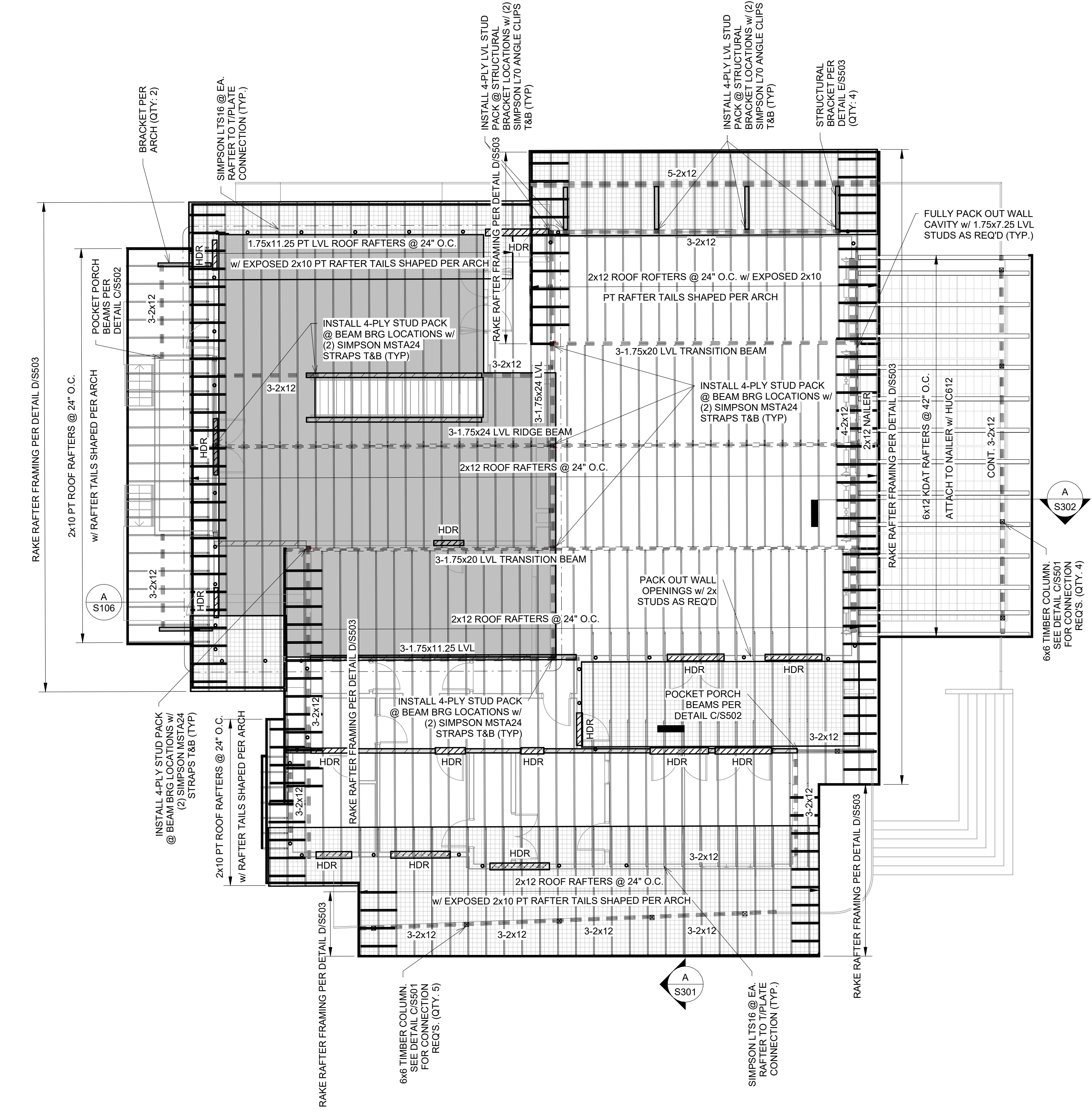


(A) DIMENSIONING PLAN
1/8" = 1'-0"

	-	DENOTES LOAD BEARING ELEMENT BELOW
	-	DENOTES LINTEL OR HDR AS NOTED ON PLAN.
	-	DENOTES AREA OF 2x10 SISTERED RAFTERS.
	-	DENOTES 1/2" DIA. ALL-THREAD HOLDDOWN @ EA. SIDE OF ALL OPENINGS, 1'-0" OR LESS FROM END WALLS, & 6'-0" O.C. THEREAFTER
	-	DENOTES EDGE OF FOUNDATION ELEMENT BELOW.
	-	DENOTES 10" DIA. TIP ROUND TIMBER PILE. EMBEDMENT DEPTH = 25 FT (MIN.) REQUIRED CAPACITY = COMPRESSIVE = 12.0 TONS TENSION = 4.9 TONS QUANTITY = 103 (CONTRACTOR TO CONFIRM)

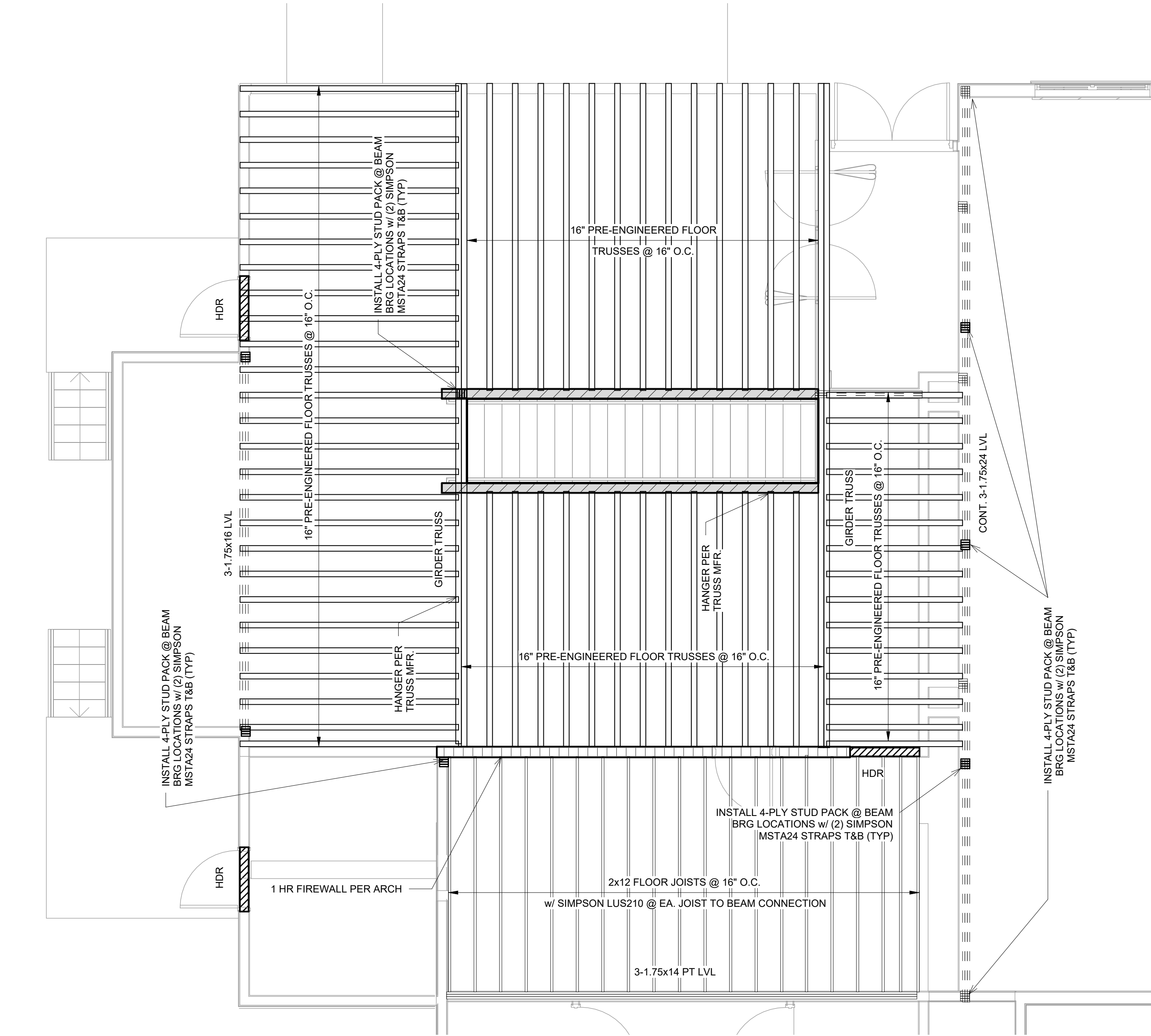
1. SEE ARCH. FOR ALL DIMENSIONS & CONDITIONS NOT SHOWN.
2. CONTRACTOR SHALL COORDINATE ALL MECHANICAL UNIT LOCATIONS, SIZES, OPENINGS, ETC. WITH MECHANICAL DRAWINGS.
3. PROVIDE FULL WIDTH CONTINUOUS STUD PACKS BELOW ALL BEAMS AND GIRDERS. (U.N.O.).
4. EACH MEMBER OF MULTI-PLY STUD PACKS SHALL BE LAMINATED TOGETHER WITH 10d NAILS @ 9" O.C. (MIN.)
5. EACH MEMBER OF MULTI-PLY BEAMS AND GIRDERS SHALL BE LAMINATED TOGETHER WITH 10d NAILS @ 12" O.C. (MIN.)
6. BEAM PLY SPLICES SHALL OCCUR WITHIN 1'-0" FROM THE CENTER LINE OF SUPPORT.
7. ALL EXTERIOR WALLS SHALL BE 100% SHEATHED. REFER TO SHEET S001 FOR ADDITIONAL SHEATHING REQUIREMENTS. .
8. FOR HEADER SIZES NOT NOTED, REFER TO TYPICAL DETAIL SCHEDULE.
9. BALLOON FRAME ALL GABLE END WALLS. (U.N.O.).
10. MULTIPLE STUD COLUMNS (STUD PACKS) SHALL MATCH WIDTH OF BEARING MEMBER BE CONTINUOUS THROUGH ZONE OF FLOOR TRUSSES VIA MATCHING BLOCKING. STRAP EA. END w/ (2) SIMPSON MTS24 STRAPS @ WOOD CONNECTIONS & SIMPSON HDU2 @ CONCRETE CONNECTIONS. (U.N.O.).
11. GIRDER TRUSSES SHALL NOT BE LOCATED OVER WINDOW OR DOOR OPENINGS UNLESS APPROVED BY THE ENGINEER OF RECORD.
12. ALL LUMBER EXPOSED TO WEATHER TO BE PRESSURE TREATED (P.T.). (U.N.O.).
13. ALL 12'-0" - 13'-6" 2x6 STUDS WITHIN 6'-0" OF CORNERS TO BE SPACED @ 12" O.C. ANY LENGTH GREATER THAN 13'-6" WILL REQUIRE E.O.R APPROVAL.

