

A horizontal number line with two tick marks. The first tick mark is labeled '1' and the second tick mark is labeled '5'.

© 2020 DAG ARCHITECTS, INC. ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE COPIED, REPRODUCED OR TRANSMITTED IN ANY MANNER WITHOUT WRITTEN PERMISSION OF DAG ARCHITECTS, INC. THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY AND COPYRIGHT OF DAG ARCHITECTS, INC. AND SHALL NOT BE USED ON ANY OTHER PROJECT OR LOCATION EXCEPT AS DESCRIBED ON THE DRAWINGS. WITHOUT THE PRIOR WRITTEN AGREEMENT OF DAG ARCHITECTS.

ABBREVIATIONS	
AB	ANCHOR BOLT
ADDL	ADDITIONAL
ALT	ALTERNATE
ALUM	ALUMINUM
ARCH	ARCHITECTURAL or ARCHITECT
BLDG	BOTTOM OF BUILDING
BLK	BLOCK
BLKG	BLOCKING
BM	BEAM
BOS	BOTTOM OF STEEL
BRG	BEARING
B/S	BOTTOM SIDE
BTM or (B)	BOTTOM, "BOT" SIM
BTW	BETWEEN
CB	CARRIAGE BOLT
CFS	COLD FORMED STEEL
CIP	CAST IN PLACE
C-J	CONTROL JOINT
CL or (L)	CENTER LINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
C.O.	CASED OPENING
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONST	CONSTRUCTION
CONT	CONTINUE or CONTINUOUS
CSJ	CONSTRUCTION JOINT
CTR	CENTERED, "CNTR'd" SIM
DET	DETAIL
DBL	DOUBLE
DIA or (Ø)	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DIR	DIRECTION
D.L.	DEAD LOAD
DN	DOWN
DWG	DRAWING
DWL	DOWEL
EA	EACH
E.E.	EACH END
E.F.	EACH FACE
EJ	EXPANSION JOINT
ELEV	ELEVATION
EMBED	EMBEDDED or EMBEDMENT
EOS	EDGE OF SLAB
E.G.	EXISTING GRADE
E.S.	EACH SIDE
EQ	EQUAL
E.W.	EACH WAY
EX	EXISTING
EXPB	EXPANSION BOLT
EXT	EXTERIOR
F.F.	FAR FACE
FFE	FINISHED FLOOR ELEVATION
F.G.	FINISHED GRADE
FIN	FINISH
FL	FLOOR
FLG	FLANGE
FND	FOUNDATION
FOC	FACE OF CONCRETE
FOM	FACE OF MASONRY
FOS	FACE OF STUD
FP	FULL PENETRATION
F/S	FAR SIDE
FT	FOOT OR FEET
FTG	FOOTING
GALV	GALVANIZED
GB	GRADE BEAM
GDR	GIRDER
GP	GUSSET PLATE
HAS	HEADED ANCHOR STUD
HD	HOLDOWN
HDG	HOT DIP GALVANIZED
HDR	HEADER
HK	HOOK
HORIZ	HORIZONTAL
HSS	HIGH STRENGTH BOLT
H.S.S.	HEADED SHEAR or "NELSON" STUD
HSS	HOLLOW STL SECTION or TUBE
I.F.	INSIDE FACE
IN	INCH OR INCHES
INT	INTERIOR
INV	INVERT
JST	JOIST
JT	JOINT
K	KIP (1,000 POUNDS)
LB	POUNDS
L.E.	LEFT END
LL	LIVE LOAD
LLD	LONG LEG DOWN
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
LVL	LAMINATED VENEER LUMBER
LWC	LIGHT WEIGHT CONCRETE
MAS	MASONRY
MAX	MAXIMUM
MB	MACHINE BOLT
MGR	MANUFACTURER
MID or (M)	MIDDLE
MIN	MINIMUM
MISC	MISCELLANEOUS
M.R.D.	METAL ROOF DECK
MTL	METAL
MTRL	MATERIAL
N.F.	NEAR FACE
NOM	NOMINAL
NO.	NUMBER OR #
NS	NEAR SIDE
NS	NON-SHRINK
N.T.S.	NOT TO SCALE
NWC	NORMAL WEIGHT CONCRETE
O.C.	ON CENTER, "C.C." SIM
O.D.	OVERALL DIM, "O'D" SIM
O.F.	OUTSIDE FACE
OPP	OPPOSITE
OVS	OVERSIZED
PCC	PRECAST CONCRETE
PEN	PENETRATION
PERP	PERPENDICULAR
PL	POINT LOAD
PL	PLATE
PSL	PARALLEL STRAND LUMBER
PT	POINT
P.T.	PRESSURE TREATED or POST-TENSION
P.U.	RIGHT END
REINF	REINFORCING
REF	REFERENCE
RET	RETURN
REQ	REQUIRED
R.L.L.	ROOF LIVE LOAD
R.O.	ROUGH OPENING
SC	SLIP CRITICAL
SCHED	SCHEDULE
S.D.N.	STRUCTURAL DESIGN NOTES
SIM	SIMILAR
SOG	SLAB ON GRADE
S.E.	STUD PACK
SPECS	SPECIFICATIONS
SQ	SQUARE
SS	STAINLESS STEEL, "S.S." SIM
STAG	STAGGERED
STD	STANDARD
STIFF	STIFFENER
STL	STEEL
STR	STRUCTURAL, "STRUCT" SIMILAR
S.W.	SHEAR WALL
SYM	SYMMETRICAL
T or (T)	TOP
T&S	TOP AND BOTTOM TEMPERATURE & SHRINKAGE REINF.
TJ	TOP OF
T/S	TOP SIDE
TDR	THREADED ROD
TEMP.	"T&S" FOR REINF. ELSE TEMPORARY
THK	THICK OR THICKNESS
TOB	TOP OF BEAM
TOC	TOP OF CONCRETE
TOP	TOP OF FOOTING
TOP	TOP OF STEEL
TOW	TOP OF WALL
TRANS	TRANSVERSE
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE, "U.N.O." SIM
UWA	UNDER WALL ABOVE, "U.W.A." SIM
VERT	VERTICAL
W/	WITH
WF	WIDE FLANGE
WP	WORKING POINT
WWF	WELDED WIRE FABRIC
WWM	WELDED WIRE MESH

- FLOOR TRUSS LOADS:

FLOOR TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS UNLESS SPECIAL LOADING PATTERN IS PROVIDED BY THE STRUCTURAL ENGINEER OF RECORD.

TOP CHORD LIVE LOAD

40 PSF

TOP CHORD DEAD LOAD

20 PSF

BOTTOM CHORD DEAD LOAD

10 PSF

TOTAL

70 PSF
- ROOF TRUSS LOADS:

ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS UNLESS A SPECIAL LOADING PATTERN IS PROVIDED BY THE STRUCTURAL ENGINEER OF RECORD.

TOP CHORD LIVE LOAD

20 PSF

TOP CHORD DEAD LOAD

20 PSF

BOTTOM CHORD DEAD LOAD

10 PSF

TOTAL

40 PSF
- CONNECTIONS

CONNECTIONS FOR STRUCTURAL TIMBER SHALL BE GALVANIZED STRONG TIE CONNECTORS BY THE SIMPSON COMPANY OR APPROVED EQUAL. CONNECTORS SHALL FOLLOW MANUF. CORROSION PROTECTION RECOMMENDATIONS.

THE NUMBER OF FASTENERS PER CONNECTION SHALL BE THE MAX. ALLOWED FOR THAT PARTICULAR FASTENER.
- 8) TIE-DOWN NOTES:

ANCHOR TIE-DOWN RODS SHALL HAVE A MINIMUM SHAFT DIAMETER OF 1/2" (ONE HALF INCH) U.N.O.

TIE-DOWN RODS MAY BE CONNECTED DIRECTLY TO THE PLAN SPECIFIED 5/8" ANCHOR BOLTS USING A SIMPSON CNW5/8"-1/2" TRANSITION COUPLER NUT ABOVE THE REQUIRED BP3-5/8" AND 5/8" NUT TOP THE SILL PLATE.

TIE-DOWN DIRECT CONNECTION MAY BE USED BY UTILIZING SIMPSON SET ANCHORAGE ADHESIVE WITH MINIMUM ROD EMBEDMENT OF 6" WHEREVER CAST-IN-PLACE ANCHOR BOLTS ARE SPECIFIED FOR COUPLED EXTENSION TO ABOVE.

ANY PLACE WHERE A TIE-DOWN LOCATION IS FILLED WITH STUDS, USE A SIMPSON LTT131 OR HDU5 HOLDOWN CONNECTED AT THE TOP AND BOTTOM OF A MINIMUM 2PLY STUD PACK TO PRESERVE TENSION LOAD PATH TO FLOORS ABOVE AND/OR BELOW. RESUME INDICATED TIE-DOWN ROD PATH AT ENDS OF STUD PACK IF POSSIBLE.

SIMPSON LTT131 & HDU5 HOLDOWNS SHALL BE ANCHORED INTO GROUT FILLED CMU WALLS OR PIERS WITH A SIMPSON SBS/8 OR SST8 ANCHOR BOLT WITH EMBEDMENT PER MFR. RECOMMENDATION.

RODS SHALL BE INSTALLED PLUMB. THE MAXIMUM ALLOWABLE ROD DRIFT FROM PLUMB SHOULD NOT EXCEED 1.33 DEGREES OR MAXIMUM CENTERLINE OFFSET DISTANCE OF 1.75" PER FLOOR.

TIE-DOWN RODS SHALL HAVE A SIMPSON BP-3 WITH NUT ABOVE THE TERMINAL TOP PLATE. RODS AT SLOPED OR RAKED PLATES SHALL REQUIRE HILLSIDE WASHERS BETWEEN BP-3 AND NUT TO LEVEL.

ALL RODS ARE TO BE TIGHTENED AT EACH FLOOR AND THE TOP PLATE CONNECTION AFTER ROOF IS FULLY LOADED. JUST PRIOR TO GYPSUM BOARD INSTALLATION, ALL RODS WITH BP AND NUT CONNECTIONS TO THE BOTTOM SIDE OF THE FIRST FLOOR FRAMING SHALL BE TIGHTENED AT THE END OF THE CONSTRUCTION PROCESS PRIOR TO C.O. OPTIONAL: CONTRACTOR MAY INSTALL A SIMPSON BRPTUD AND RTUD TAKE-UP DEVICE AT THE TERMINAL TOP PLATE CONNECTION TO REMOVED UNWANTED SYSTEM SLACK DUE TO STRUCTURE SETTLING OR CREEP.
- 9) MASONRY WALLS:

ALL MASONRY CONSTRUCTION SHALL CONFORM TO ACI 530/ASCE 5/TMS 402 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" AND ACI 530.1/ASCE 5/TMS 602 "SPECIFICATION FOR MASONRY STRUCTURES", LATEST EDITION.

MASONRY UNITS SHALL MEET ASTM C-90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 1900 PSI ON THE NET AREA (fm = 1500 PSI). MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C-270.

GROUT SHALL BE 3000 PSI MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C-476 AND HAVE A SLUMP BETWEEN 8" AND 11" WITH WATER CM RATIO OF 0.55 MAXIMUM AND WITH 3/8" MAXIMUM AGGREGATE.

PROVIDE HOOKED DOWELS IN FOUNDATIONS FOR VERTICAL REINFORCING ABOVE. REFER TO TABLE FOR SPLICE LENGTH.

BLOCK CELLS SHALL BE GROUT FILLED WITH VERTICAL REINFORCING BARS AT CORNERS, INTERSECTIONS, EACH SIDE OF OPENINGS AND AS SHOWN ON THE DRAWINGS.

DOWELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW, UNLESS NOTED OTHERWISE.

USE METAL LATH, MORTAR OR SPECIAL UNITS TO CONFINE CONCRETE AND GROUT TO AREA AS REQUIRED.

MASONRY SHALL BE LAID IN RUNNING BOND PATTERN UNLESS NOTED OTHERWISE. AT FILLED CELLS LAY UNITS WITH FULL BED JOINTS AROUND CELLS.

PROVIDE 9 GAUGE GALVANIZED HORIZONTAL JOINT REINFORCING AT ALTERNATE BLOCK COURSES. LADDER TYPE IS RECOMMENDED WITH 2" SPACING. PROVIDE PREFABRICATED "TEE" OR CORNER SECTIONS AT WALL INTERSECTIONS.

CONTROL JOINTS SHALL BE CONSTRUCTED IN CONCRETE MASONRY CONSTRUCTION AT A MAXIMUM HORIZONTAL SPACING BETWEEN JOINTS OF 25'-0" AND NOT MORE THAN 126" FROM CORNERS. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS. CONSTRUCT INTERIOR CONTROL JOINTS AT A MAXIMUM HORIZONTAL SPACING OF 32'-0" OR 16'-0" FROM CORNERS. NO JOINTS SHALL BE LOCATED WITHIN 2'-0" OF STEEL BEAM BEARINGS. HORIZONTAL WALL REINFORCING SHALL BE STOPPED EACH SIDE OF CONTROL JOINTS. SEE ARCHITECTURAL DRAWINGS FOR SEALANT REQUIREMENTS AT CONTROL JOINTS.

SUBMIT PROPOSED GROUT MIX DESIGNS FOR REVIEW PRIOR TO USE. MIX NUMBER OR OTHER POSITIVE IDENTIFICATION SHALL UNIQUELY IDENTIFY MIX.

USE OF SUPERPLASTICIZER IS PROHIBITED.

CELLS TO BE GROUT FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL GROUT SPACE.

CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF CELLS TO BE GROUT FILLED IN EACH POUR IN EXCESS OF 5 FEET IN HEIGHT. AFTER INSPECTION AND BEFORE GROUTING, THE REBAR SHALL BE TIED AT THE CLEANOUTS AND THE CLEANOUTS SHALL BE SEALED.

ANY OVERHANGING MORTAR OR OTHER OBSTRUCTION OR DEBRIS SHALL BE REMOVED FROM THE INSIDES OF SUCH CELL WALLS.

VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 192 BAR DIAMETERS.

ALL CELLS SHALL BE FILLED SOLIDLY WITH GROUT (U.N.O.). SAMPLE AND TEST GROUT PER ASTM C1019.

GROUT SHALL BE POURED IN LIFTS OF 4 FEET MAXIMUM HEIGHT. GROUT SHALL BE CONSOLIDATED AT TIME OF PLACING BY VIBRATING AND RECONSOLIDATED LATER BY VIBRATING BEFORE PLASTICITY IS LOST.

WHEN TOTAL GROUT POUR EXCEEDS 5 FEET IN HEIGHT, (HIGH LIFT GROUTING), THE GROUT SHALL BE PLACED IN 4 FOOT LIFTS WITH MINIMUM OF A 30 MINUTE DELAY BETWEEN LIFTS. MINIMUM CELL DIMENSION SHALL BE IN ACCORDANCE WITH TABLE 5 OF ACI 530.1 (3" X 3" FOR COARSE GROUT, 12 FT. MAXIMUM POUR HEIGHT).

WHEN THE GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZONTAL CONSTRUCTION JOINTS SHALL BE MADE BY STOPPING THE POUR OF GROUT NOT LESS THAN 1-1/2 INCH BELOW THE TOP OF THE UPPERMOST UNIT GROUTED.

MASONRY WALLS MARKED AS "LOAD BEARING" ARE DESIGNED TO CARRY FLOOR GRAVITY LOADS AND MUST BE CONSTRUCTED TO SUPPORT THE CONCRETE FLOOR SLAB CONCURRENTLY WITH CONCRETE COLUMN CONSTRUCTION.

MASONRY WALLS INDICATED AS "INFILL" ARE DESIGNED TO RESIST LATERAL LOADS AND MUST BE CONSTRUCTED AFTER THE CONCRETE SLAB IS CAST AND POST TENSIONING OPERATION IS COMPLETED. INFILL WALLS SHALL BE CONSTRUCTED STARTING AT THE FOUNDATION LEVEL AND WORKING UPWARD ONE LEVEL AT A TIME. DO NOT START NEXT HIGHER LEVEL OF WALL PRIOR TO COMPLETION OF WALL BELOW. ALLOW A MINIMUM OF 3 DAYS CURING FOR GROUT OF WALL BELOW PRIOR TO STARTING WALL ABOVE.

SINGLE STORY MASONRY WALLS INDICATED AS "PARTITION WALLS" SHALL BE CAST ON THICKENED SLAB FOUNDATIONS AND ARE NOT DESIGNED TO CARRY ANY LOADS FROM THE MAIN BUILDING STRUCTURES. ISOLATE TOP OF PARTITION WALLS FROM UNDERSIDE OF CONCRETE SLAB WITH A MINIMUM 1/2" THICK COMPRESSIBLE MATERIAL.

SUBMIT WRITTEN CONSTRUCTION SEQUENCES AND PROCEDURES PRIOR TO THE START OF MASONRY CONSTRUCTION.
- 10) CHEMICAL (ADHESIVE) ANCHORS:

SHALL BE A TWO PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS HILTI HIT HY150, HILTI RE500, OR SIMPSON SET ADHESIVE SYSTEM, OR ENGINEER APPROVED SUBSTITUTION.

STRUCTURAL GENERAL NOTES

- EPOXY TYPES AND BRANDS VARY IN THEIR BOND STRENGTH AND SUITABILITY OF USE. DEPENDING ON TYPE OF LOADING, ANCHOR SPACING, ETC. WHEN A PARTICULAR TYPE OF EPOXY IS SPECIFIED IN THESE DRAWINGS, A UNIQUE CALCULATION HAS BEEN MADE BASED ON THE PROPERTIES OF THAT SPECIFIC TYPE OF EPOXY FOR THE SPECIFIC CONDITION SHOWN IN THE DETAIL. SUBSTITUTION OF EPOXY TYPE IS NOT ALLOWED WHERE DETAIL SPECIFIES ONLY ONE TYPE OF EPOXY. WITHOUT PRIOR WRITTEN APPROVAL BY THE ENGINEER OF RECORD, NOT ALL EPOXY BRANDS OR TYPES WILL BE ALLOWED AS SUBSTITUTES.

SUBSTITUTION OF EPOXIES IN ONE CONDITION SHALL NOT BE CONSTRUED AS APPROVAL TO MAKE SIMILAR SUBSTITUTION OF EPOXIES IN OTHER DIFFERING CONDITIONS. EACH SUBSTITUTION MUST RECEIVE PRIOR WRITTEN APPROVAL BY THE ENGINEER OF RECORD.

INSTALL IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS.

THE MANUFACTURER'S REPRESENTATIVE SHALL TRAIN INSTALLERS.

THE CONTRACTOR IS RESPONSIBLE TO ENSURE THAT ALL HOLE CLEAN-OUT REQUIREMENTS ARE FULLY COMPLETED BY THE INSTALLERS PRIOR TO INJECTING EPOXY INTO THE HOLES.

NO LOAD SHALL BE APPLIED TO THE EPOXY ANCHORS UNTIL THE EPOXY HAS FULLY CURED AND HAS ACHIEVED IT'S SPECIFIED STRENGTH.

IF DETAIL SHOWS EPOXY ANCHORS IN SLOTTED HOLES, IT IS IMPERATIVE THAT ANY EXCESS EPOXY IS CLEANED UP FROM AROUND THE ANCHOR ROD, SO THAT IT DOES NOT INTERFERE WITH ADJUSTABILITY OF ANCHOR ROD IN SLOTTED HOLE.
- 11) MECHANICAL ANCHORS:

SHALL BE EITHER HEAVY DUTY CONCRETE SCREW ANCHOR (SUCH AS POWERS WEDGE- BOLT, SIMPSON TITEN HD, OR HILTI HUS-H) OR WEDGE TYPE EXPANSION ANCHOR (SUCH AS POWERS POWER-STUD, SIMPSON WEDGE-ALL, OR HILTI KWIK BOLT 3).

TYPE OF ANCHOR SHALL BE AS SPECIFIED ON THE DRAWINGS, WHILE BRAND AND MODEL OF ANCHOR MAY BE SELECTED FROM THE ABOVE LISTED ANCHORS. SUBSTITUTION ANCHORS MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVED IN WRITING BY THE ENGINEER OF RECORD PRIOR TO USE.