WINDOW & DOOR HEADER SCHEDULE - 2x6 WALL				
LOCATION	MAXIMUM CLEAR SPAN LENGTH			
	UP TO 3'-0" R.O.	3'-1" TO 6'-0" R.O.	6'-1" TO 9'-0" R.O.	GREATER THAN 9'-0"
FIRST FLOOR	3-2x8	SEE PLAN	SEE PLAN	SEE PLAN



(A) ROOF FRAMING PLAN
1/8" = 1'-0"

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MECHANICAL PLATFORM FRAMING PLAN
1/4" = 1'-0"

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OKALOOSA ISLAND
PRIME PROP LLC

**OK ISLAND
MARINA
RESTAURANT**

1345 E Highway 98 Fort
Walton Beach, FL 32548

NO.	DESCRIPTION	DATE
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Project Number	25001
Dated	03 JULY 2026

**MECHANICAL
PLATFORM
FRAMING PLAN**

S106

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PRIME PROP LLC

OK ISLAND MARINA RESTAURANT

345 E Highway 98 Fort
Walton Beach, FL 32548

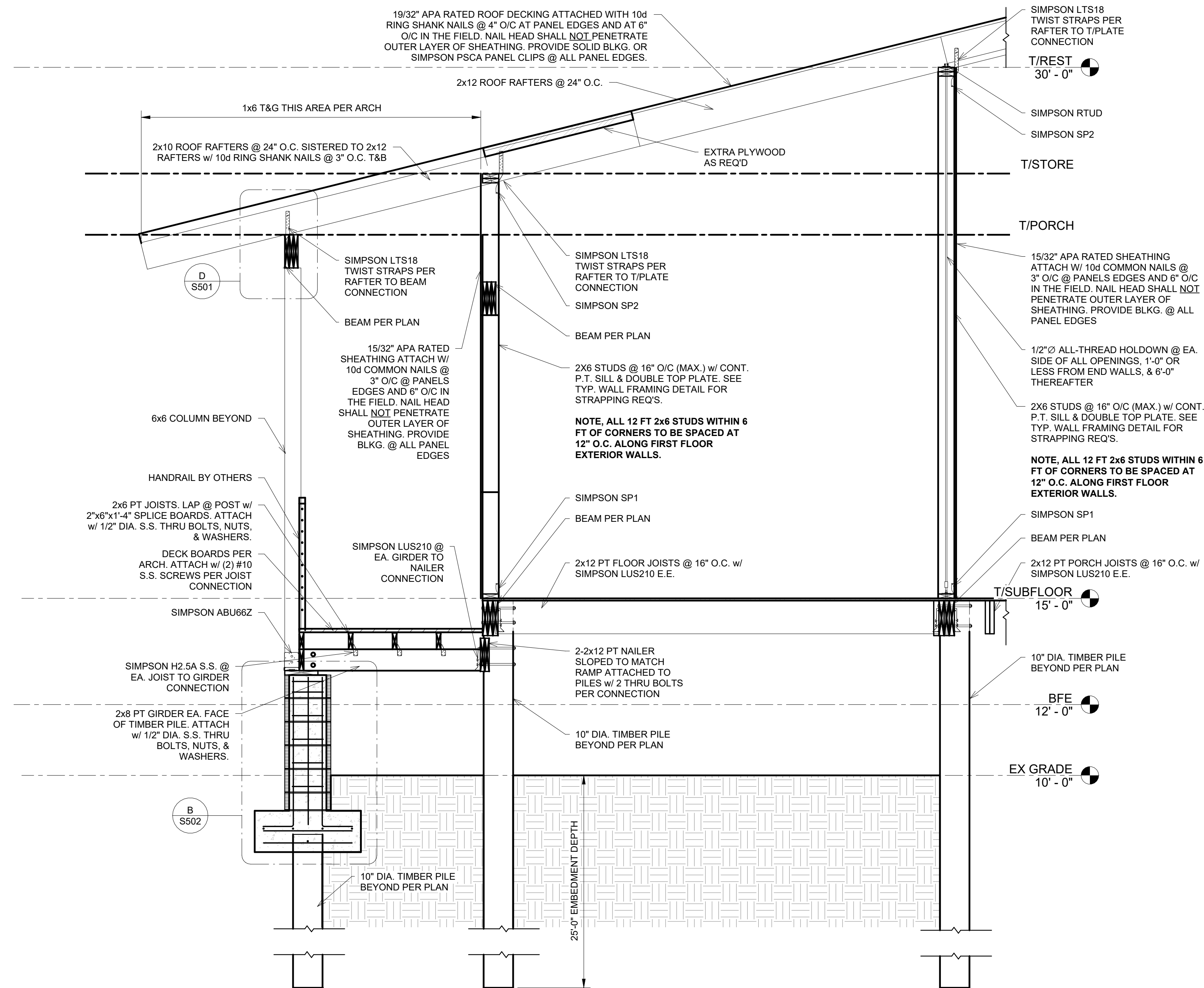
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NO. DESCRIPTION	DATE

Project Number 25001

Dated 03 JULY 2026

STRUCTURAL SECTIONS

S301



(A) TYP. WALL SECTION
1/2" = 1'-0"

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OKALOOSA ISLAND
PRIME PROP LLC