IN SOME CASES OF CRITICAL LOADING OR GEOMETRIC CONDITIONS, ONLY SPECIFIC ANCHORS WILL BE ALLOWED, AS NOTED ON THE DRAWINGS. IN THESE CASES, THE SPECIFIED BRAND AND MODEL OF ANCHOR MUST BE USED.

INSTALL IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS.

THE MANUFACTURER'S REPRESENTATIVE SHALL TRAIN INSTALLERS.

MINIMUM EMBEDMENT DEPTH OF 1/4" TAPCONS INSTALLED IN CONCRETE SHALL BE 1.25" AND INSTALLED INTO MASONRY SHALL BE 1.5". SELECT ANCHOR LENGTH AS REQUIRED TO ACHIEVE THE SPECIFIED MINIMUM EMBEDMENT DEPTH.

TAPCON SCREWS MAY BE REPLACED W/ Ø 15MM SHANK DIAMETER PAF ANCHORS (HILTI X-U OR EQUAL) ON A 1:1 SUBSTITUTION BASIS. MINIMUM EMBEDMENT DEPTH SHALL BE 1.25" WHEN INSTALLED INTO CONCRETE OR GROUTED MASONRY. FOLLOW MANUFACTURER'S INSTALLATION RECOMMENDATIONS, MINIMUM EDGE DISTANCES, AND PLACEMENT LIMITATIONS (RELATIVE TO MORTAR JOINTS IN MASONRY).
- 12) STRUCTURAL STEEL:

STEEL WORK SHALL BE NEW AND CONFORM TO THE ANSII/ASCE 360-05 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS.

MATERIAL SHALL CONFORM TO THE FOLLOWING, EXCEPT AS NOTED:

WIDE FLANGE SHAPES

ASTM A992 (Fy=50 KSI)

ANGLES, CHANNELS AND PLATES

ASTM A36 (Fy=36 KSI)

RECTANGULAR HSS ASTM

A500, GRADE B (Fy=46 KSI)

HIGH STRENGTH BOLTS

ASTM A325 OR A490

THREADED RODS

ASTM A36 (Fy=36 KSI)

HEAVY HEX NUTS

ASTM A563

HARDENED STEEL WASHERS

ASTM F436

ANCHOR RODS

ASTM F1554 GR. 36 (Fy=36 KSI)

CONNECTIONS:

BOLTS SHALL BE HIGH-STRENGTH, BEARING TYPE IN SNUG TIGHT CONDITION, U.N.O. TIGHTEN BY AN AISC APPROVED METHOD.

WELDING ELECTRODES SHALL BE PER AWS D1.1. RETURN FILLET WELDS FOR FRAMED CONNECTIONS 1/2" AT EACH END.

FIELD CONNECTIONS SHALL BE BOLTED, EXCEPT AS NOTED OTHERWISE.

DETAIL FLOOR AND ROOF FRAMING CONNECTIONS FOLLOWING THE REQUIREMENTS SHOWN IN THE TYPICAL CONNECTION SCHEDULES SHOWN IN THESE DRAWINGS, BASED ON THE BEAM OR GIRDER SIZE.

FOR THE PURPOSE OF CORRECTLY INTERPRETING THE CONNECTION SCHEDULES, GIRDERS SHALL BE CONSIDERED AS ANY FLOOR OR ROOF BEAM WHICH CARRIES OTHER FLOOR OR ROOF BEAMS, OR ANY FLOOR OR ROOF BEAM WHICH CARRIES STEEL COLUMNS.

DETAIL DIAGONAL BRACING CONNECTIONS AS SHOWN IN THE DETAILS. IF NO DETAIL IS PROVIDED, DETAIL CONNECTION TO DEVELOP THE FULL TENSION CAPACITY OF THE DIAGONAL BRACING MEMBER.

BOLTS SHARING LOAD WITH WELDS IN A CONNECTION SHALL BE FULLY PRETENSIONED AND SLIP CRITICAL.

WHERE FULLY PRETENSIONED OR SLIP CRITICAL BOLTS ARE REQUIRED, TIGHTENING SHALL BE ACHIEVED USING EITHER TWIST-OFF TENSION CONTROL BOLTS OR DIRECT TENSION INDICATING WASHERS.

ALL STRUCTURAL STEEL EXPOSED TO EXTERIOR CONDITIONS SHALL BE HOT DIPPED GALVANIZED PER ASTM A123 AND ALL FASTENERS AND HARDWARE SHALL BE HOT DIPPED GALVANIZED PER ASTM A153.

GROUT UNDER BEARING PLATES SHALL BE NON-METALLIC, NON-SHRINK TYPE WITH A COMPRESSIVE STRENGTH OF AT LEAST 5,000 PSI IN 28 DAYS.

THE CAMBER OF STEEL MEMBERS SHALL BE VERIFIED IN THE SHOP AND THE FIELD. WHEN NO CAMBER IS INDICATED, TURN THE MEMBER NATURAL CAMBER UP.

APPLY FIREPROOFING TO STEEL STRUCTURE CALCULATING THE THICKNESS OF FIREPROOFING BY COMPARING THE ACTUAL MEMBER SIZE TO THE MEMBER SIZE USED IN THE DESIGNATED UL RATING AND ADJUSTING APPROPRIATELY.

13) RAILINGS AND HANDRAILS:

ENGINEERED RAILING SYSTEM AND CONNECTION OF SAME TO THIS STRUCTURE SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA.

THE CONFIGURATION OF THE RAILING SYSTEM SHALL BE AS SHOWN ON THE ARCHITECTURAL DRAWINGS.

RAILING SYSTEM AND CONNECTIONS SHALL BE DESIGNED FOR APPLICABLE LOADS AS INDICATED ON THE DRAWINGS AND IN THE BUILDING CODE.

THE LOADS SHALL BE CLEARLY INDICATED ON SHOP DRAWINGS AND SHALL COMPLY WITH ALL APPLICABLE CODES.

SHOP DRAWINGS SHALL SHOW AND SPECIFY CONNECTIONS UTILIZED WITHIN THE RAILING SYSTEM AS WELL AS CONNECTIONS TO AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM SHOWN ON THESE DRAWINGS.

SHOP DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA.

14) STRUCTURAL SUBMITTALS

SHOP DRAWINGS OR REPORTS FOR THE FOLLOWING ITEMS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD PRIOR TO FABRICATION OR CONSTRUCTION (AS APPLICABLE) U.N.O.

CONCRETE ADMIXTURES

CONCRETE STRENGTH TESTS

CONCRETE DETAILING

ENGINEERED FILL COMPACTION TESTING

ENGINEERED ROOF AND FLOOR TRUSSES

STEEL JOISTS

STRUCTURAL STEEL

HANDRAILS AND STAIRS

HOLLOWCORE PLANKS

DAG
Architects

AR0017640
1223 Airport Road
Suite 104
Destin, Florida 32541
850.837.8152
www.dagarchitects.com

Digital Signature

FOR PERMIT

GROVES & KING, LLC

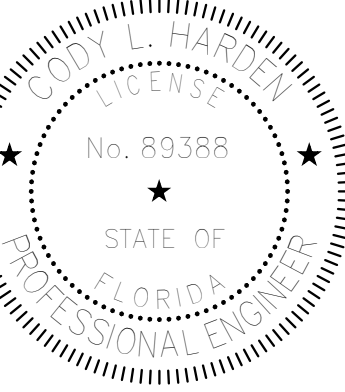
GARAGES LUXURY AT MIRAMAR - PHASE 1

U.S. Highway 98
Miramar Beach, FL

REVISIONS		
NO.	DESCRIPTION	DATE

Project Number	25038
Dated	10.05.2026

STRUCTURAL
GENERAL NOTES
S002



FOR PERMIT

GARAGES LUXURY AT MIRAMAR - PHASE 1

REVISIONS	
NO.	DESCRIPTION

Project Number	25038
----------------	-------

Dated 10.05.2026

FOUNDATION
PLAN - SALES
CENTER

S101



CONCRETE COLUMN SCHEDULE		
DESIGNATION	FLEXURAL REINF.	SHEAR REINF.
CC-1	(10) #5 BARS	#4 CLOSED TIES @ 12" O.C.
CC-2	(10) #6 BARS	#4 CLOSED TIES @ 12" O.C.
CC-3	(6) #6 BARS	#4 CLOSED TIES @ 12" O.C.

FOOTING SCHEDULE				
DESIGNATION	TYPE	DIMENSIONS	LONGITUDINAL REINF.	TRANS. REINF.
FTG-24	STRIP	12" D x 24" W	(3) #5 BARS	#5 BARS x 18" @ 18" O.C.
FTG-36	STRIP	12" D x 36" W	(4) #5 BARS	#5 BARS x 30" @ 18" O.C.
FTG-48	STRIP	12" D x 48" W	(5) #5 BARS	#5 BARS x 42" @ 12" O.C.
FTG-60	STRIP	12" D x 60" W	(8) #6 BARS T&B	#5 BARS x 48" @ 10" O.C.
PED-36	PEDESTAL FTG.	14" D x 36" SQR.	(3) #5 BARS EQ. SPACED	(3) #5 BARS EQ. SPACED
PED-48	PEDESTAL FTG.	14" D x 48" SQR.	(5) #5 BARS EQ. SPACED	(5) #5 BARS EQ. SPACED
PED-72	PEDESTAL FTG.	14" D x 72" SQR.	(8) #5 BARS EQ. SPACED	(8) #5 BARS EQ. SPACED
TD-8	TURNDOWN	16" D x 8" W	(1) #5 BAR	N/A
TD-12	TURNDOWN	16" D x 12" W	(2) #5 BARS	#5 BARS x 6" @ 18" O.C.
TD-36	TURNDOWN	16" D x 36" W	(4) #5 BARS	#5 BARS x 30" @ 18" O.C.
TD-48	TURNDOWN	16" D x 48" W	(6) #5 BARS	#5 BARS x 42" @ 18" O.C.
TS-48	STRIP	16" D x 48" W	(6) #5 BARS	#5 BARS x 42" @ 18" O.C.

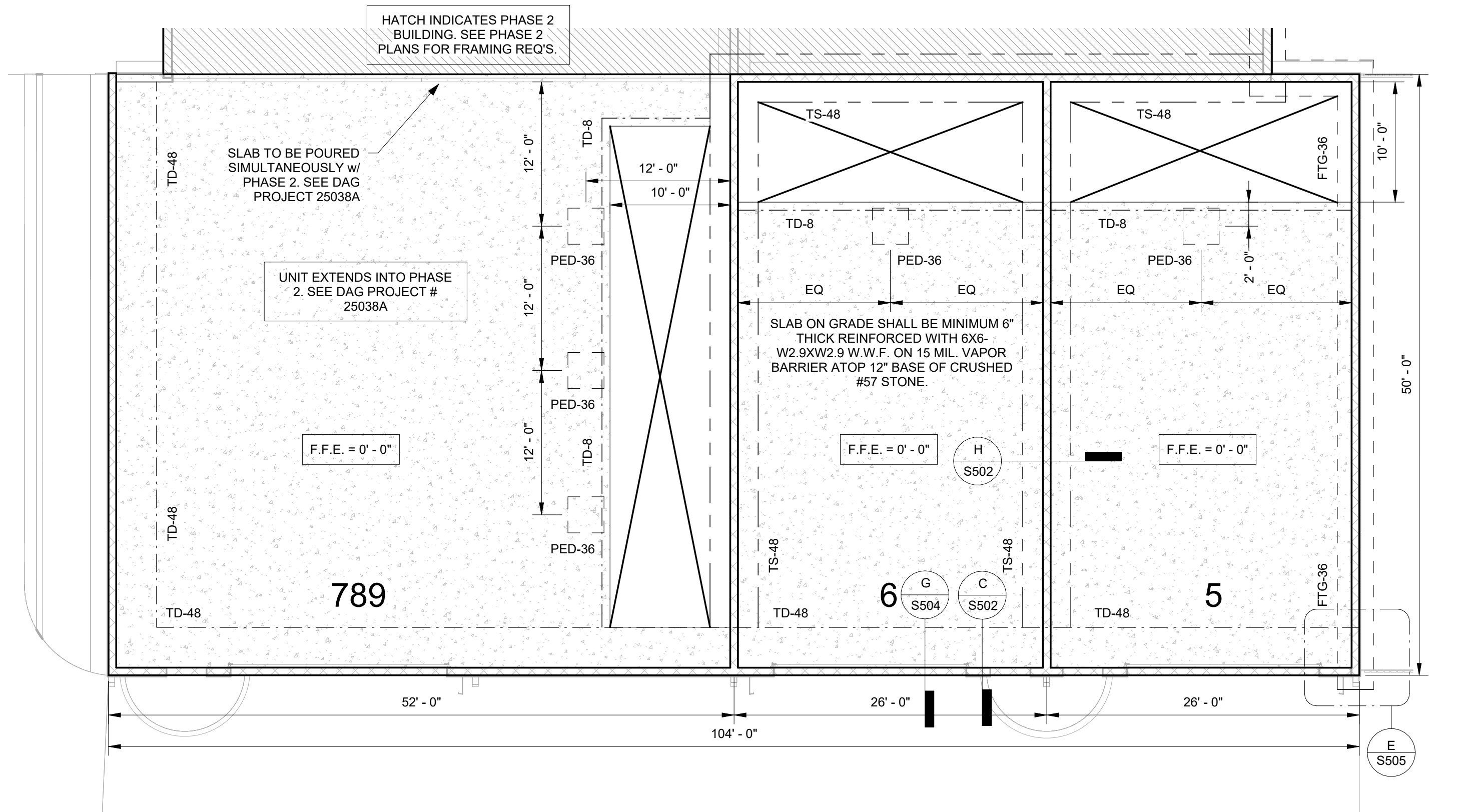
- ## SLAB-ON-GRADE NOTES
1. TOP OF FIRST FLOOR SLAB IS AT DATUM EL. 0'-0" (0'-0" NAVD). (U.N.O.).
 2. SLABS ARE DESIGNED USING 6" THICK 3,000 PSI CONC. REINFORCED W/ 6X6-W2.9XW2.9 WELDED WIRE MESH ON 15 MIL. VAPOR BARRIER ATOP A 12" THICK BASE OF CRUSHED #57 STONE.
 3. SLABS UTILIZING 6X6-W2.9XW2.9 WELDED WIRE MESH SHOULD HAVE CONTROL JOINTS @ MAXIMUM 12 FT O.C.E.W. TO REDUCE EXCESSIVE TEMPERATURE & SHRINKAGE CRACKING.
 4. ALL TURNDOWN SLAB FOOTINGS MUST HAVE A MINIMUM PERIMETER HAUNCH DEPTH OF 20" (U.N.O.).
 5. SEE ARCH. DRAWINGS FOR DIMENSIONS NOT SHOWN. IF A CONFLICT EXISTS, THE ARCH. DIMENSIONS SHALL GOVERN. BOTH ENGINEER AND ARCHITECT OF RECORD SHALL BE NOTIFIED IN WRITING OF ALL CONFLICTS.
 6. FOR TYPICAL DETAILS SEE S500 SERIES SHEETS.

FOOTING NOTES

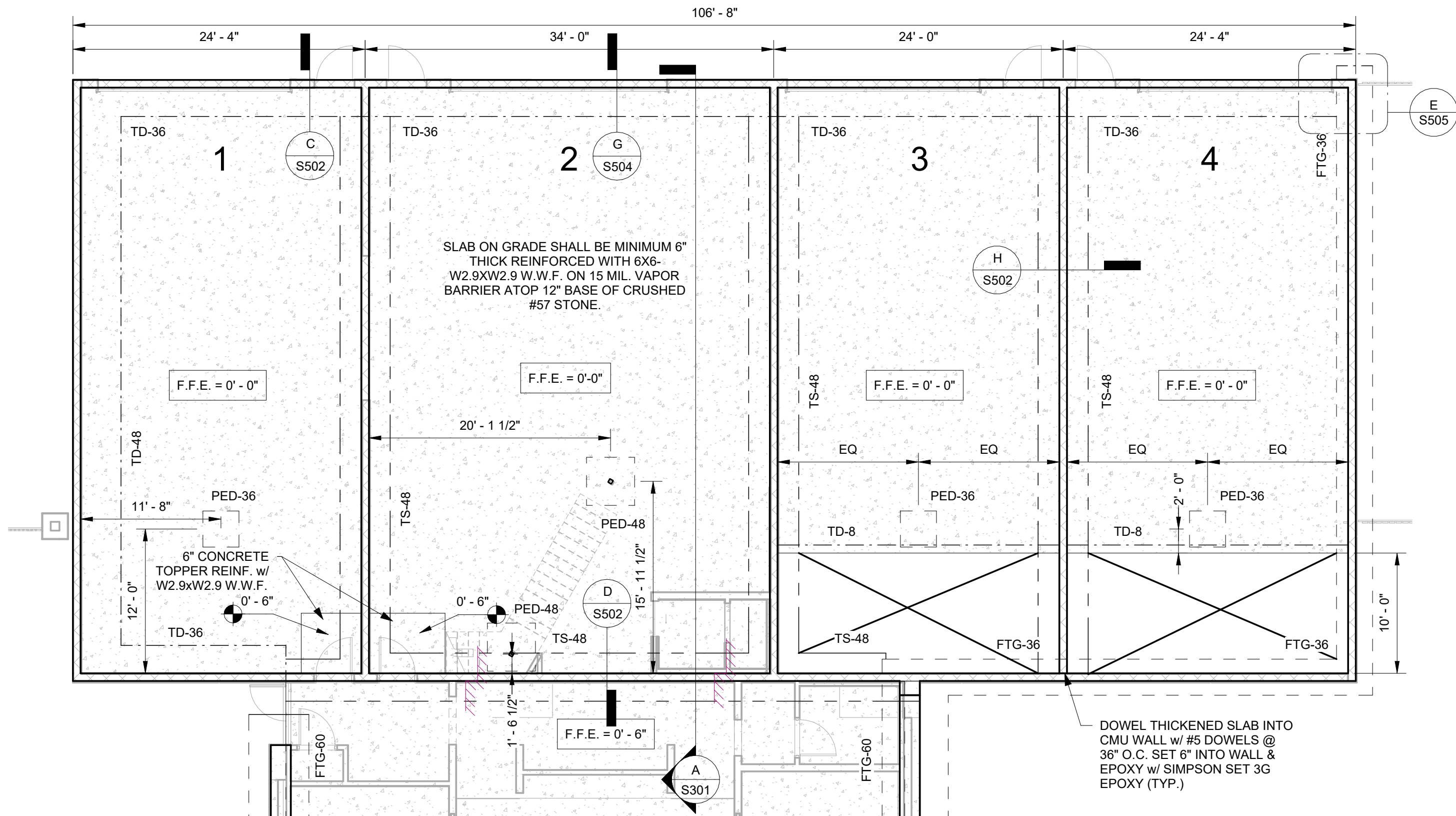
1. TOPS OF ALL FOOTINGS (EXCEPT TURNDOUNOS) MUST BE COVERED WITH A MINIMUM OF 12" OF FINISH GRADE MATERIAL, (U.N.O.).
2. BOTTOMS OF ALL FOOTINGS MUST BEAR AT/ON UNDISTURBED SOIL OR ENGINEERED FILL COMPACTED TO 95% MAXIMUM MODIFIED PROCTOR A MINIMUM OF 12" BELOW EXISTING GRADE UNLESS PROPER PROVISIONS HAVE BEEN MADE FOR THE USE OF ENGINEERED FILL BASED ON A SITE SPECIFIC GEOTECHNICAL EVALUATION (BY OTHERS).
3. MINIMUM FOOTING WIDTHS MUST ALLOW FOR A MINIMUM 8" OFFSET FROM BOTH FACES OF CMU WALLS (TYP).
4. ALL CONTINUOUS FOOTINGS & PEDESTALS MUST BE CENTERED ON THE WALL/PIER UNLESS SPECIFICALLY NOTED AS ECCENTRIC FOOTING.
5. FOOTING SIZES, REINFORCING TYPE & SPACING LISTED WITHIN THE FOOTING SCHEDULE SHALL GOVERN OVER FOOTING NOTES.
6. VERTICAL DOWEL HOOKS SHOULD EXTEND 9" IN TO FTG'S, BELOW & RUNNING PERPENDICULAR TO FLEXURAL REINF.
7. NO "WET STICK" SETTING OF REINFORCING WILL BE APPROVED.
8. ALL VERTICAL REINFORCING MUST BE IN PLACE & TIED TO FOOTING REINF. PRIOR TO POUR.
9. COORDINATE ALL FOOTING LOCATIONS WITH CIVIL, PLUMBING, AND ELECTRICAL DRAWINGS. CONTRACTOR SHALL PROVIDE PIPE SLEEVES BELOW OR ABOVE CONTINUOUS WALL FOOTINGS.
10. SEE ARCH. DRAWINGS FOR DIMENSIONS NOT SHOWN. IF A CONFLICT EXISTS, THE ARCH. DIMENSIONS SHALL GOVERN. BOTH ENGINEER AND ARCHITECT OF RECORD SHALL BE NOTIFIED IN WRITING OF ALL CONFLICTS.
11. FOR TYPICAL DETAILS SEE S500 SERIES SHEETS.

A T/SLAB SALES CENTER
1/4" = 1'-0"

© 2023 DAG ARCHITECTS, INC. ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE COPIED, REPRODUCED OR TRANSMITTED IN ANY MANNER WITHOUT WRITTEN PERMISSION OF DAG ARCHITECTS, INC. THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY AND COPYRIGHT OF DAG ARCHITECTS, INC. AND SHALL NOT BE USED ON ANY OTHER PROJECT OR LOCATION EXCEPT AS DESCRIBED ON THE DRAWINGS WITHOUT THE PRIOR WRITTEN AGREEMENT OF DAG ARCHITECTS.

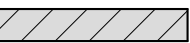


03 - T/ SLAB BUILDING B UNITS
1/8" = 1'-0"



03 - T/ SLAB BUILDING A UNITS
1/8" = 1'-0"

FOUNDATION LEGEND

-  - DENOTES STEMWALL BELOW. REINF. PER PLAN.
-  - DENOTES TYP. 8" CMU WALL REINF. w/ VERT. #5 BARS @ ENDS, CORNERS, EACH SIDE OF INTERSECTIONS, & 24" O.C. THEREAFTER. INSTALL DBL. COURSE OF OPEN BOTTOM BOND BEAMS w/ (2) CONT. #5 BARS EA. @ TOP OF CMU WALLS & MID FLOORS (TYP)
-  - DENOTES EDGE OF FOUNDATION ELEMENT BELOW

SLAB-ON-GRADE NOTES

- TOP OF FIRST FLOOR SLAB IS AT DATUM EL. 0'-0" (0'-0" NAVD), (U.N.O.).
- SLABS ARE DESIGNED USING 6" THICK 3,000 PSI CONC. REINFORCED W/ 6X6-W2.9XW2.9 WELDED WIRE MESH ON 15 MIL. VAPOR BARRIER ATOP A 12" THICK BASE OF CRUSHED #57 STONE.
- SLABS UTILIZING 6X6-W2.9XW2.9 WELDED WIRE MESH SHOULD HAVE CONTROL JOINTS @ MAXIMUM 12 FT O.C.E.W. TO REDUCE EXCESSIVE TEMPERATURE & SHRINKAGE CRACKING.
- ALL TURNDOWN SLAB FOOTINGS MUST HAVE A MINIMUM PERIMETER HAUNCH DEPTH OF 20" (U.N.O.).
- SEE ARCH. DRAWINGS FOR DIMENSIONS NOT SHOWN. IF A CONFLICT EXISTS, THE ARCH. DIMENSIONS SHALL GOVERN. BOTH ENGINEER AND ARCHITECT OF RECORD SHALL BE NOTIFIED IN WRITING OF ALL CONFLICTS.
- FOR TYPICAL DETAILS SEE S500 SERIES SHEETS.

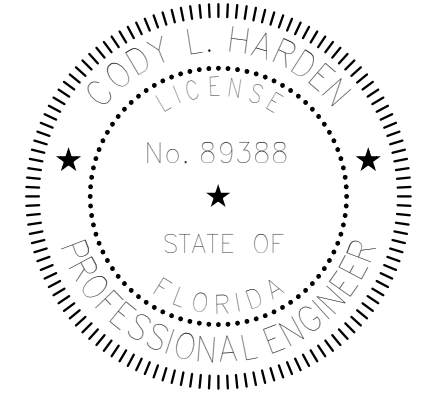
FOOTING NOTES

- TOPS OF ALL FOOTINGS (EXCEPT TURNDOWNS) MUST BE COVERED WITH A MINIMUM OF 12" OF FINISH GRADE MATERIAL, (U.N.O.).
- BOTTOMS OF ALL FOOTINGS MUST BEAR ATOP UNDISTURBED SOIL OR ENGINEERED FILL COMPACTED TO 95% MAXIMUM MODIFIED PROCTOR A MINIMUM OF 12" BELOW EXISTING GRADE UNLESS PROPER PROVISIONS HAVE BEEN MADE FOR THE USE OF ENGINEERED FILL BASED ON A SITE SPECIFIC GEOTECHNICAL EVALUATION (BY OTHERS).
- MINIMUM FOOTING WIDTHS MUST ALLOW FOR A MINIMUM 8" OFFSET FROM BOTH FACES OF CMU WALLS (TYP).
- ALL CONTINUOUS FOOTINGS & PEDESTALS MUST BE CENTERED ON THE WALL/PIER UNLESS SPECIFICALLY NOTED AS ECCENTRIC FOOTING.
- FOOTING SIZES, REINFORCING TYPE & SPACING LISTED WITHIN THE FOOTING SCHEDULE SHALL GOVERN OVER FOOTING NOTES.
- VERTICAL DOWEL HOOKS SHOULD EXTEND 9" IN TO FTG'S, BELOW & RUNNING PERPENDICULAR TO FLEXURAL REINF.
- NO "WET STICK" SETTING OF REINFORCING WILL BE APPROVED.
- ALL VERTICAL REINFORCING MUST BE IN PLACE & TIED TO FOOTING REINF. PRIOR TO POUR.
- COORDINATE ALL FOOTING LOCATIONS WITH CIVIL, PLUMBING, AND ELECTRICAL DRAWINGS. CONTRACTOR SHALL PROVIDE PIPE SLEEVES BELOW OR ABOVE CONTINUOUS WALL FOOTINGS.
- SEE ARCH. DRAWINGS FOR DIMENSIONS NOT SHOWN. IF A CONFLICT EXISTS, THE ARCH. DIMENSIONS SHALL GOVERN. BOTH ENGINEER AND ARCHITECT OF RECORD SHALL BE NOTIFIED IN WRITING OF ALL CONFLICTS.
- FOR TYPICAL DETAILS SEE S500 SERIES SHEETS.

CONCRETE BEAM SCHEDULE					
DESIGNATION	DIMENSIONS	TOP REINF.	MID REINF.	BTM REINF.	CLOSED SHEAR REINF. @ BEARING E.E
CB-1	16x20	(3) #6 BARS	(2) #5 BARS	(3) #6 BARS	#4 TIES @ 4" O.C.

CONCRETE COLUMN SCHEDULE		
DESIGNATION	FLEXURAL REINF.	SHEAR REINF.
CC-1	(10) #5 BARS	#4 CLOSED TIES @ 12" O.C.
CC-2	(10) #6 BARS	#4 CLOSED TIES @ 12" O.C.
CC-3	(6) #6 BARS	#4 CLOSED TIES @ 12" O.C.

FOOTING SCHEDULE				
DESIGNATION	TYPE	DIMENSIONS	LONGITUDINAL REINF.	TRANS. REINF.
FTG-24	STRIP	12" D x 24" W	(3) #5 BARS	#5 BARS x 18" @ 18" O.C.
FTG-36	STRIP	12" D x 36" W	(4) #5 BARS	#5 BARS x 30" @ 18" O.C.
FTG-48	STRIP	12" D x 48" W	(5) #5 BARS	#5 BARS x 42" @ 12" O.C.
FTG-60	STRIP	12" D x 60" W	(8) #6 BARS T&B	#5 BARS x 48" @ 10" O.C.
PED-36	PEDESTAL FTG.	14" D x 36" SQR.	(3) #5 BARS EQ. SPACED	(3) #5 BARS EQ. SPACED
PED-48	PEDESTAL FTG.	14" D x 48" SQR.	(5) #5 BARS EQ. SPACED	(5) #5 BARS EQ. SPACED
PED-72	PEDESTAL FTG.	14" D x 72" SQR.	(8) #5 BARS EQ. SPACED	(8) #5 BARS EQ. SPACED
TD-8	TURNDOWN	16" D x 8" W	(1) #5 BAR	N/A
TD-12	TURNDOWN	16" D x 12" W	(2) #5 BARS	#5 BARS x 6" @ 18" O.C.
TD-36	TURNDOWN	16" D x 36" W	(4) #5 BARS	#5 BARS x 30" @ 18" O.C.
TD-48	TURNDOWN	16" D x 48" W	(6) #5 BARS	#5 BARS x 42" @ 18" O.C.
TS-48	STRIP	16" D x 48" W	(6) #5 BARS	#5 BARS x 42" @ 18" O.C.



Digital Signature

FOR PERMIT

GROVES & KING, LLC

**GARAGES
LUXURY AT
MIRAMAR -
PHASE 1**

U.S. Highway 98
Miramar Beach, FL

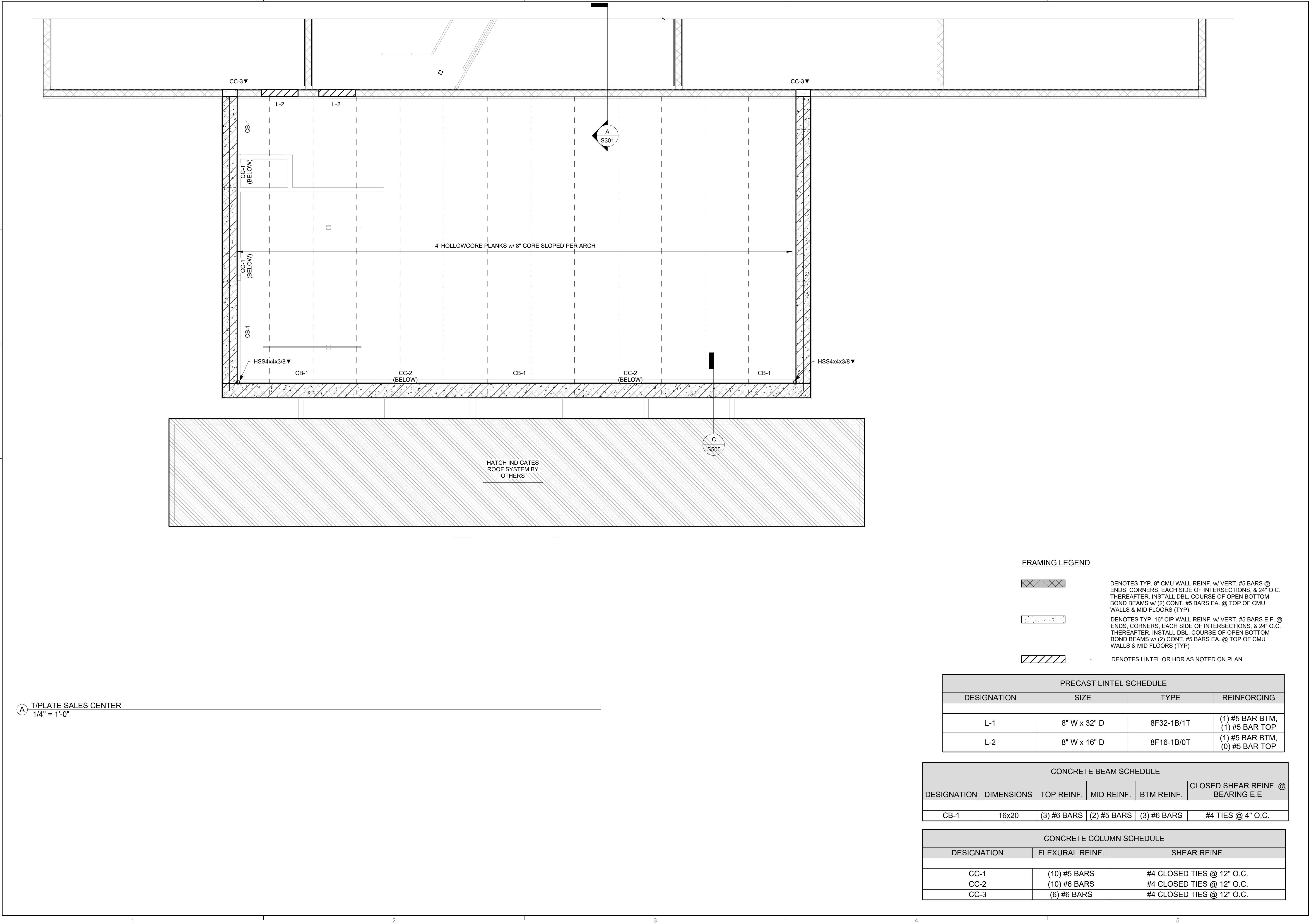
REVISIONS
NO. DESCRIPTION DATE

Project Number **25038**
Dated 10.05.2026

**FOUNDATION
PLAN -
BUILDINGS A & B**

S102

© 2023 DAG ARCHITECTS, INC. ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE COPIED, REPRODUCED OR TRANSMITTED IN ANY MANNER WITHOUT WRITTEN PERMISSION OF DAG ARCHITECTS, INC. THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY AND COPYRIGHT OF DAG ARCHITECTS, INC. AND SHALL NOT BE USED ON ANY OTHER PROJECT OR LOCATION EXCEPT AS DESCRIBED ON THE DRAWINGS WITHOUT THE PRIOR WRITTEN AGREEMENT OF DAG ARCHITECTS.



A T/PLATE SALES CENTER
1/4" = 1'-0"

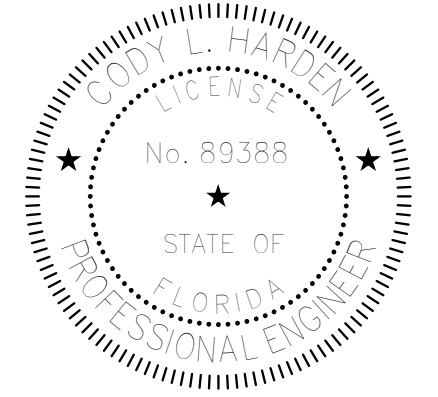
FRAMING LEGEND

- DENOTES TYP. 8" CMU WALL REINF. w/ VERT. #5 BARS @ ENDS, CORNERS, EACH SIDE OF INTERSECTIONS, & 24" O.C. THEREAFTER. INSTALL DBL. COURSE OF OPEN BOTTOM BOND BEAMS w/ (2) CONT. #5 BARS EA. @ TOP OF CMU WALLS & MID FLOORS (TYP)
- DENOTES TYP. 16" CIP WALL REINF. w/ VERT. #5 BARS E.F. @ ENDS, CORNERS, EACH SIDE OF INTERSECTIONS, & 24" O.C. THEREAFTER. INSTALL DBL. COURSE OF OPEN BOTTOM BOND BEAMS w/ (2) CONT. #5 BARS EA. @ TOP OF CMU WALLS & MID FLOORS (TYP)
- DENOTES LINTEL OR HDR AS NOTED ON PLAN.

PRECAST LINTEL SCHEDULE			
DESIGNATION	SIZE	TYPE	REINFORCING
L-1	8" W x 32" D	8F32-1B/1T	(1) #5 BAR BTM, (1) #5 BAR TOP
L-2	8" W x 16" D	8F16-1B/0T	(1) #5 BAR BTM, (0) #5 BAR TOP

CONCRETE BEAM SCHEDULE					
DESIGNATION	DIMENSIONS	TOP REINF.	MID REINF.	BTM REINF.	CLOSED SHEAR REINF. @ BEARING E.E
CB-1	16x20	(3) #6 BARS	(2) #5 BARS	(3) #6 BARS	#4 TIES @ 4" O.C.

CONCRETE COLUMN SCHEDULE		
DESIGNATION	FLEXURAL REINF.	SHEAR REINF.
CC-1	(10) #5 BARS	#4 CLOSED TIES @ 12" O.C.
CC-2	(10) #6 BARS	#4 CLOSED TIES @ 12" O.C.
CC-3	(6) #6 BARS	#4 CLOSED TIES @ 12" O.C.



Digital Signature

FOR PERMIT

GROVES & KING, LLC

GARAGES
LUXURY AT
MIRAMAR -
PHASE 1

U.S. Highway 98
Miramar Beach, FL

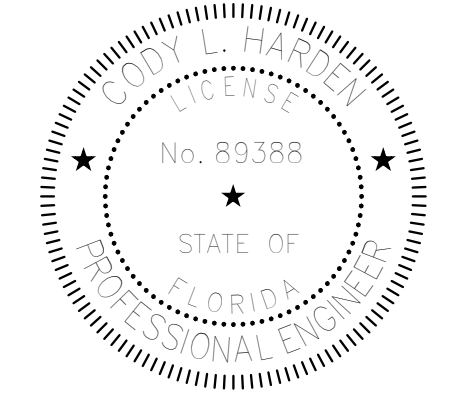
REVISIONS		
NO.	DESCRIPTION	DATE

Project Number	25038
Dated	10.05.2026

ROOF FRAMING
PLAN - SALES
CENTER

S103

© 2023 DAG ARCHITECTS, INC. ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE COPIED, REPRODUCED OR TRANSMITTED BY ANY MEANS WITHOUT WRITTEN PERMISSION OF DAG ARCHITECTS, INC. THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY AND COPYRIGHT OF DAG ARCHITECTS, INC. AND SHALL NOT BE USED ON ANY OTHER PROJECT OR LOCATION EXCEPT AS DESCRIBED ON THE DRAWINGS. WITHOUT THE PRIOR WRITTEN AGREEMENT OF DAG ARCHITECTS.