BOOK ISLAND MARINA RESTAURANT

345 E Highway 98 Fort
Walton Beach, FL 32548

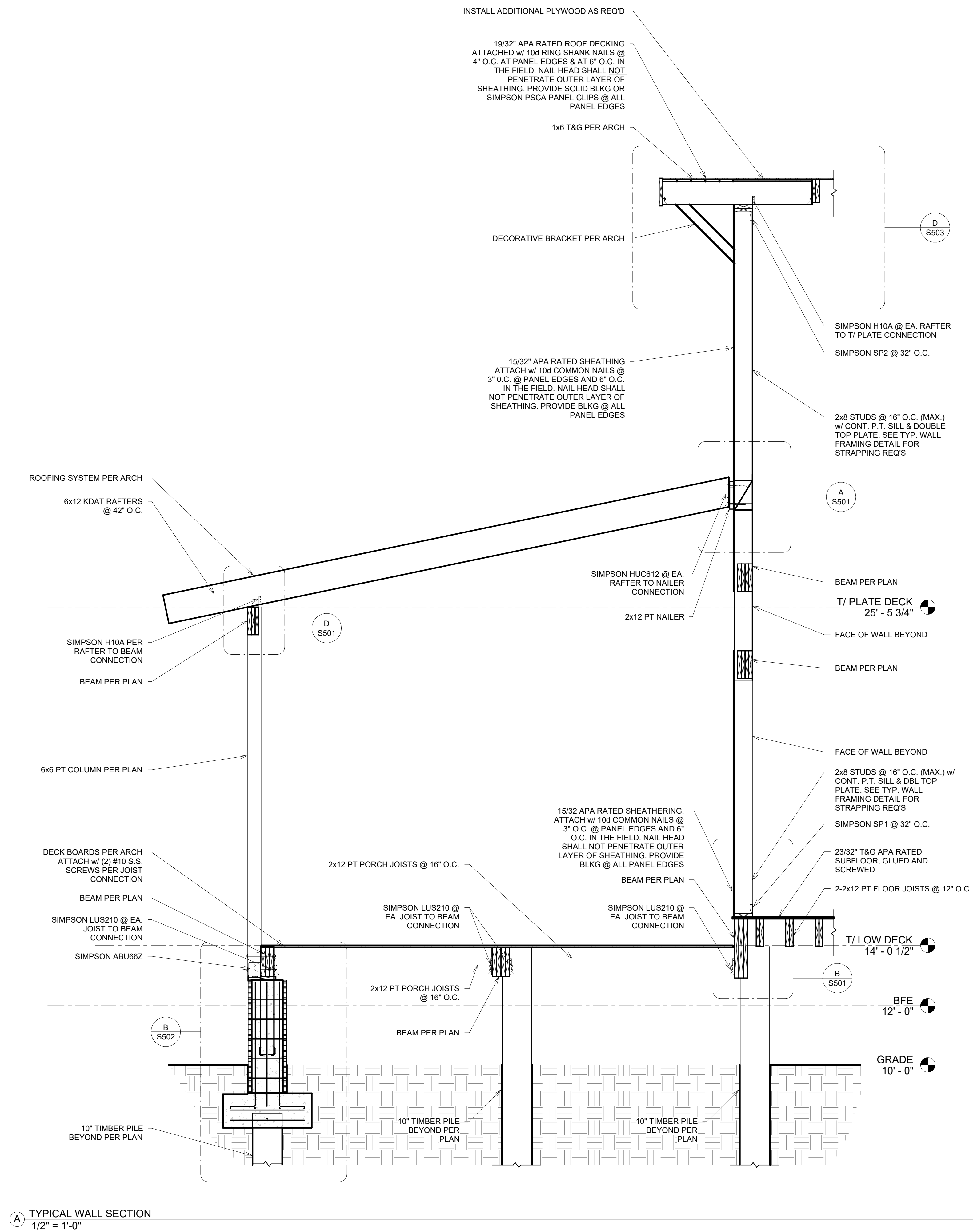
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NO.	DESCRIPTION DATE

Project Number 25001

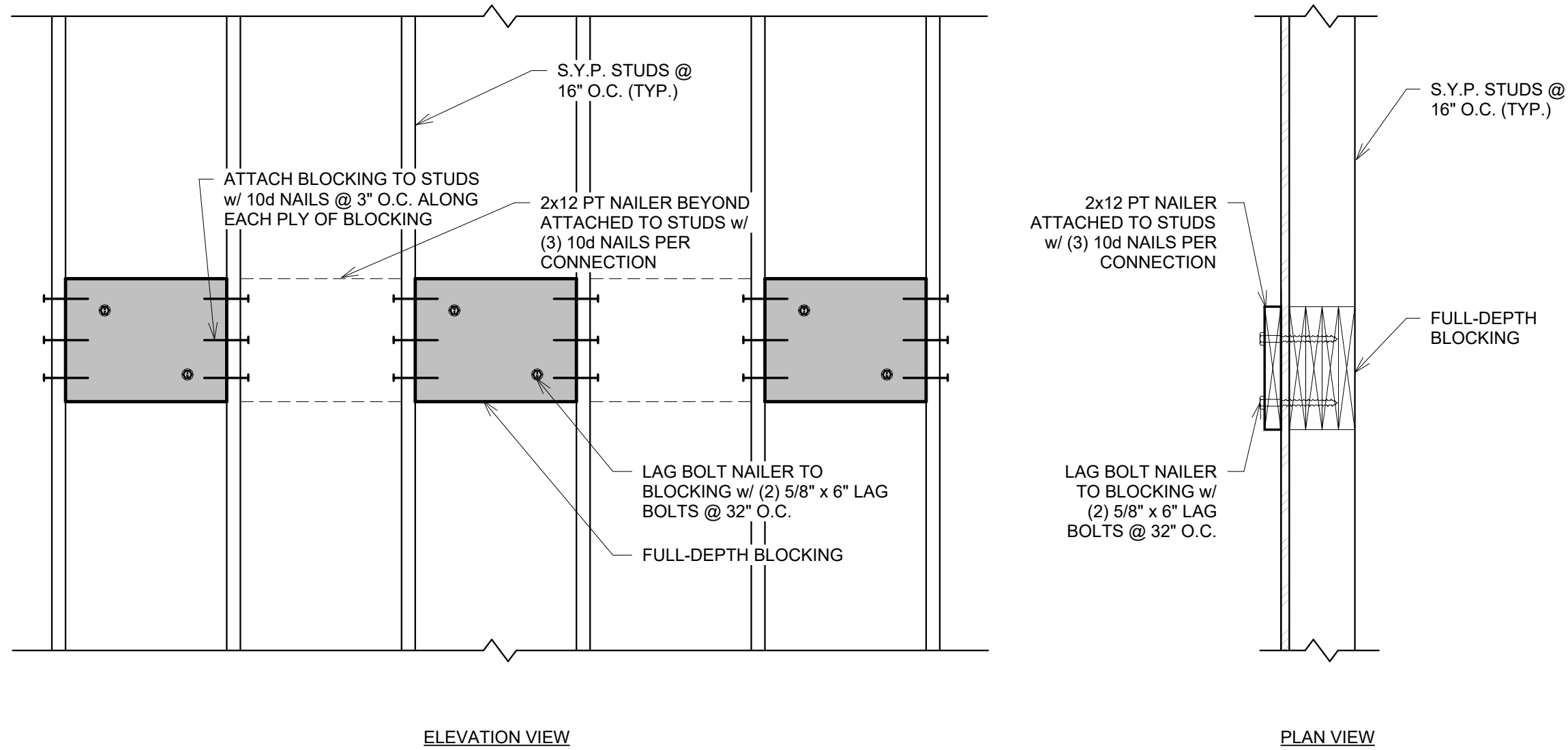
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STRUCTURAL SECTIONS

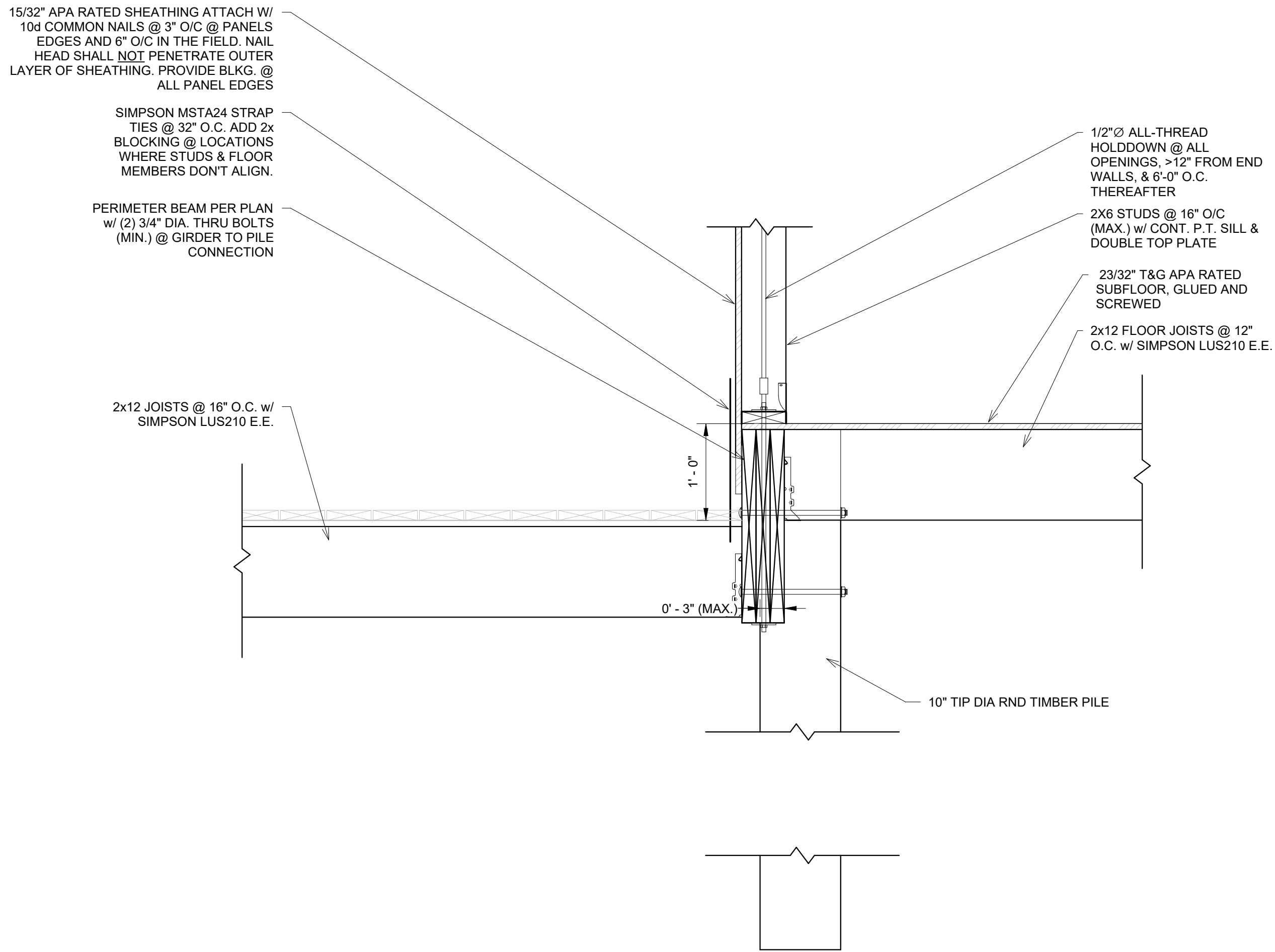
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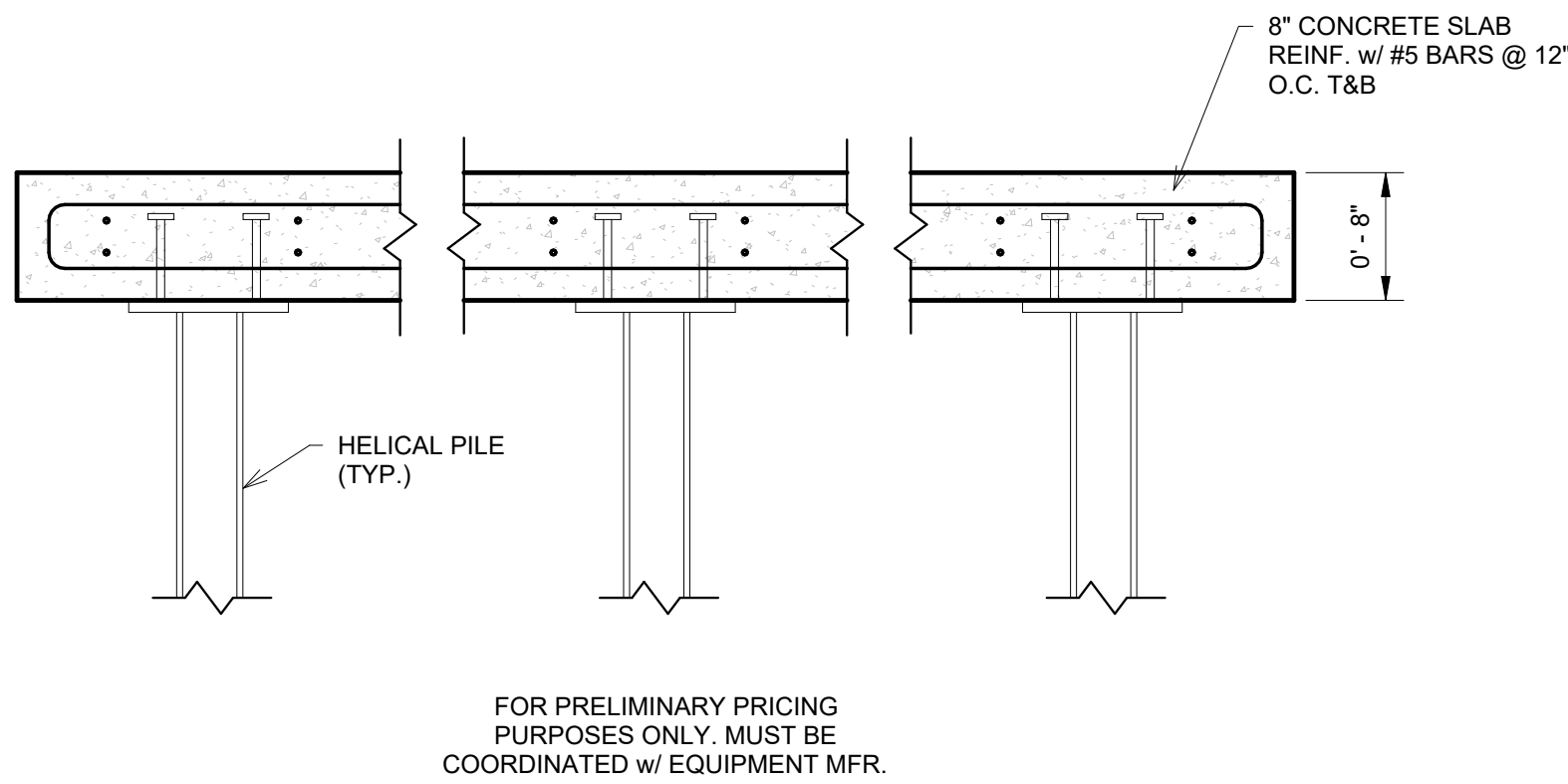
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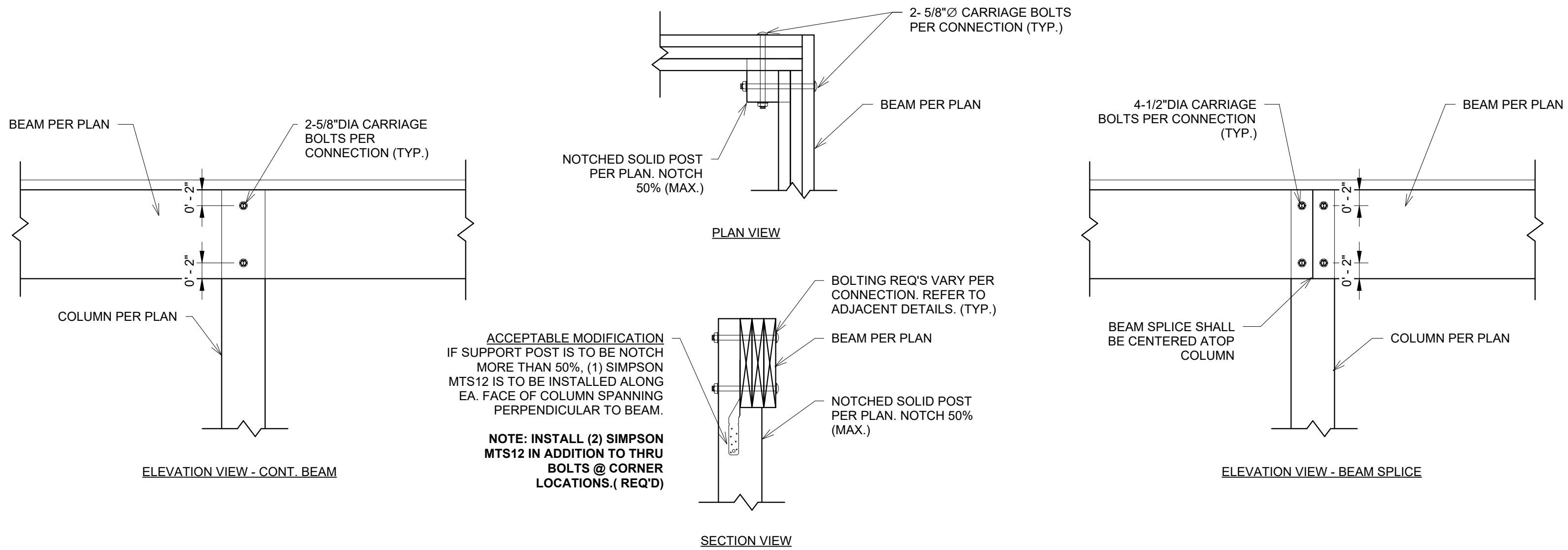
A TYP. NAILER w/ FULL DEPTH BLOCKING DETAIL
1" = 1'-0"