Digital Signature
FOR PERMIT

GROVES & KING, LLC

**GARAGES
LUXURY AT
MIRAMAR -
PHASE 1**

U.S. Highway 98
Miramar Beach, FL

REVISIONS		
NO.	DESCRIPTION	DATE

Project Number **25038**
Dated 10.05.2026

ROOF FRAMING
PLAN -
BUILDINGS A & B

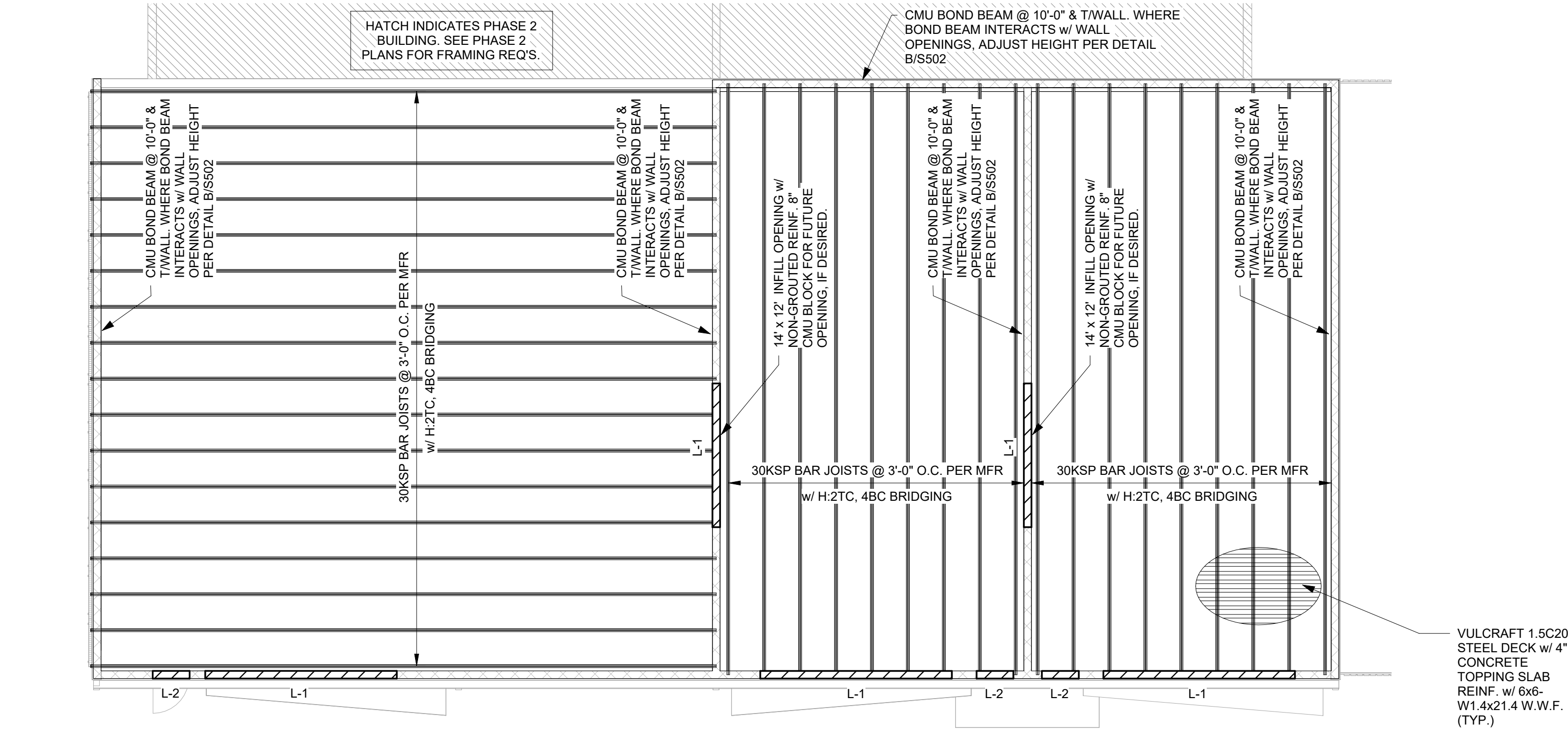
S104

FRAMING LEGEND

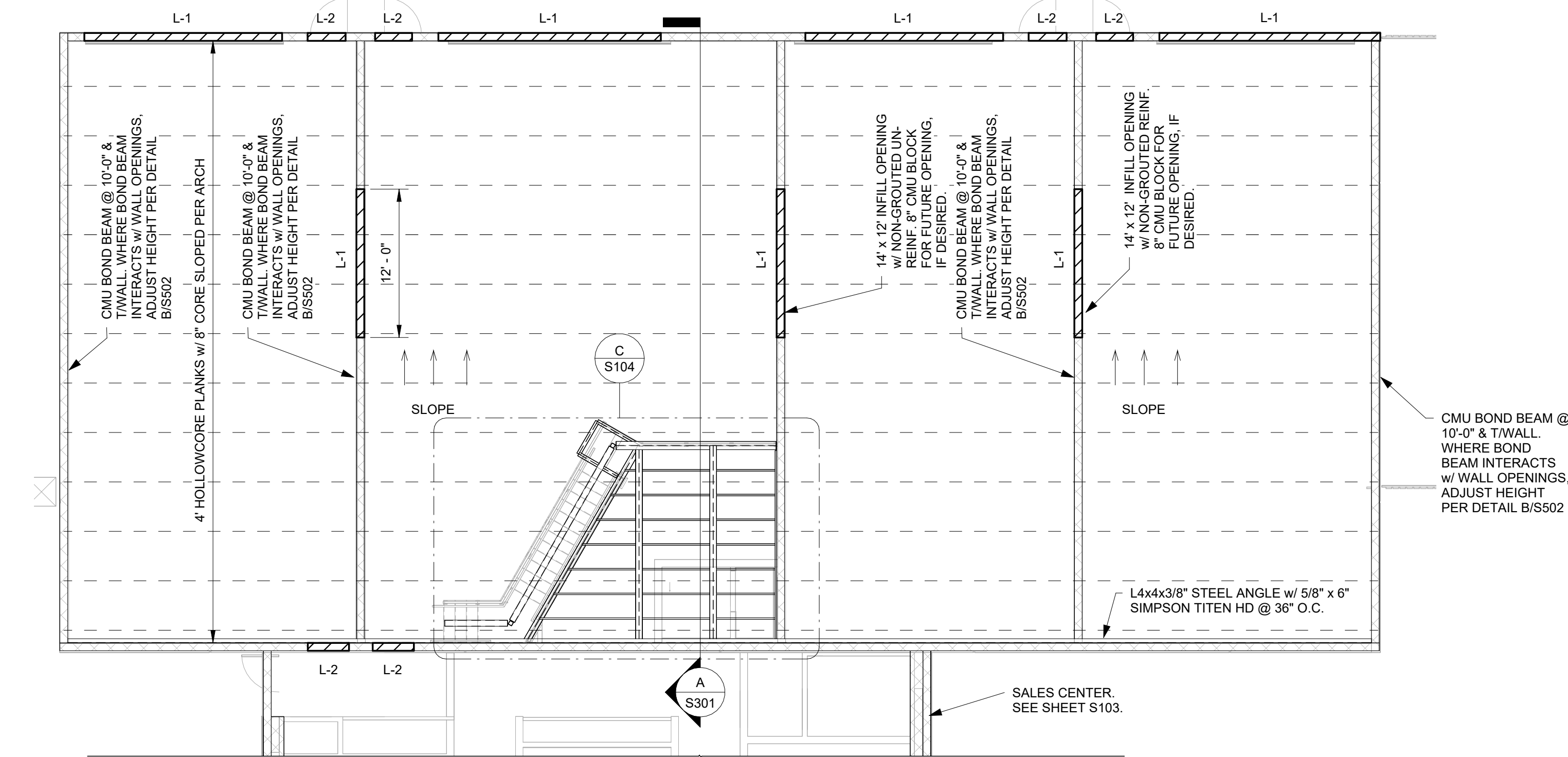
- DENOTES TYP. 8" CMU WALL REINF. w/ VERT. #5 BARS @ ENDS, CORNERS, EACH SIDE OF INTERSECTIONS, & 24" O.C. THEREAFTER, INSTALL DBL. COURSE OF OPEN BOTTOM BOND BEAMS w/ (2) CONT. #5 BARS EA. @ TOP OF CMU WALLS & MID FLOORS (TYP)
- DENOTES TYP. 16" CIP WALL REINF. w/ VERT. #5 BARS E.F. @ ENDS, CORNERS, EACH SIDE OF INTERSECTIONS, & 24" O.C. THEREAFTER, INSTALL DBL. COURSE OF OPEN BOTTOM BOND BEAMS w/ (2) CONT. #5 BARS EA. @ TOP OF CMU WALLS & MID FLOORS (TYP)
- DENOTES LINTEL OR HDR AS NOTED ON PLAN.

PRECAST LINTEL SCHEDULE			
DESIGNATION	SIZE	TYPE	REINFORCING
L-1	8" W x 32" D	8F32-1B/1T	(1) #5 BAR BTM, (1) #5 BAR TOP
L-2	8" W x 16" D	8F16-1B/0T	(1) #5 BAR BTM, (0) #5 BAR TOP

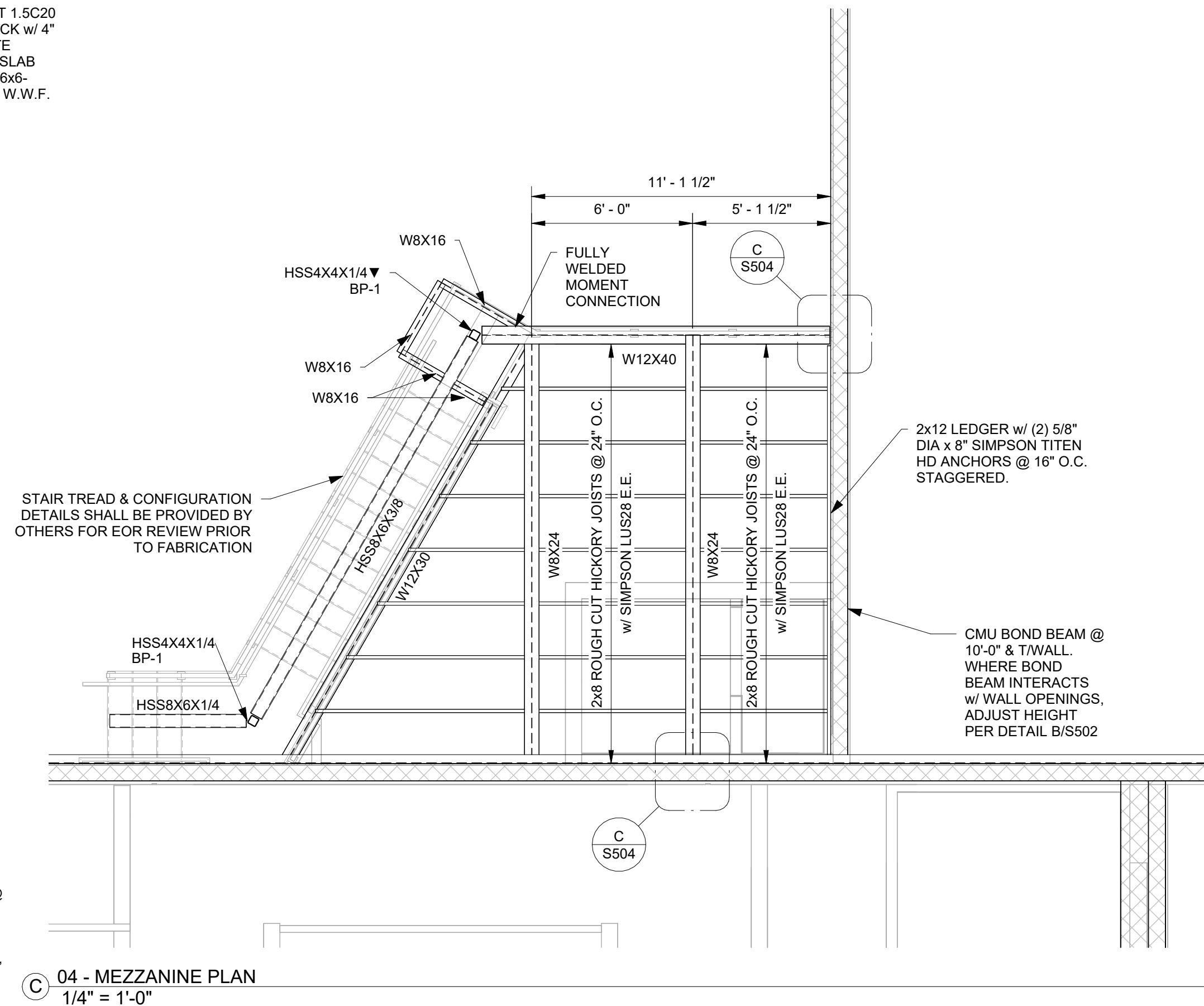
CONCRETE COLUMN SCHEDULE		
DESIGNATION	FLEXURAL REINF.	SHEAR REINF.
CC-1	(10) #5 BARS	#4 CLOSED TIES @ 12" O.C.
CC-2	(10) #6 BARS	#4 CLOSED TIES @ 12" O.C.
CC-3	(6) #6 BARS	#4 CLOSED TIES @ 12" O.C.



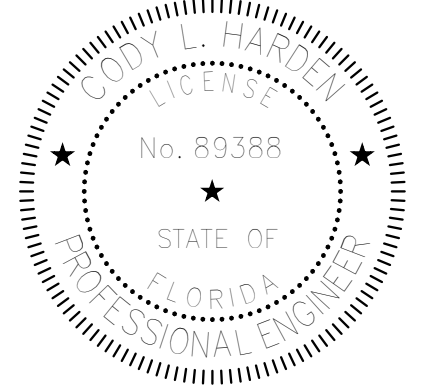
A 04 - T/ PLATE BUILDING B UNITS
1/8" = 1'-0"



B 04 - T/ PLATE BUILDING A UNITS
1/8" = 1'-0"



C 04 - MEZZANINE PLAN
1/4" = 1'-0"



Digital Signature
FOR PERMIT

GROVES & KING, LLC

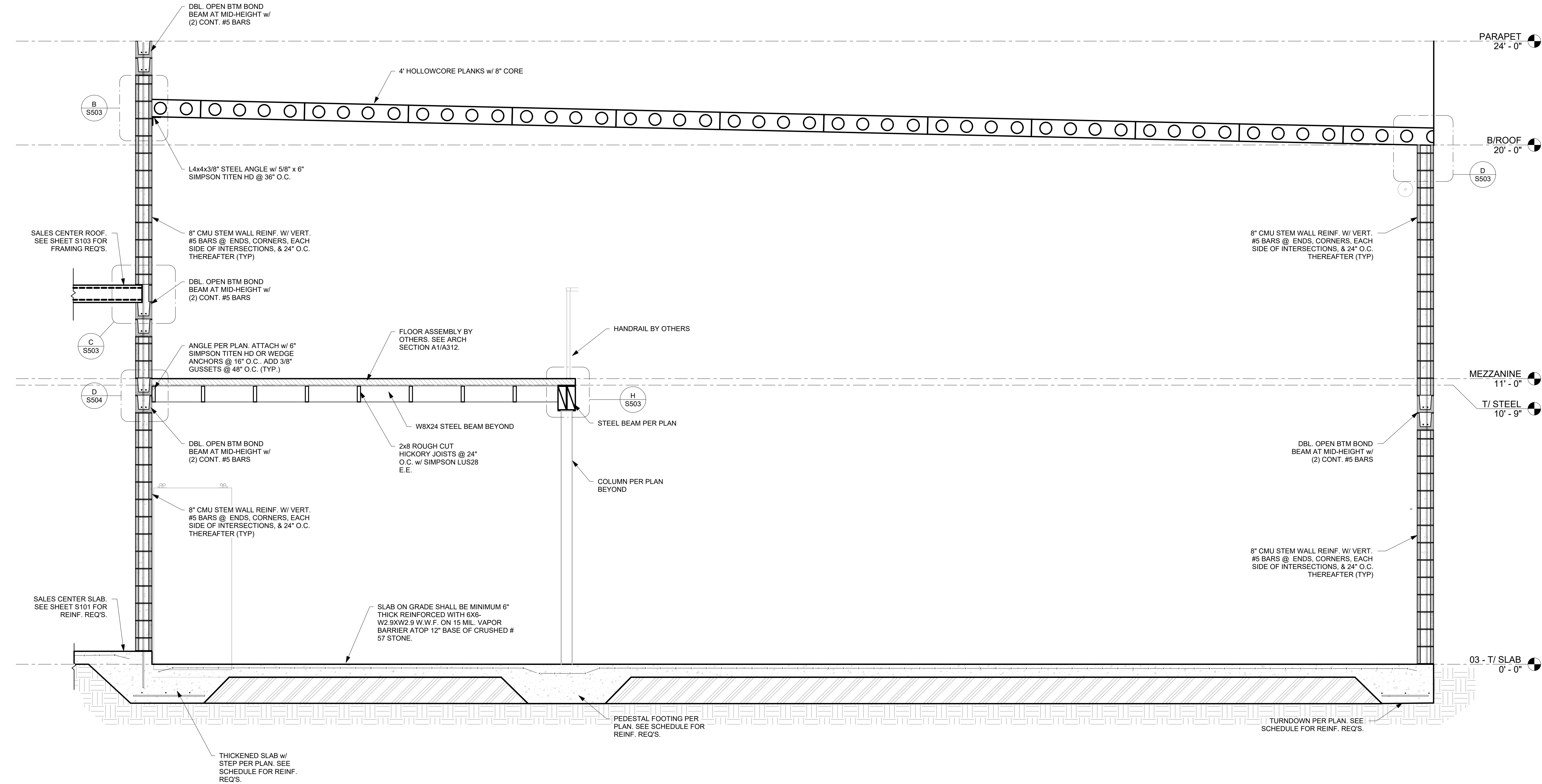
**GARAGES
LUXURY AT
MIRAMAR -
PHASE 1**

U.S. Highway 98
Miramar Beach, FL

REVISIONS		
NO.	DESCRIPTION	DATE

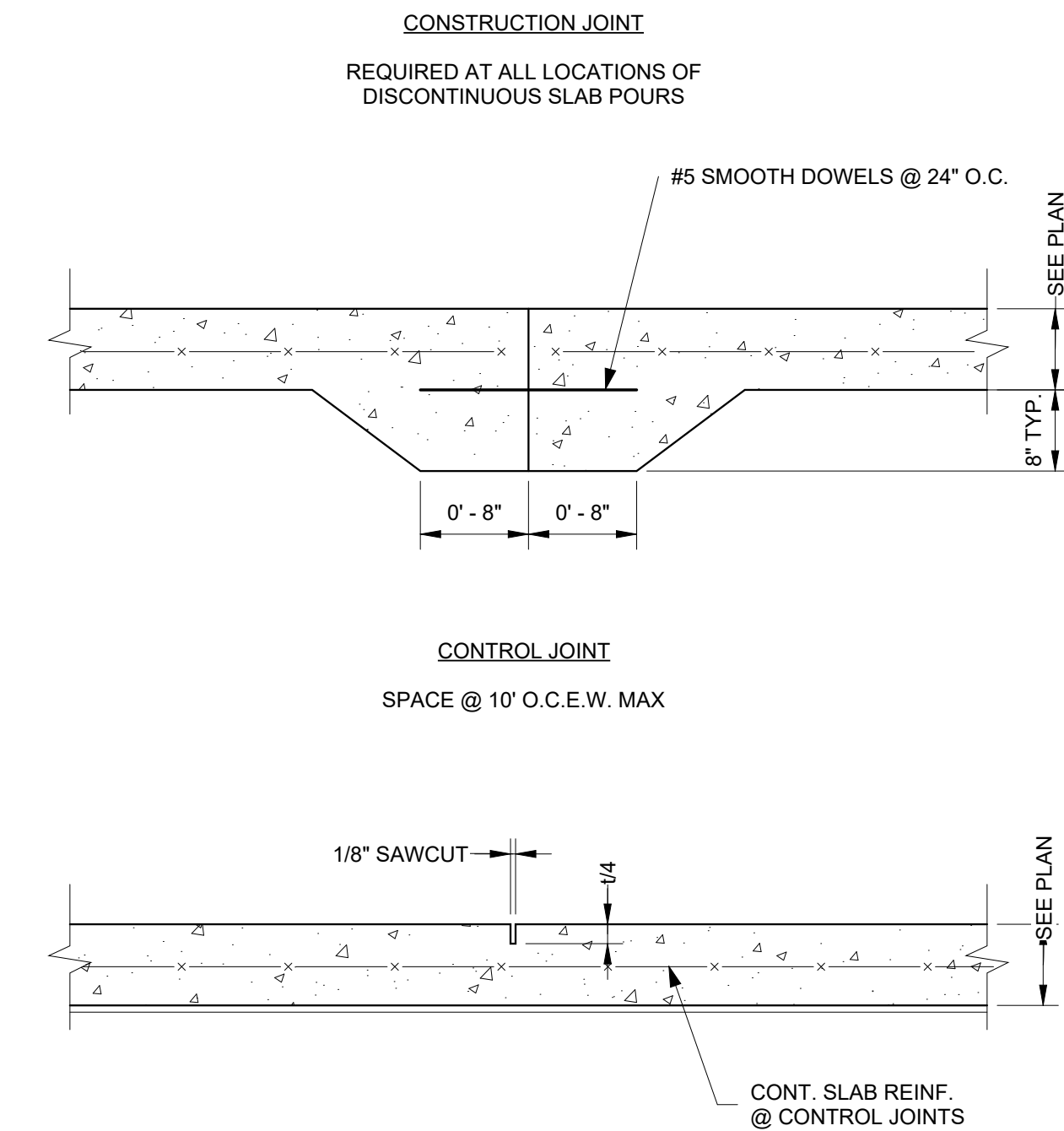
Project Number **25038**
Dated 10.05.2026

STRUCTURAL
SECTIONS
S301

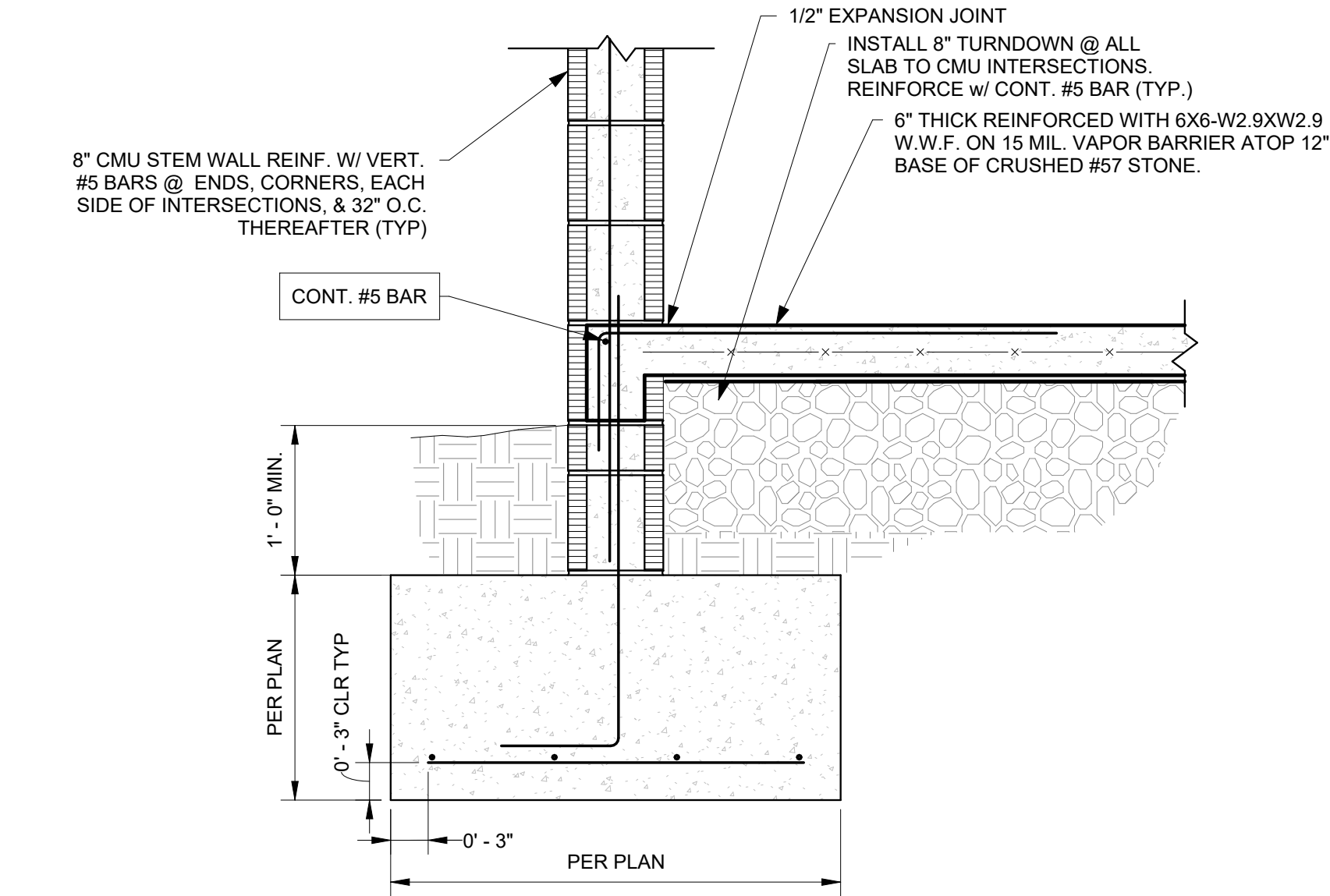


(A) TYP. WALL SECTION
1/2" = 1'-0"

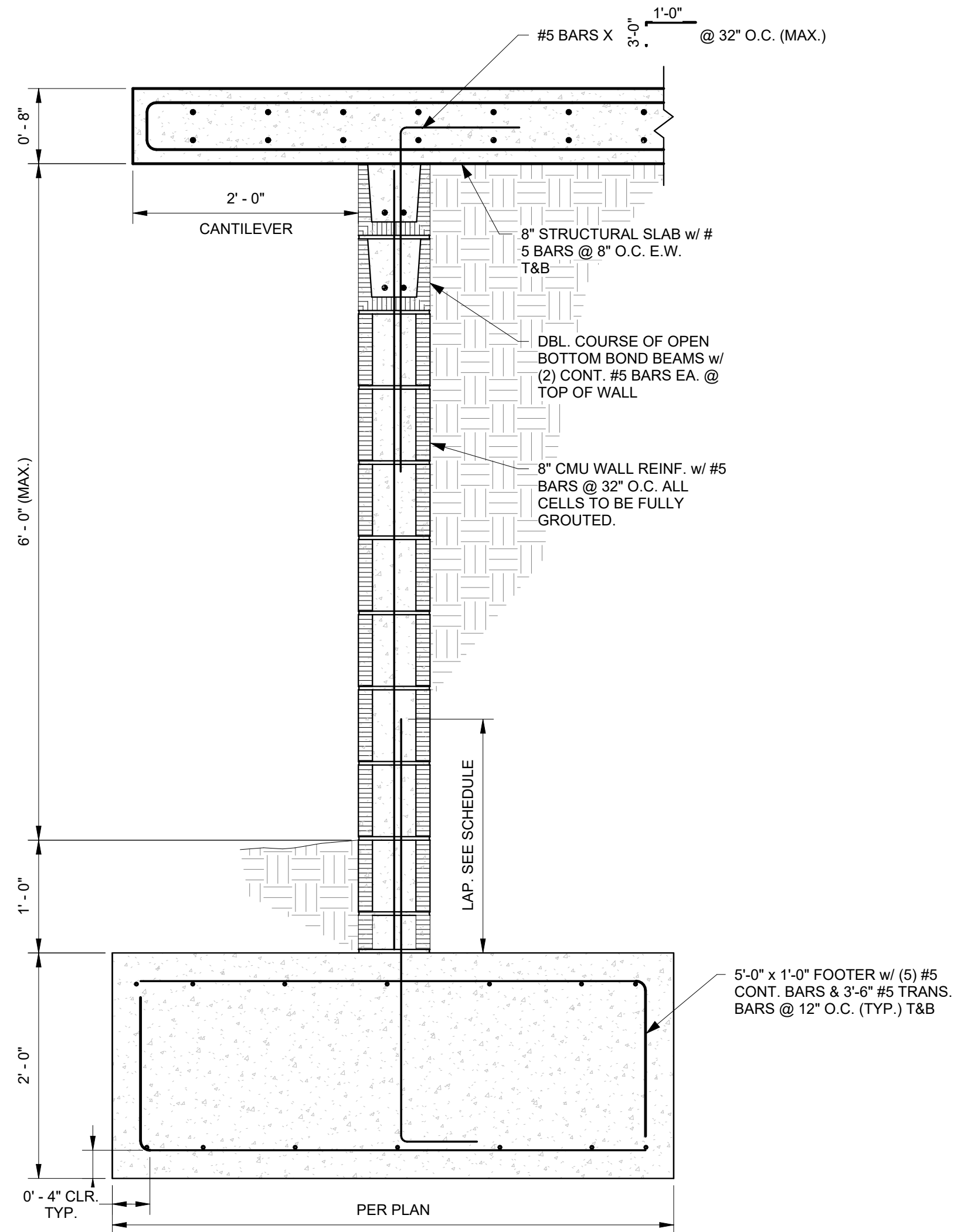
© 2023 DAG ARCHITECTS, INC. ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE COPIED, REPRODUCED OR TRANSMITTED IN ANY MANNER WITHOUT WRITTEN PERMISSION OF DAG ARCHITECTS, INC. THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY AND COPYRIGHT OF DAG ARCHITECTS, INC. AND SHALL NOT BE USED ON ANY OTHER PROJECT OR LOCATION EXCEPT AS DESCRIBED ON THE DRAWINGS WITHOUT THE PRIOR WRITTEN AGREEMENT OF DAG ARCHITECTS.



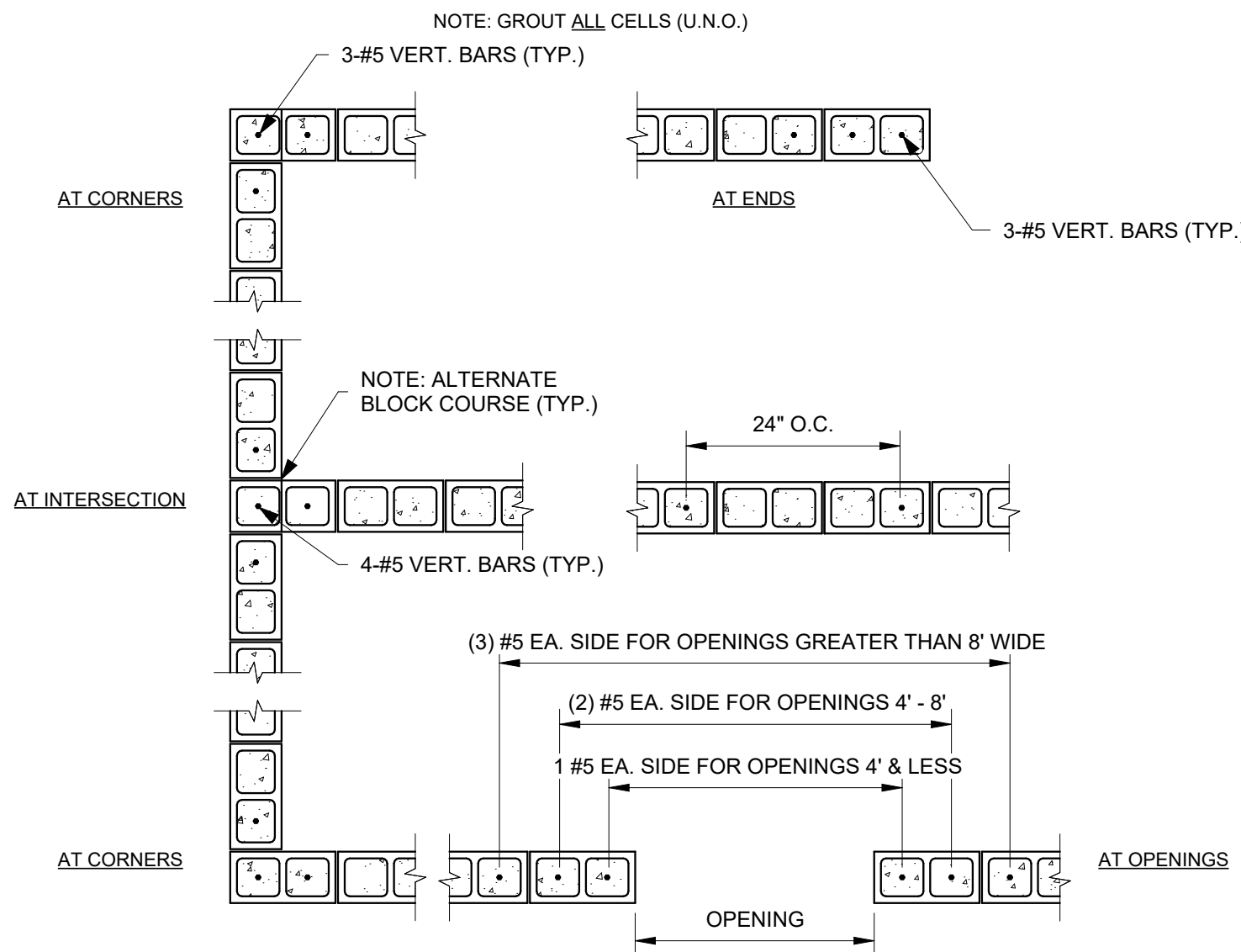
A TYP. SLAB FINISHING DETAIL
1" = 1'-0"



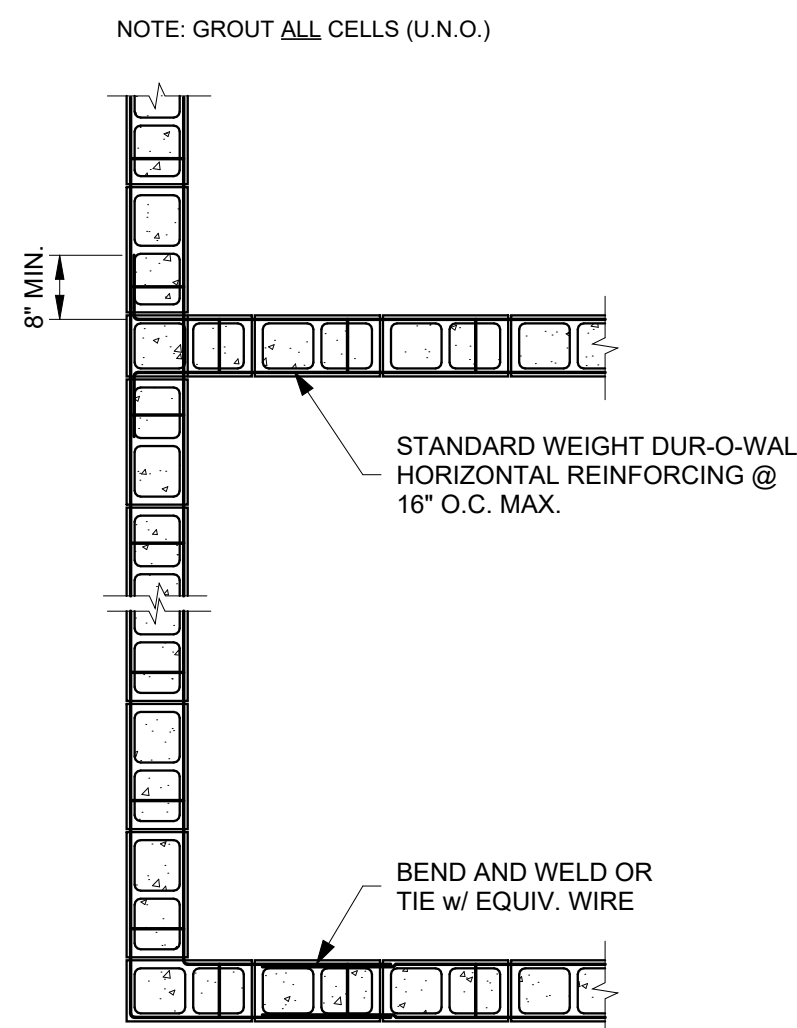
B TYP. PERIMETER FOOTING DETAIL
1" = 1'-0"



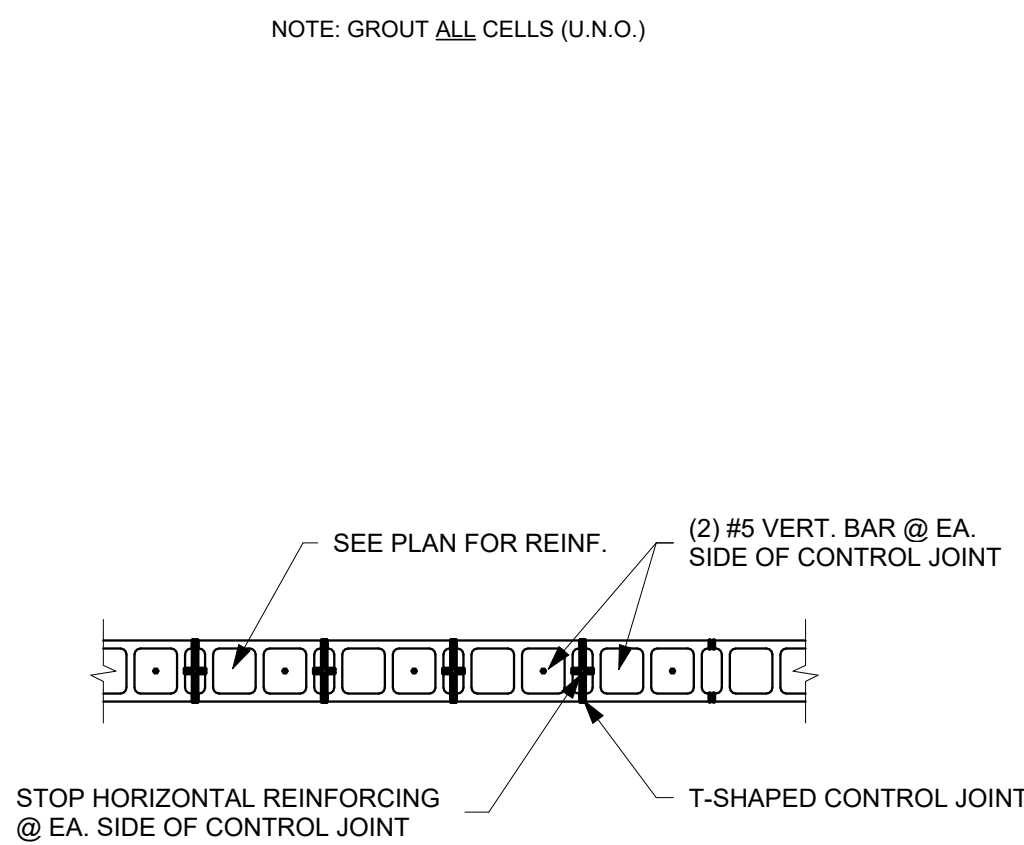
C TYP. 6 FT RETAINING WALL DETAIL
1" = 1'-0"



D TYP. CMU WALL VERT. REINFORCING DETAIL
1" = 1'-0"

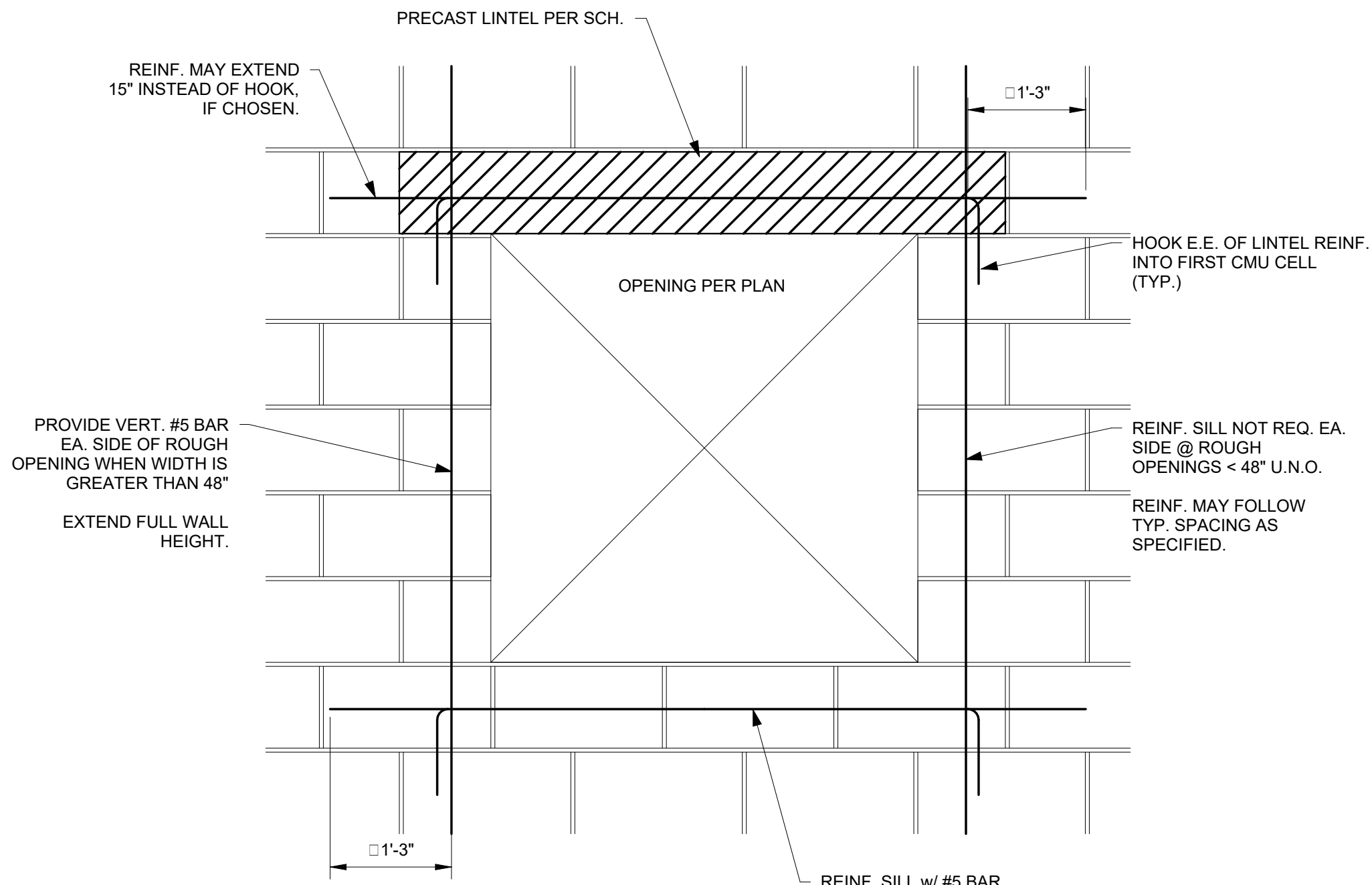


E TYP. CMU WALL HORIZ. REINFORCING DETAIL
1" = 1'-0"