B TYP. TIMBER PILE STEP DOWN DETAIL
1" = 1'-0"

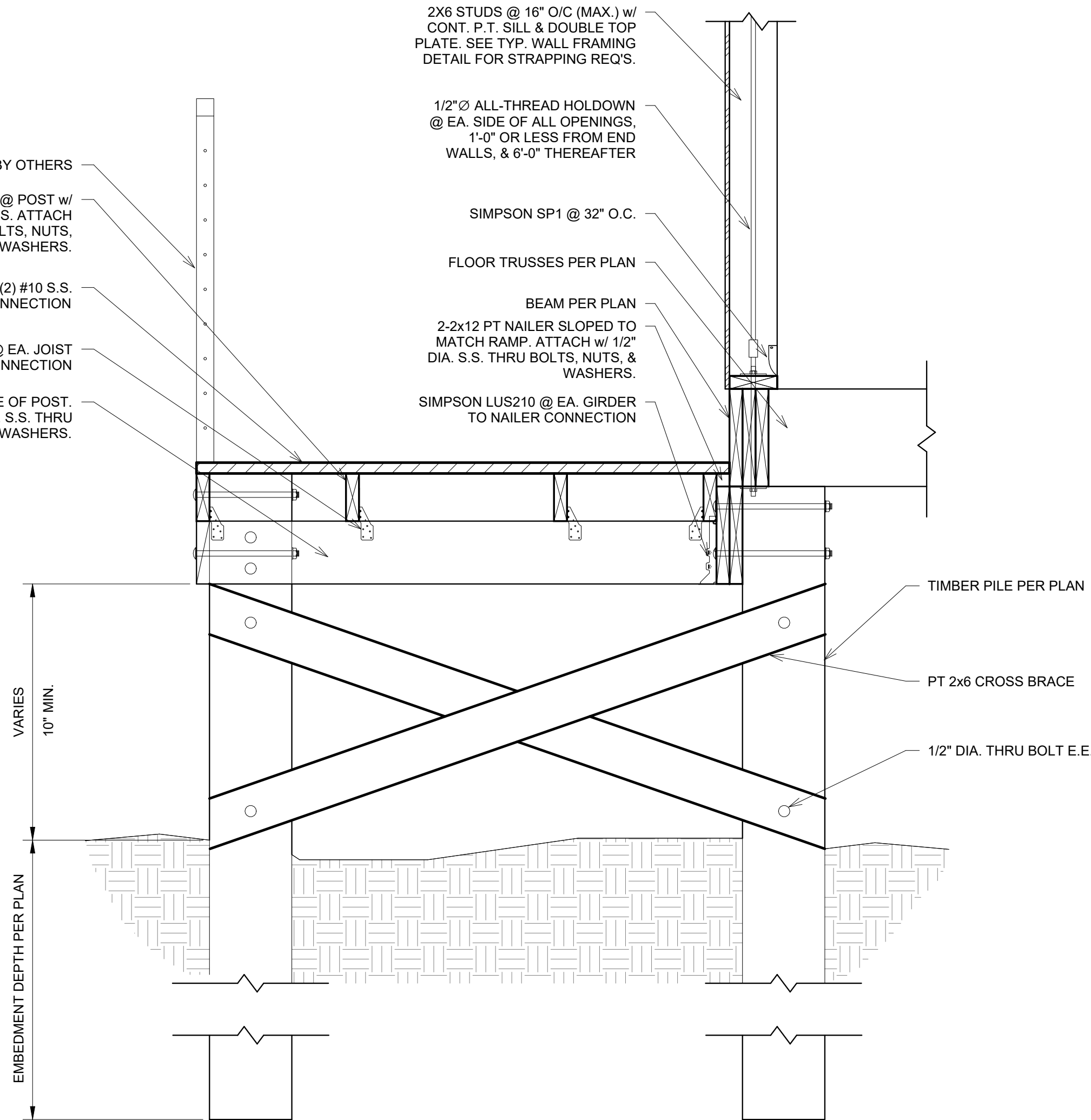


C TYP. PILE SUPPORTED SLAB
1" = 1'-0"

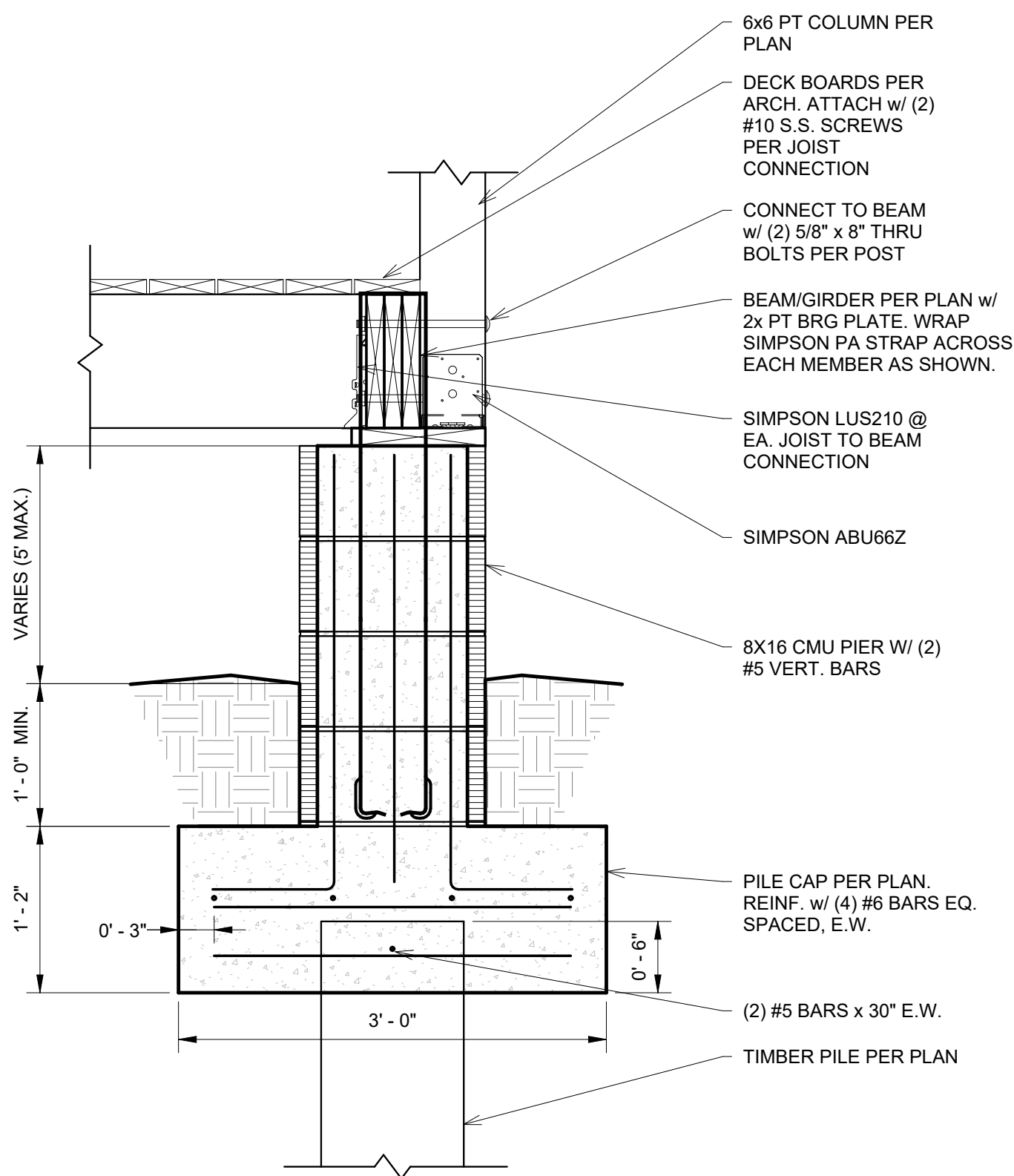


D TYP. BEAM TO COLUMN CONNECTION DETAIL
1" = 1'-0"