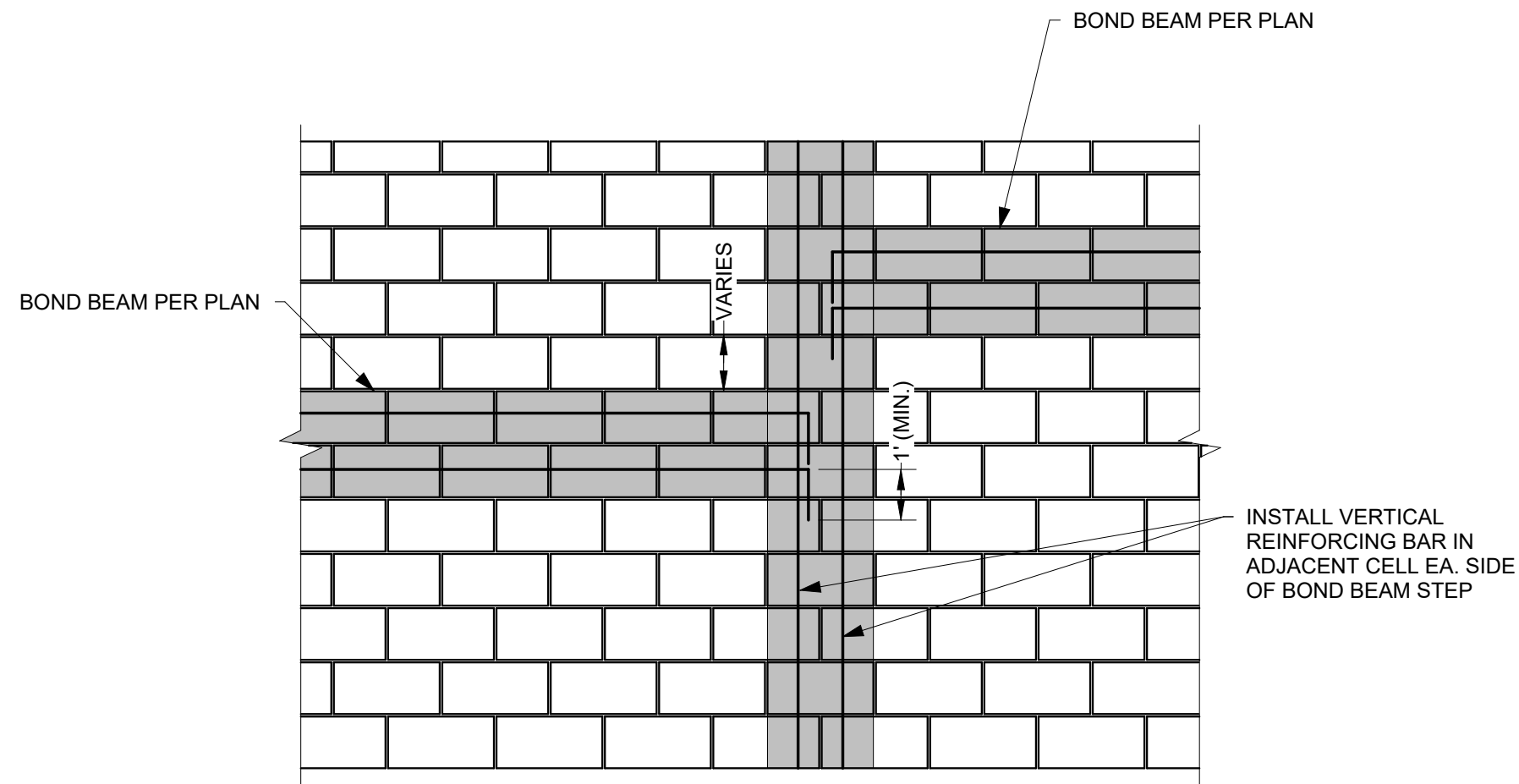


F TYP. CMU WALL CONTROL JOINT REINF.
1" = 1'-0"

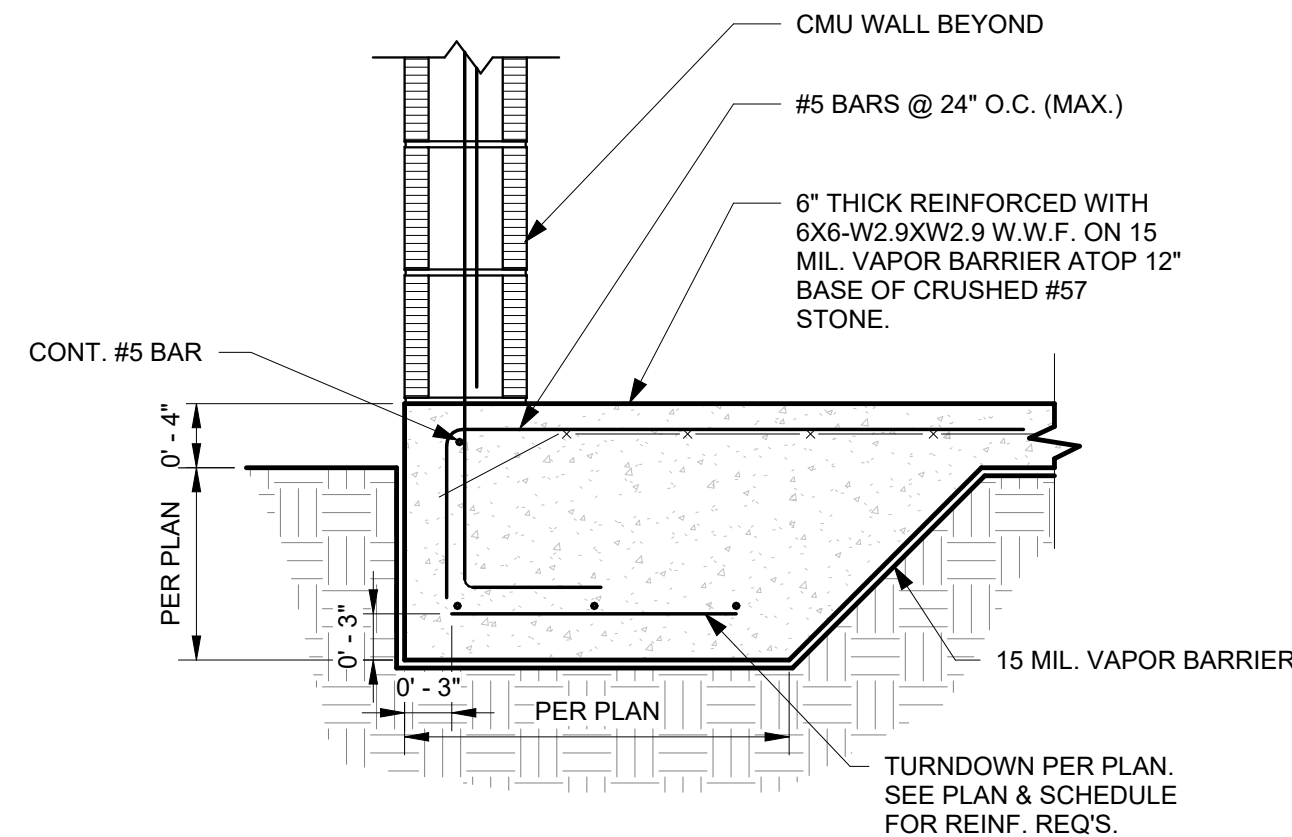
© 2024 DAG ARCHITECTS, INC. ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE COPIED, REPRODUCED OR TRANSMITTED IN ANY MANNER WITHOUT WRITTEN PERMISSION OF DAG ARCHITECTS, INC. THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY AND COPYRIGHT OF DAG ARCHITECTS, INC. AND SHALL NOT BE USED ON ANY OTHER PROJECT OR LOCATION EXCEPT AS DESCRIBED ON THE DRAWINGS. WITHOUT THE PRIOR WRITTEN AGREEMENT OF DAG ARCHITECTS.



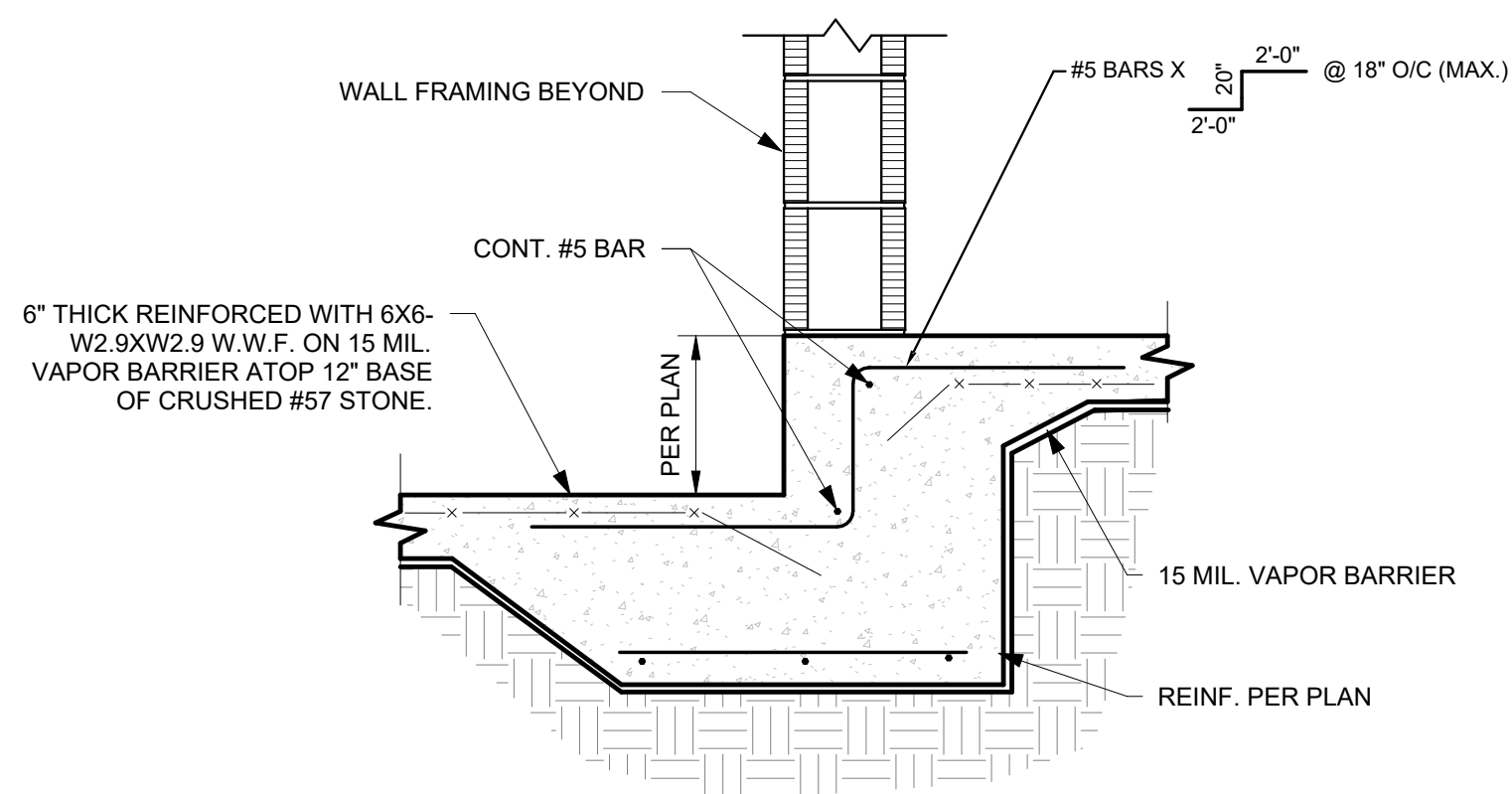
A TYP. CMU WALL OPENING DETAIL
1" = 1'-0"



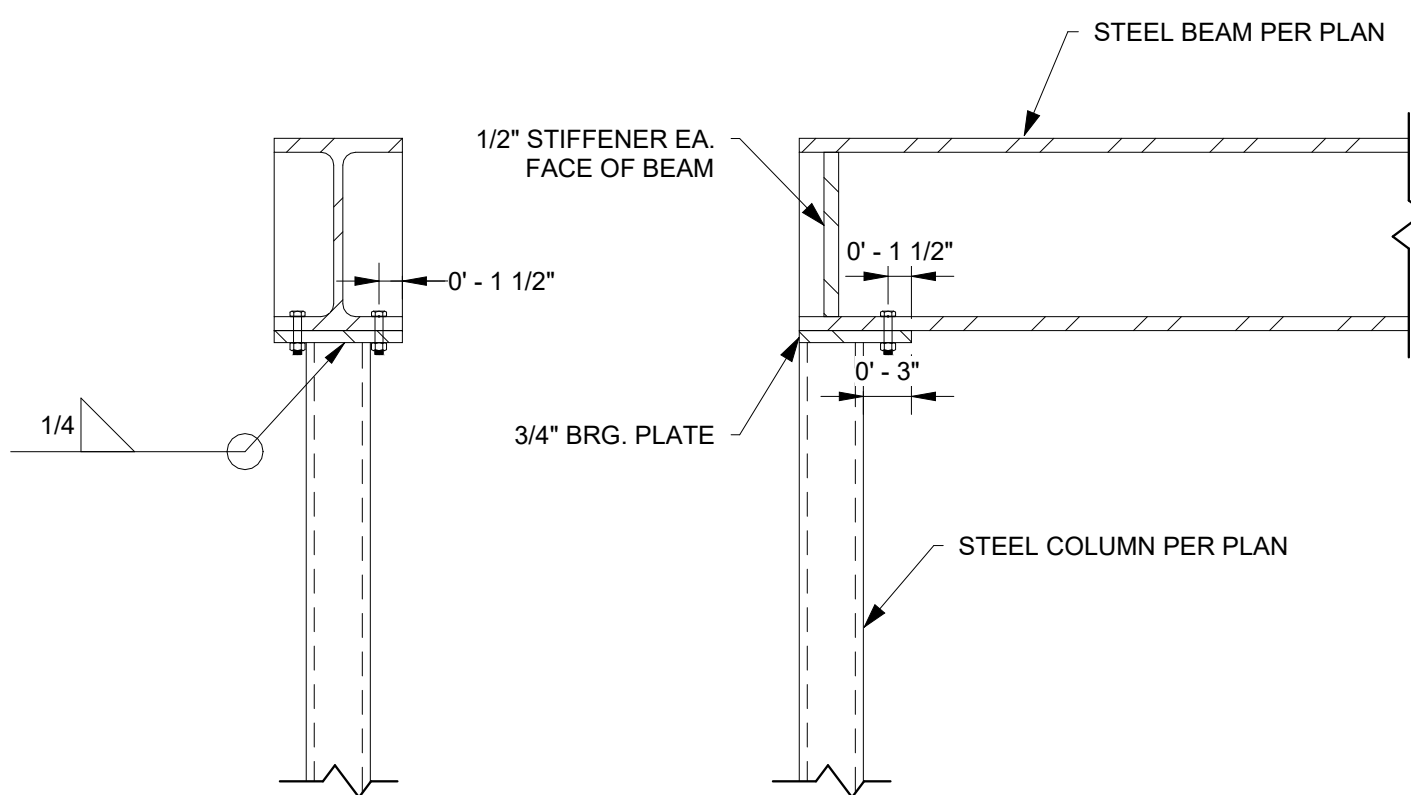
B TYP. BOND BEAM STEP DETAIL
1" = 1'-0"



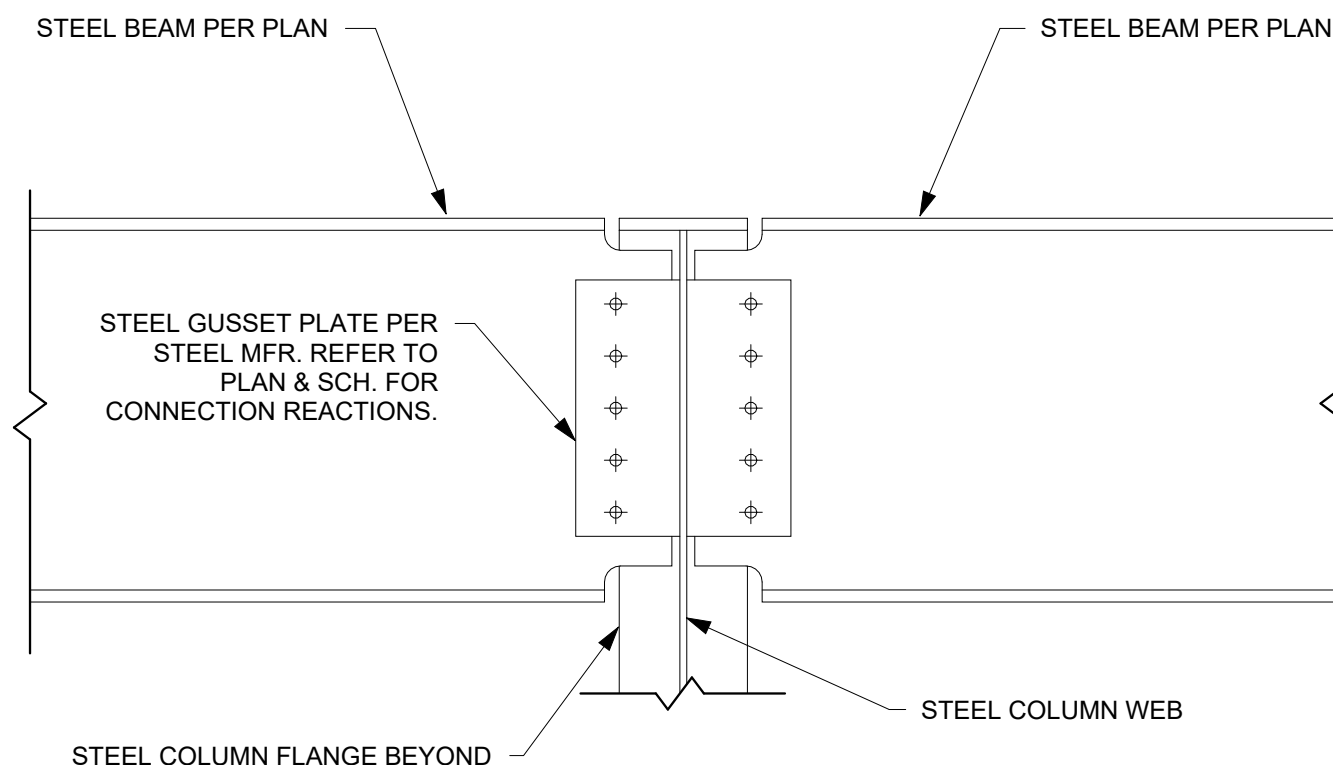
C TYP. TURNDOWN DETAIL
1" = 1'-0"