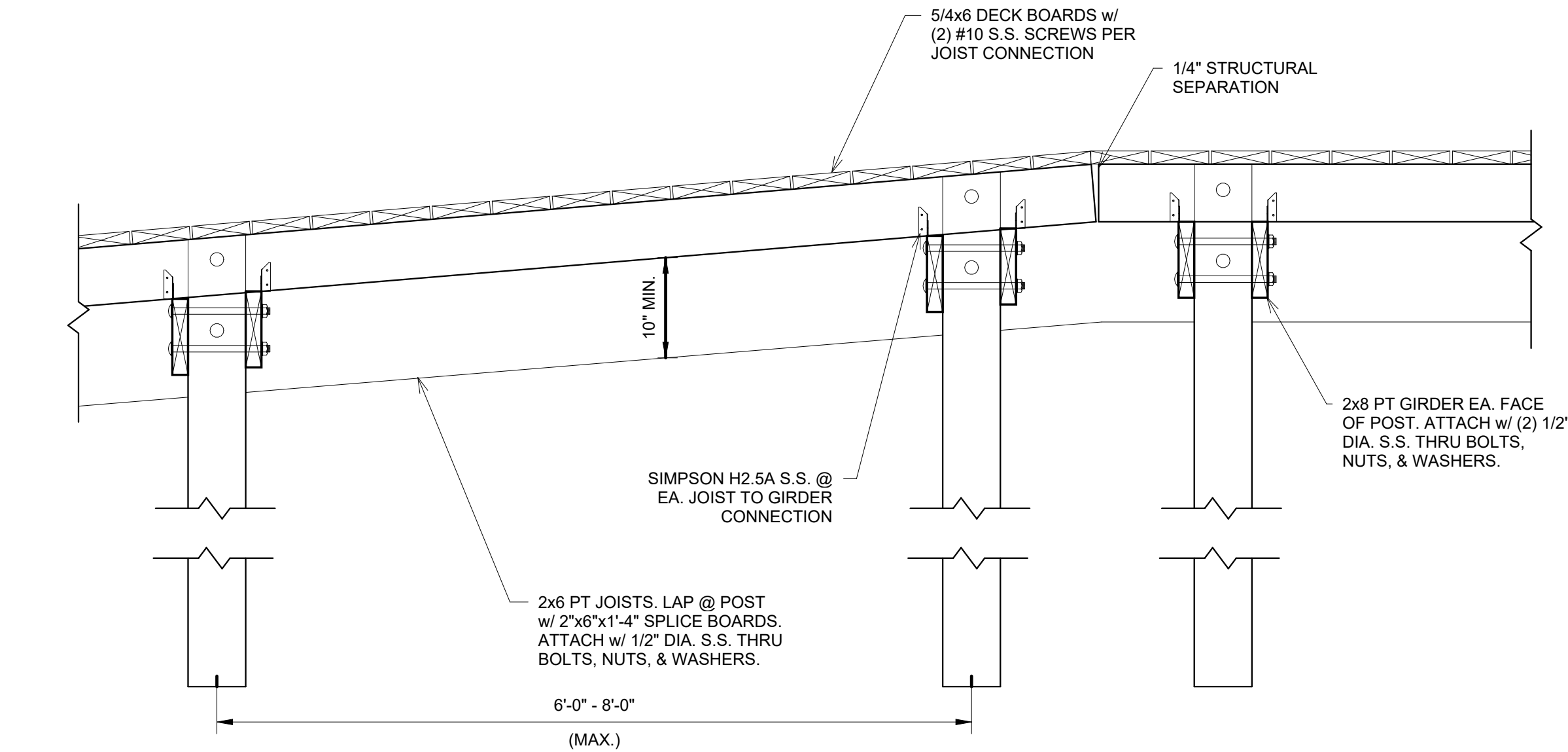
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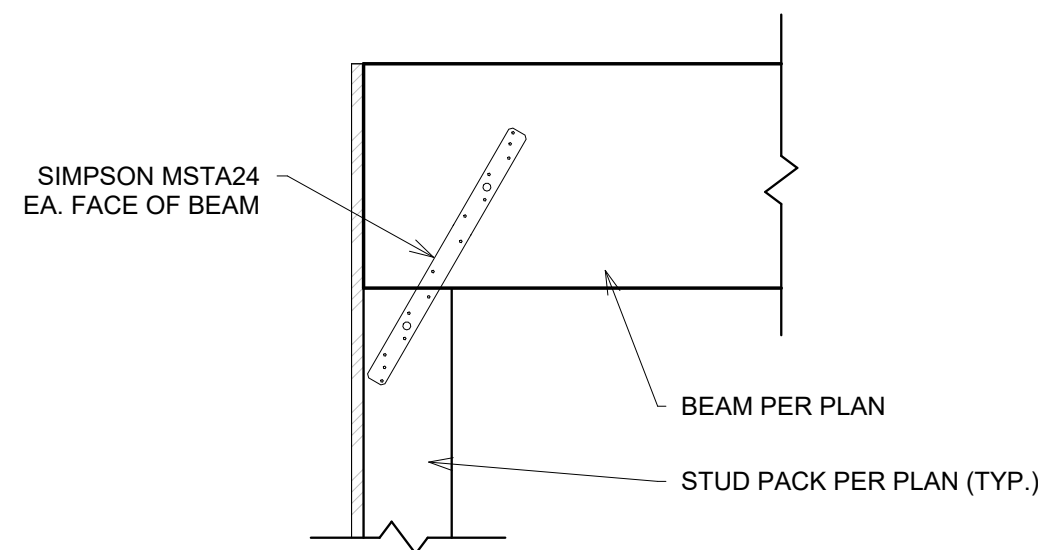
A TYP. WALKWAY CROSS SECTION DETAIL
1" = 1'-0"



B TYP. PILE CAP "PC-1" w/ 8x16 CMU PEIR DETAIL
1" = 1'-0"

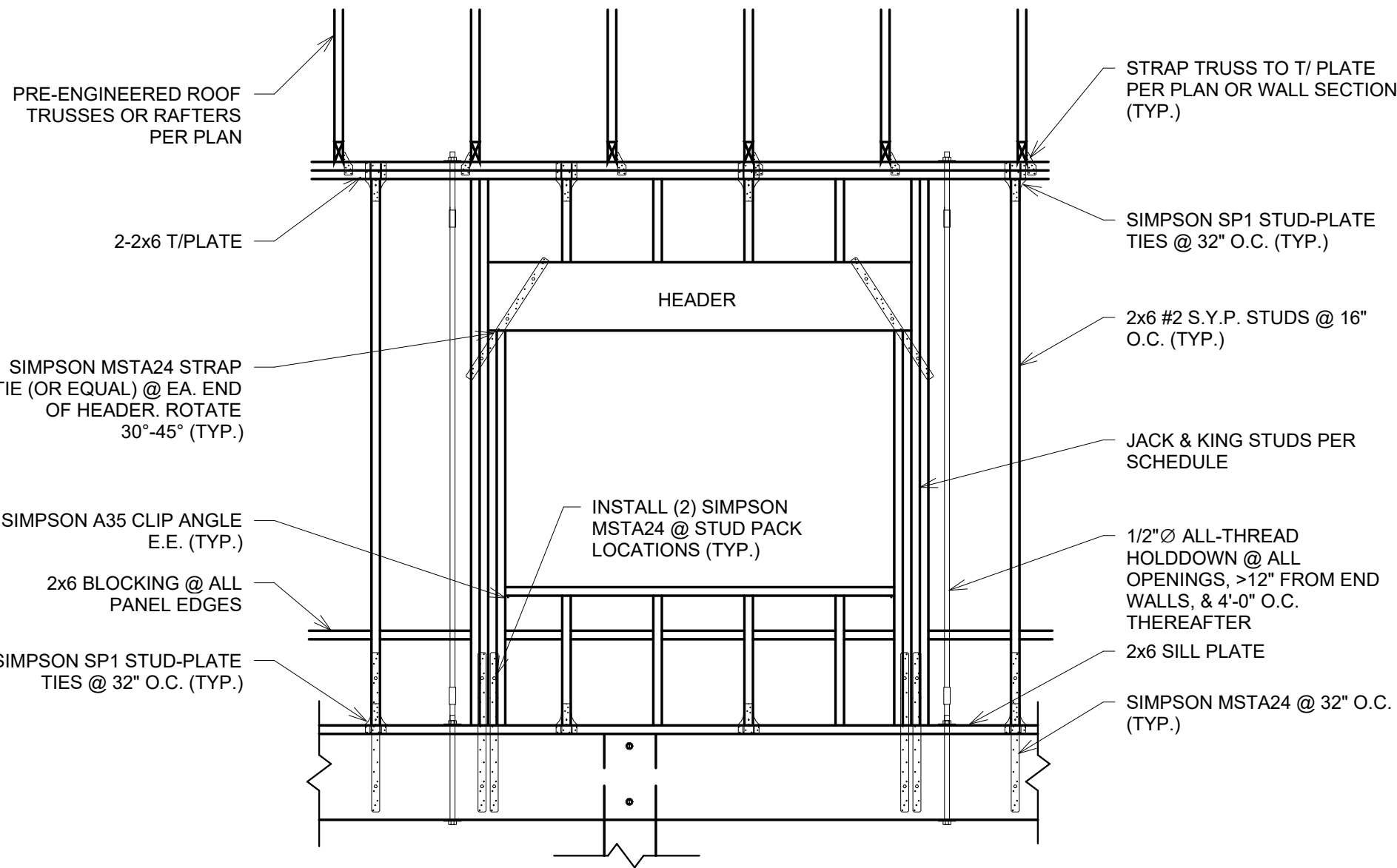


C TYP. WALKWAY DETAIL
1" = 1'-0"