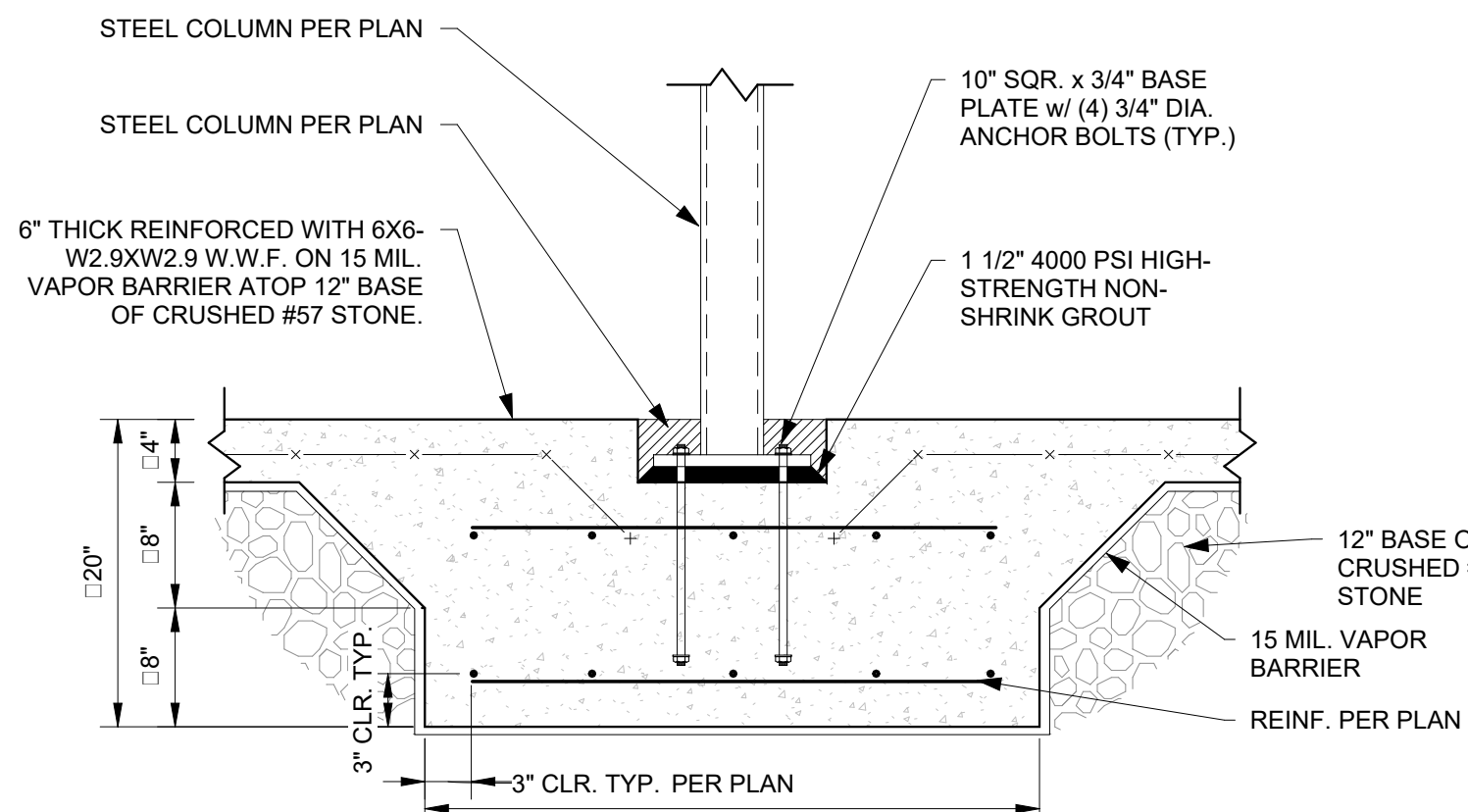
D THICKENED SLAB w/ STEPDOWN
1" = 1'-0"



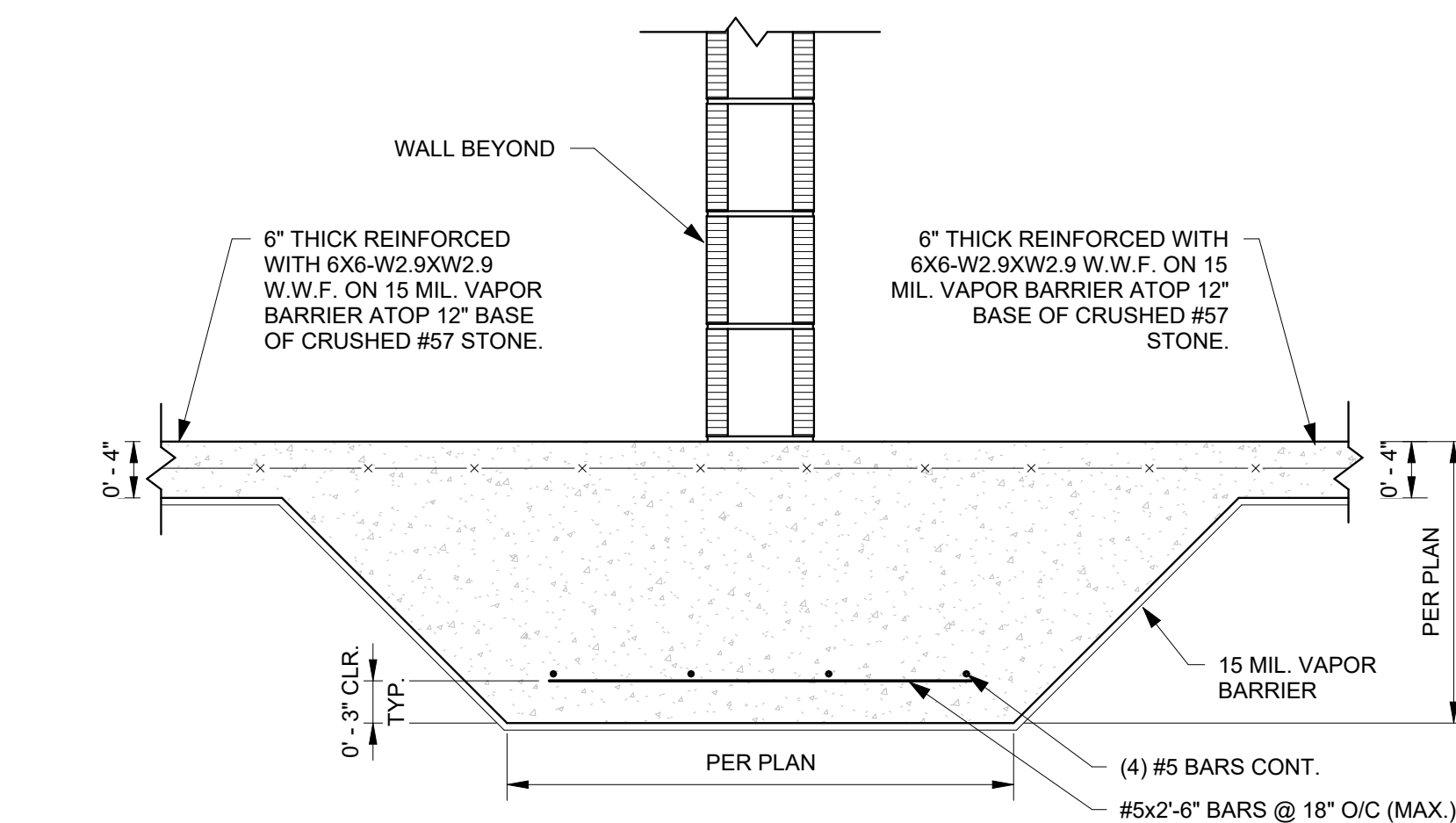
E TYP. STEEL BEAM TO COLUMN DETAIL
1" = 1'-0"



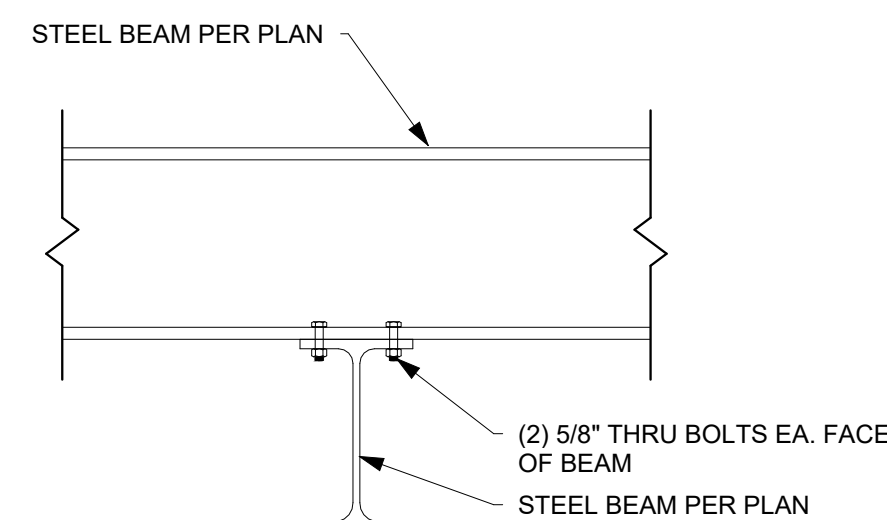
F TYP. STEEL BEAM TO COLUMN CONNECTION
1" = 1'-0"



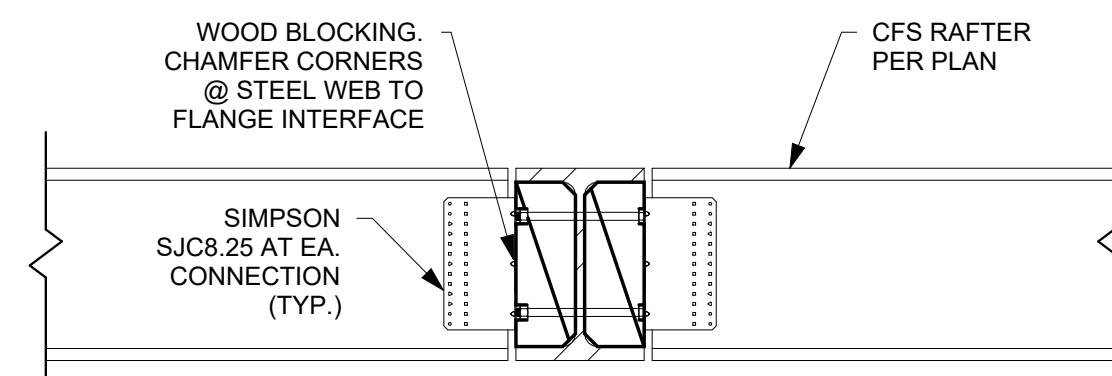
G TYP. STEEL COLUMN TO SLAB CONNECTION
1" = 1'-0"



H TYP. THICKENED SLAB DETAIL
1" = 1'-0"

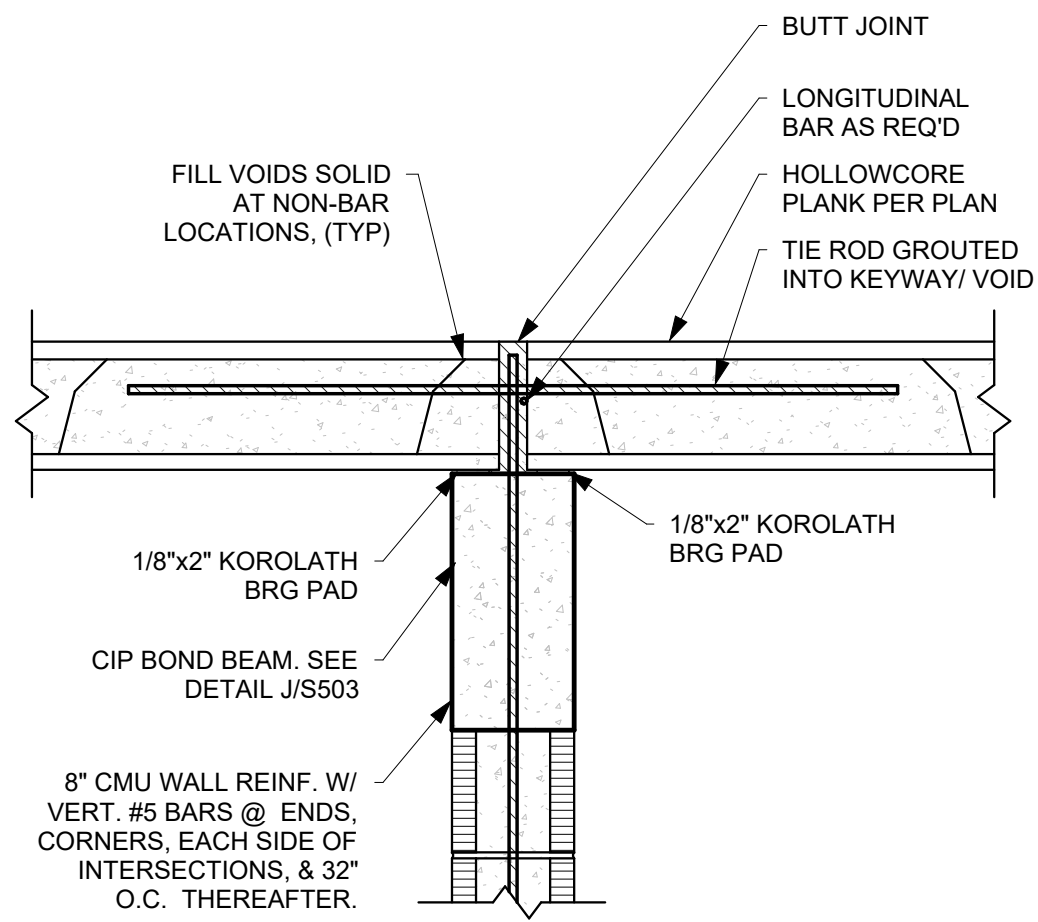


J TYP. STEEL BEAM ATOP BEAM CONNECTION
1" = 1'-0"

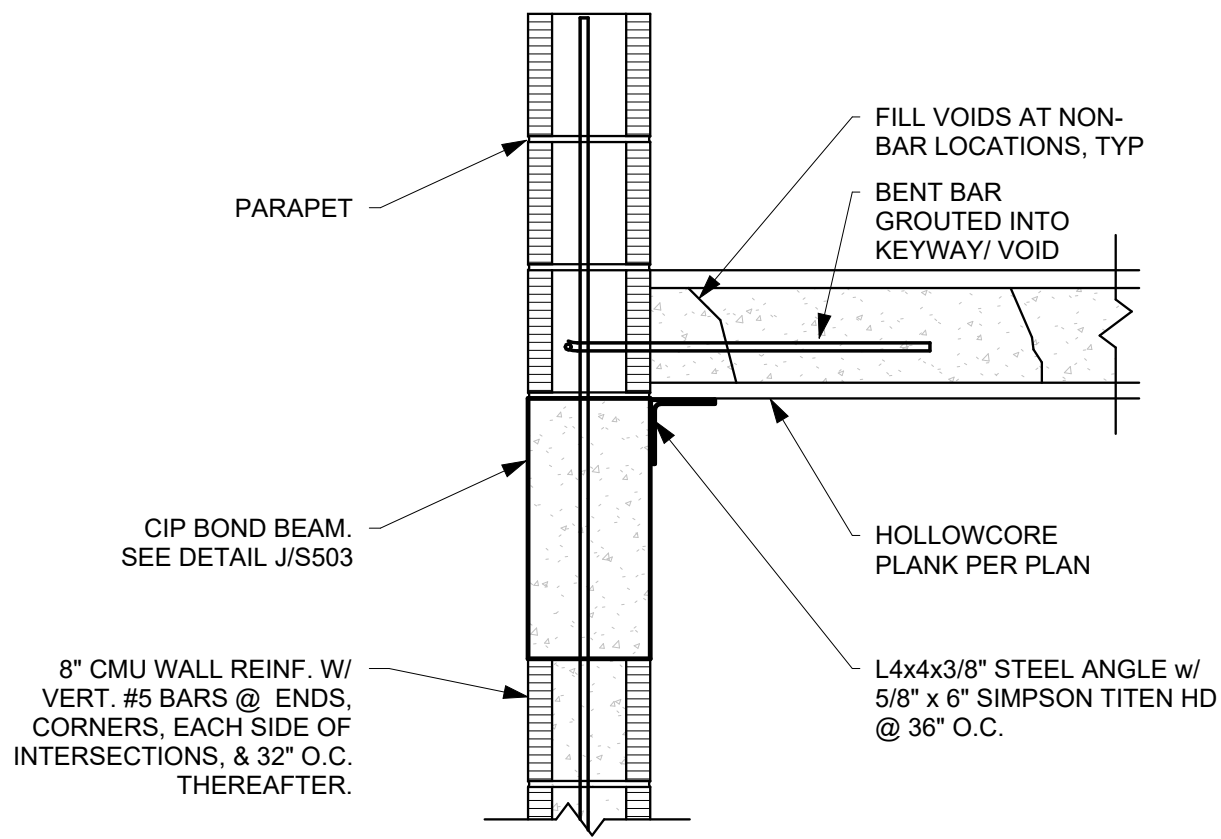


K TYP. STEEL BEAM TO CFS RAFTER CONNECTION
1" = 1'-0"

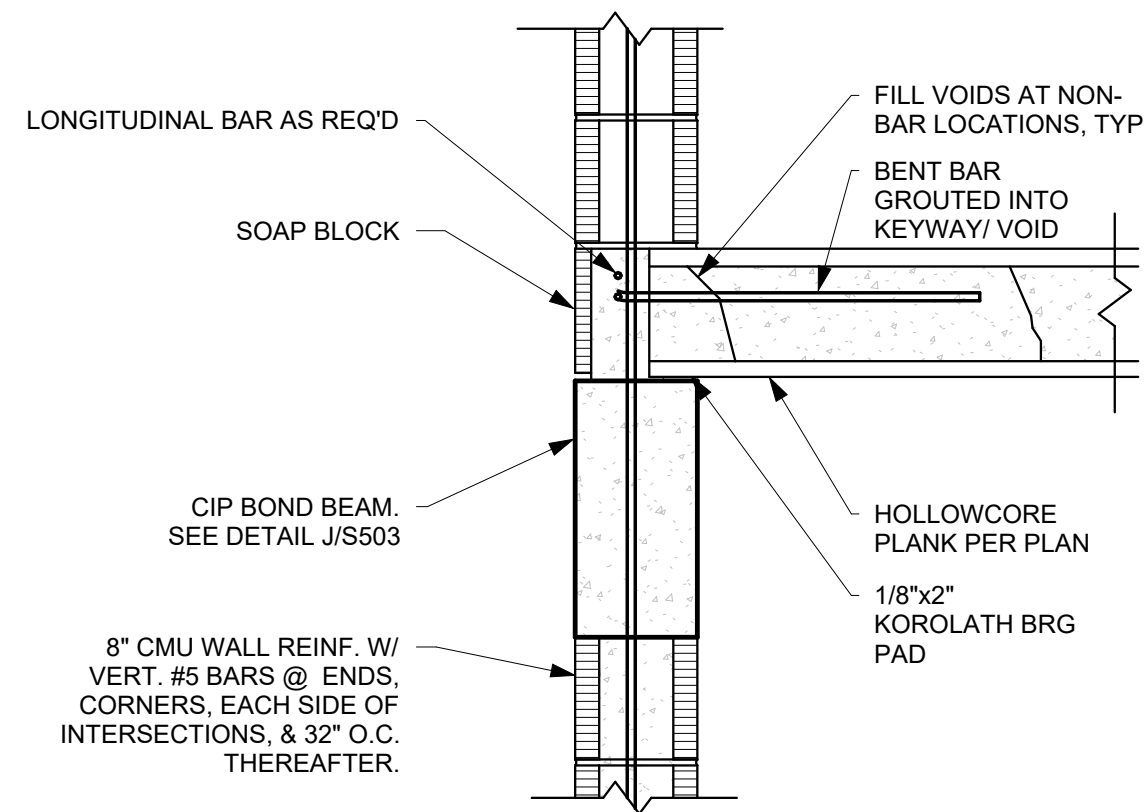
© 2023 DAG ARCHITECTS, INC. ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE COPIED, REPRODUCED OR TRANSMITTED IN ANY MANNER WITHOUT WRITTEN PERMISSION OF DAG ARCHITECTS, INC. THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY AND COPYRIGHT OF DAG ARCHITECTS, INC. AND SHALL NOT BE USED ON ANY OTHER PROJECT OR LOCATION EXCEPT AS DESCRIBED ON THE DRAWINGS. WITHOUT THE PRIOR WRITTEN AGREEMENT OF DAG ARCHITECTS.