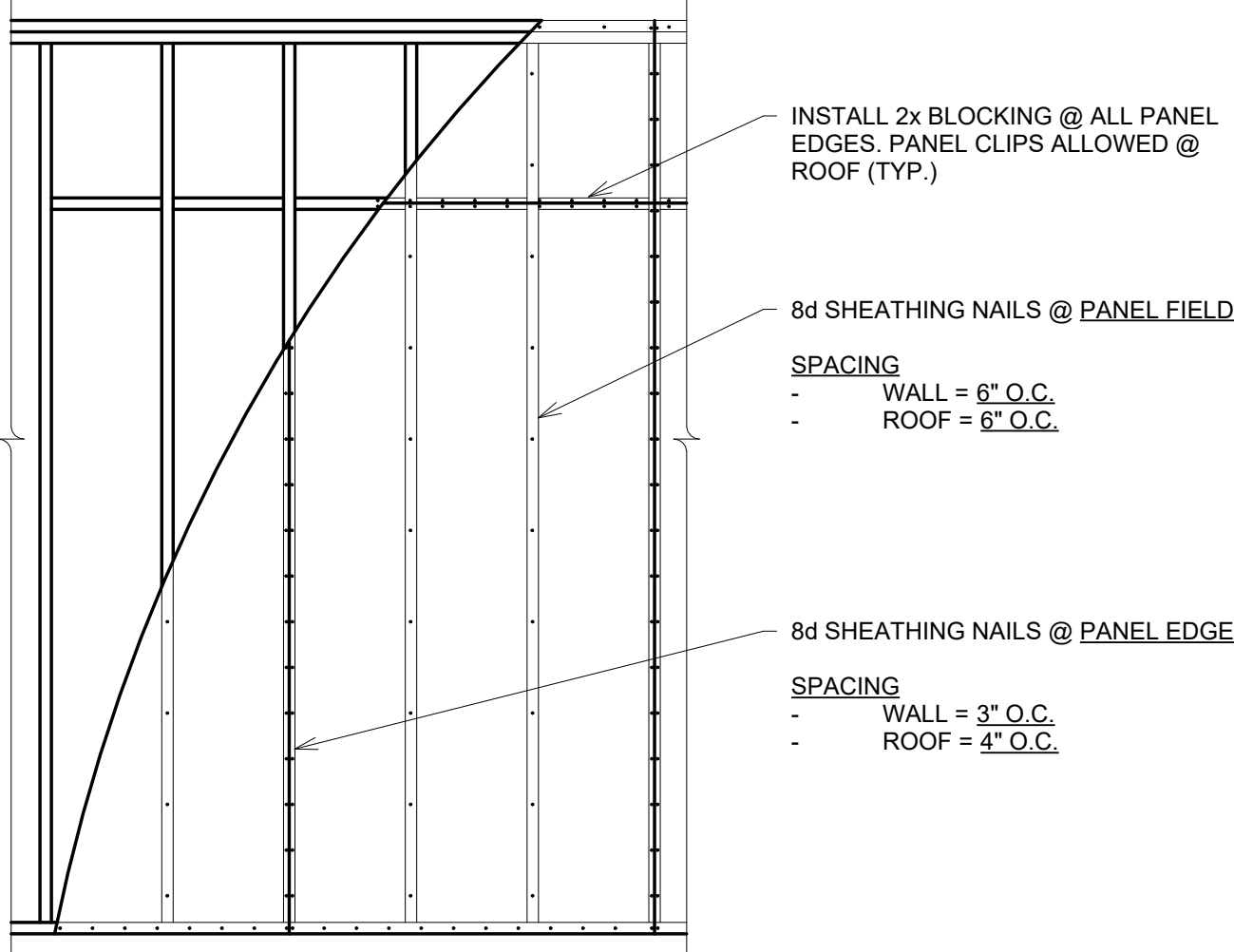


D TYP. BEAM TO STUD PACK DETAIL
1" = 1'-0"

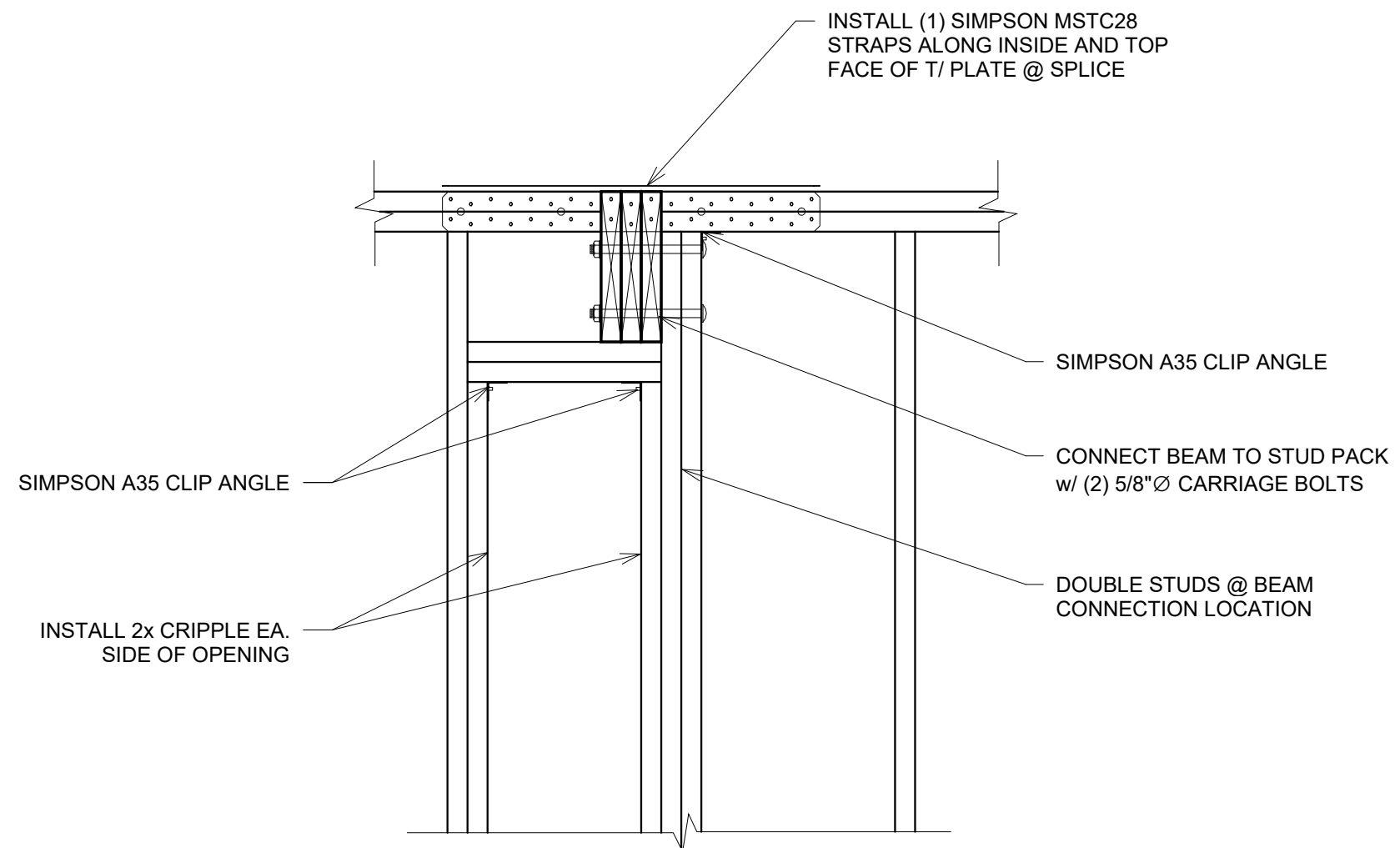
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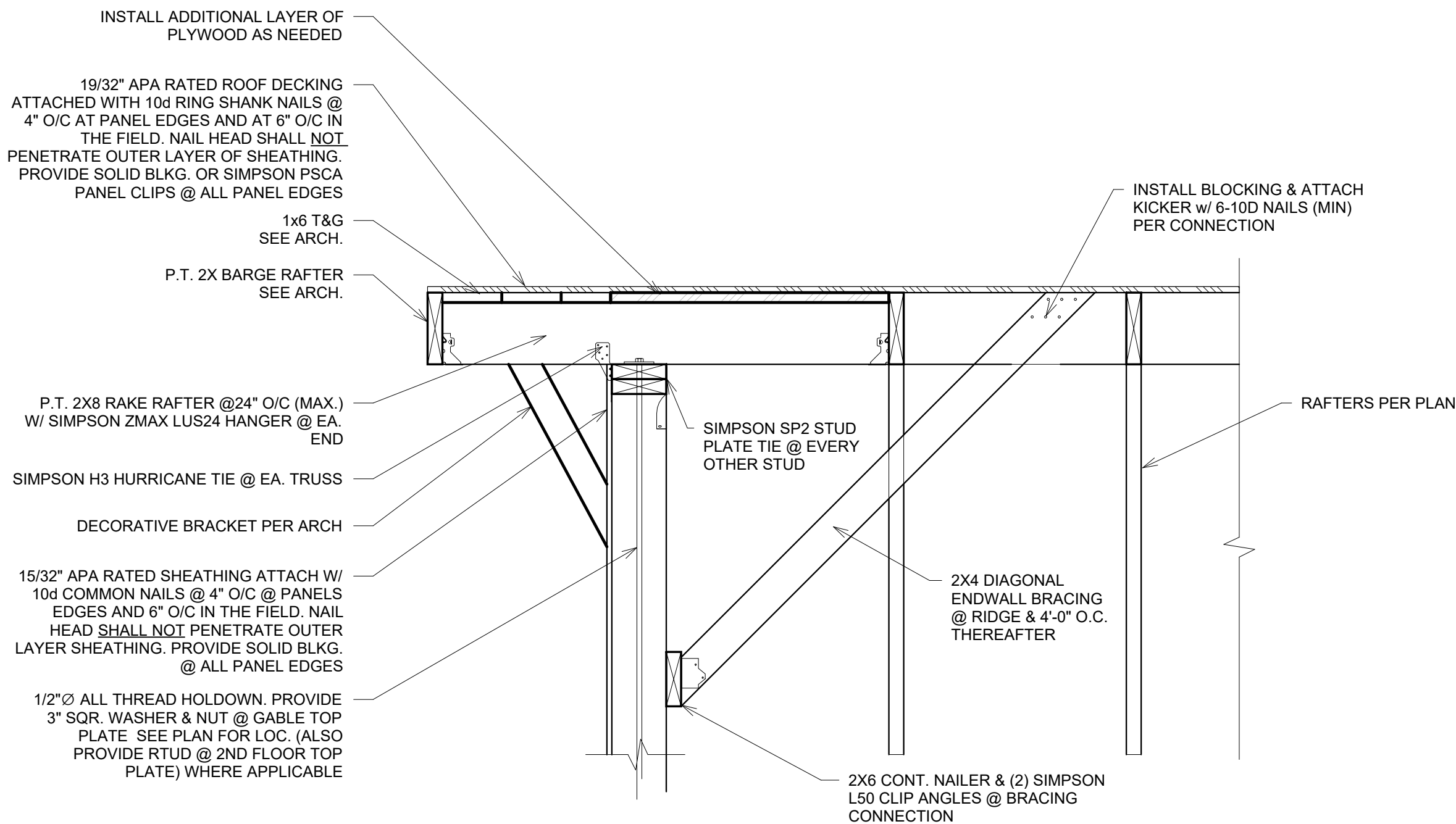
A TYP. 1 STORY FRAMING DETAIL
1" = 1'-0"



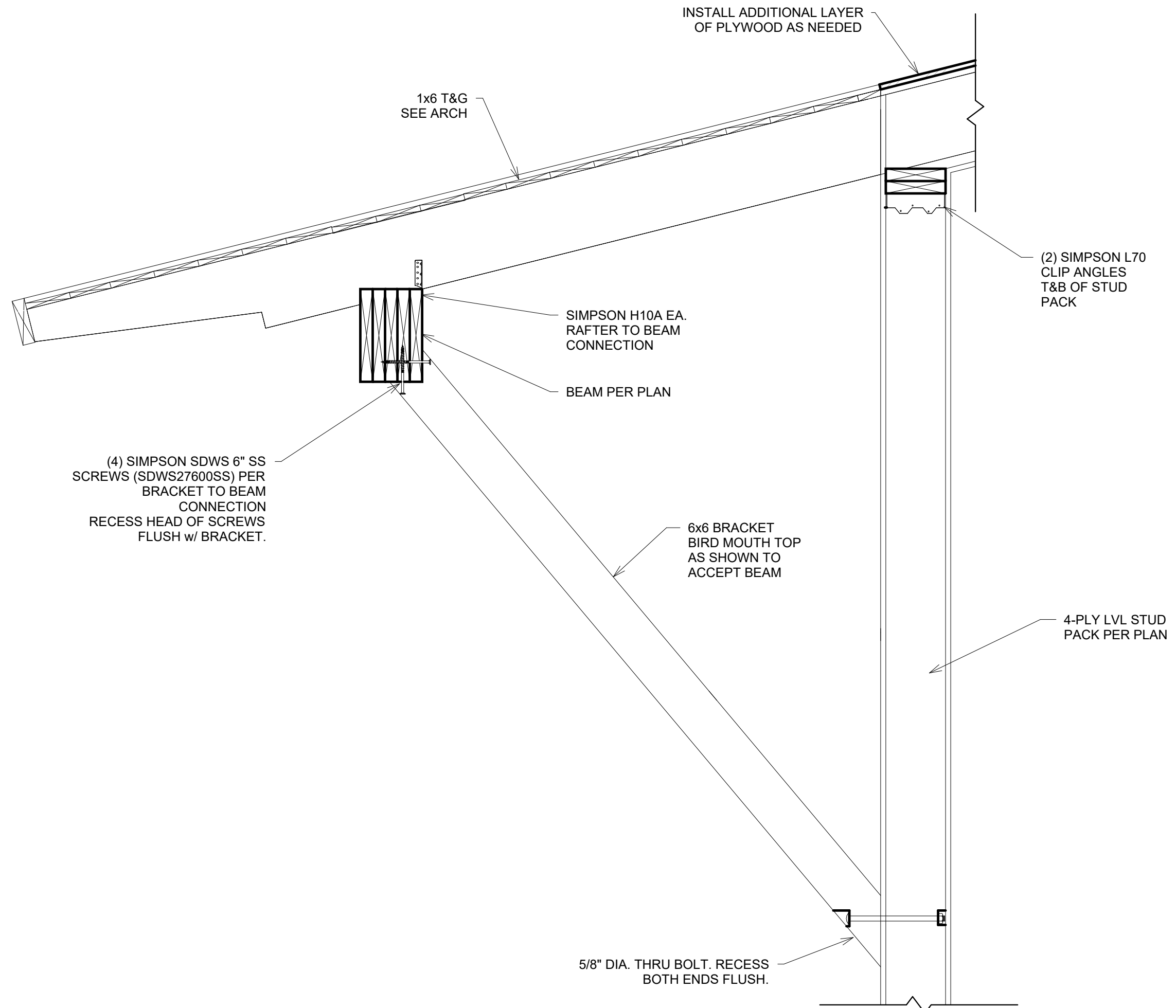
B TYP. SHEATHING NAIL-OFF PATTERN
1" = 1'-0"



C TYP. BEAM POCKET DETAIL
1" = 1'-0"



D TYP. RAKE RAFTER & GABLE ENDWALL BRACING
1" = 1'-0"



E TYP. STRUCTURAL BRACKET DETAIL
1" = 1'-0"