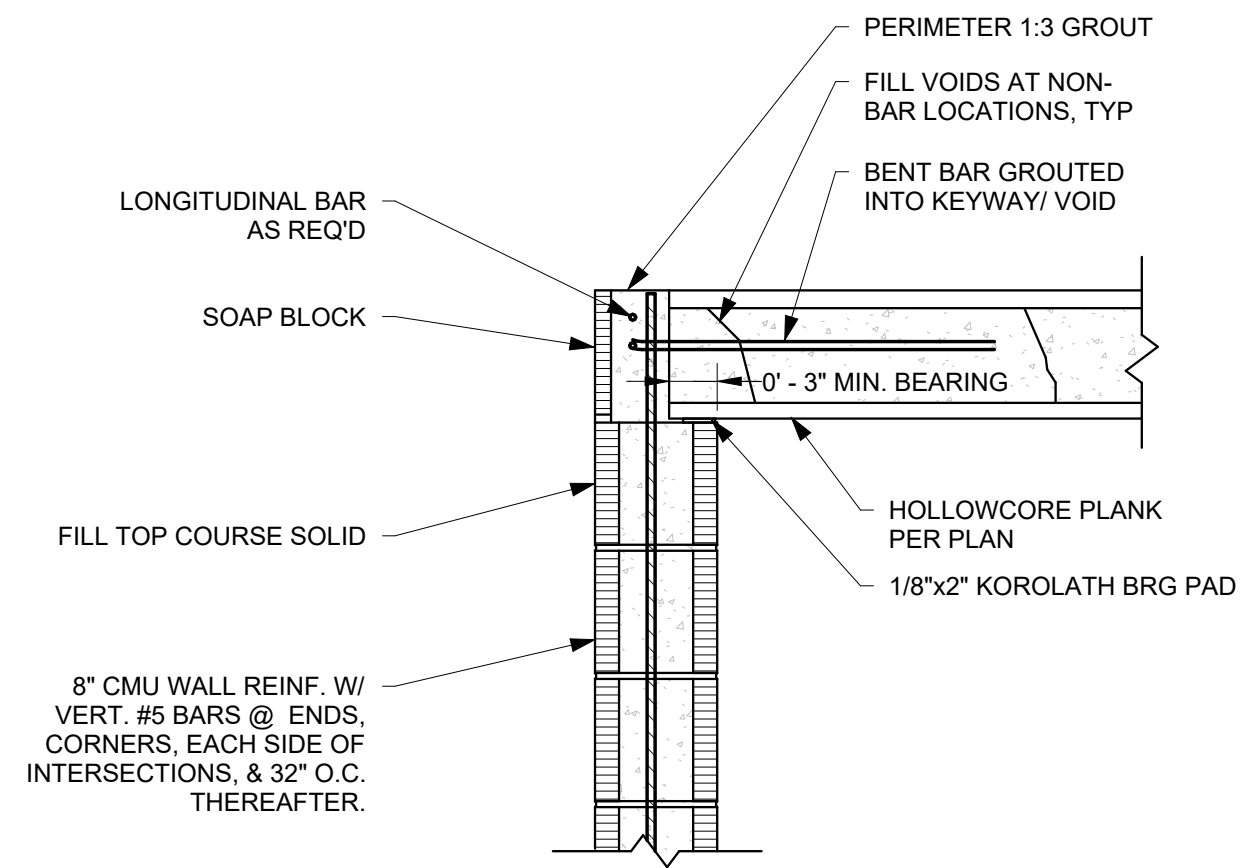
A TYP. HOLLOWCORE INTERIOR MASONRY BEARING
1" = 1'-0"



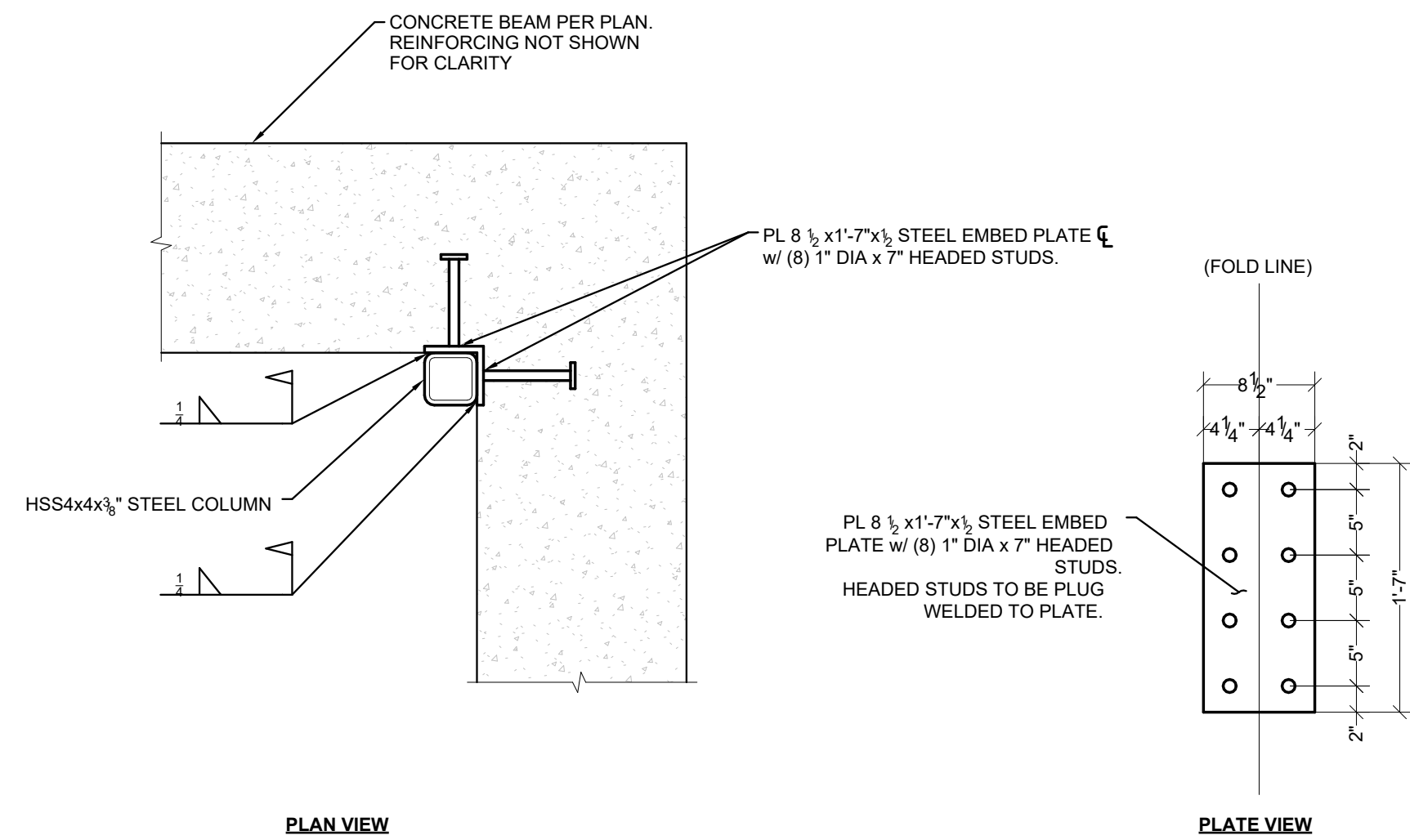
B TYP. HOLLOWCORE BEARING @ CONT. WALL CONDITION w/ ANGLE DETAIL
1" = 1'-0"



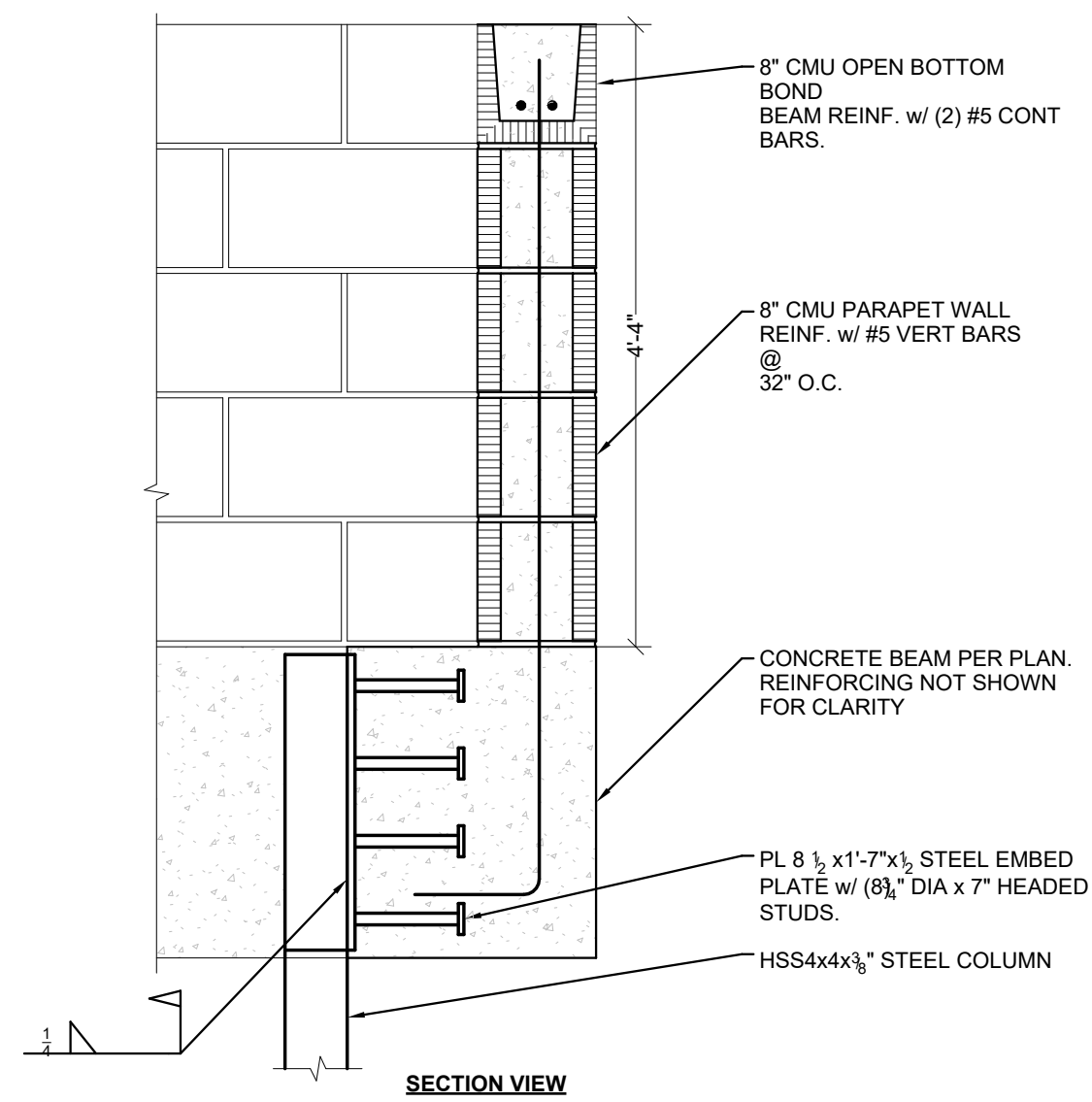
C TYP. HOLLOWCORE BEARING @ CONT. WALL CONDITION DETAIL
1" = 1'-0"



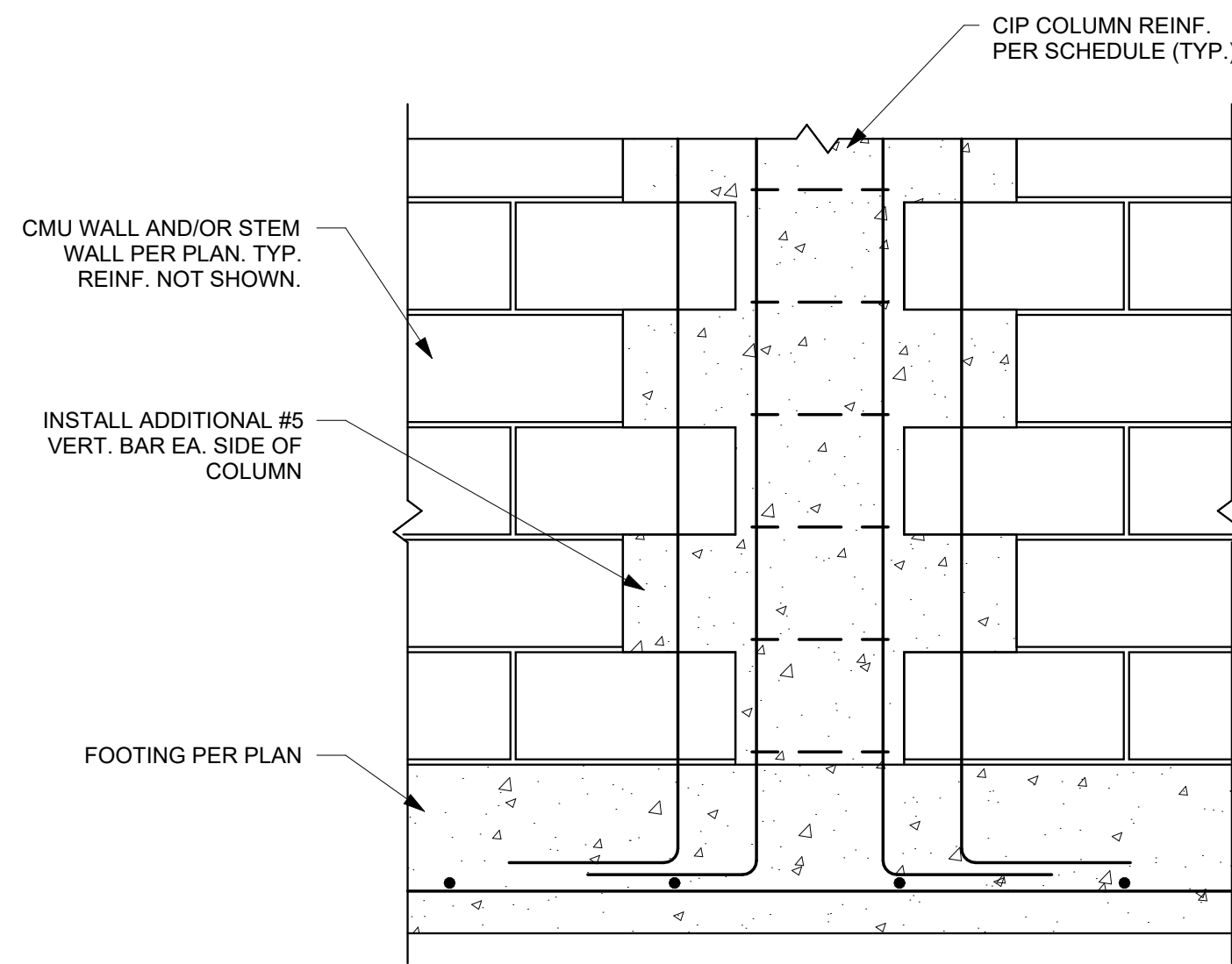
D TYP. HOLLOWCORE EXTERIOR BEARING (ROOF)
1" = 1'-0"



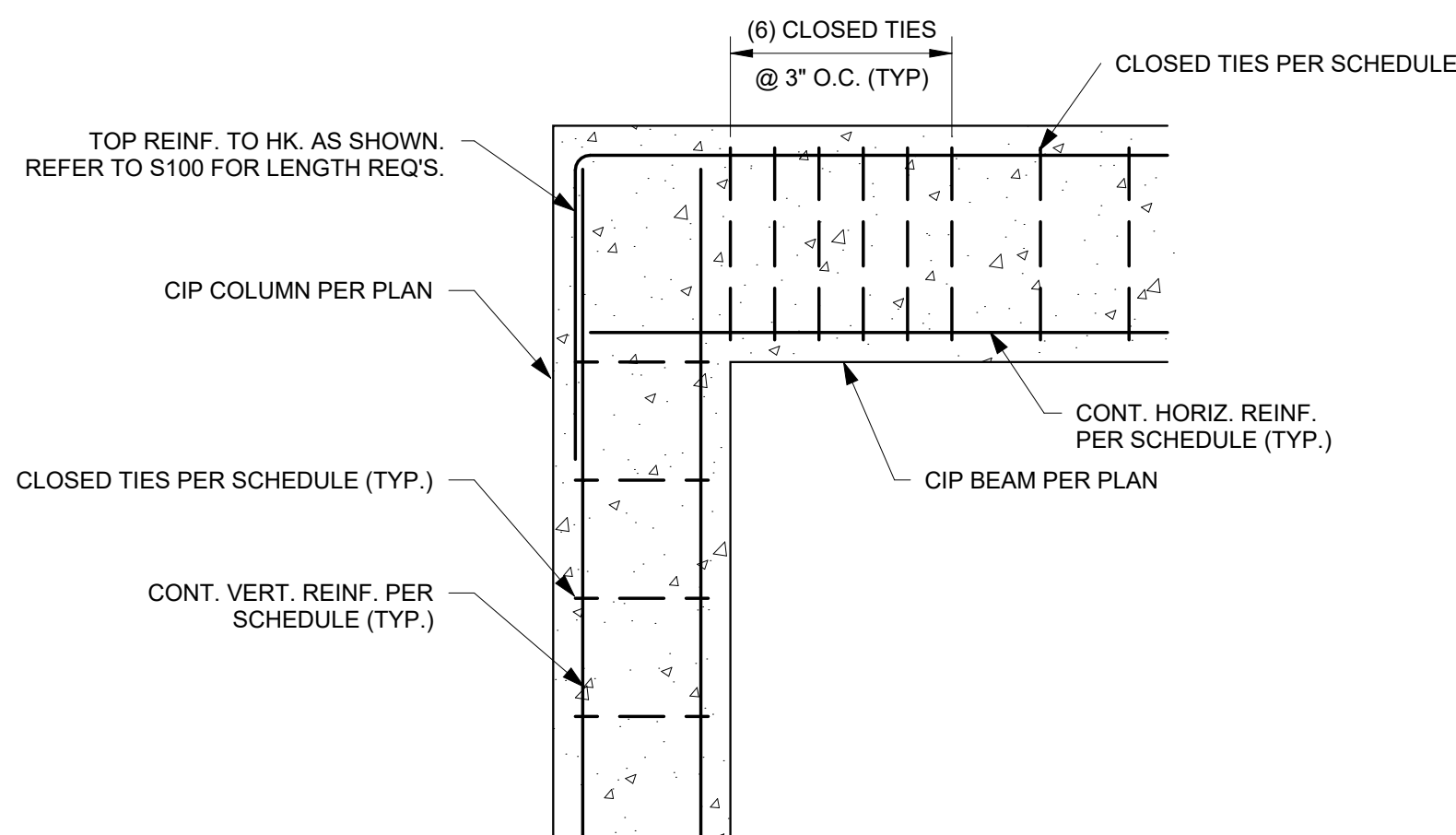
E TYP. ECCENTRICALLY LOADED STEEL COLUMN TO CIP BEAM DETAIL
1" = 1'-0"



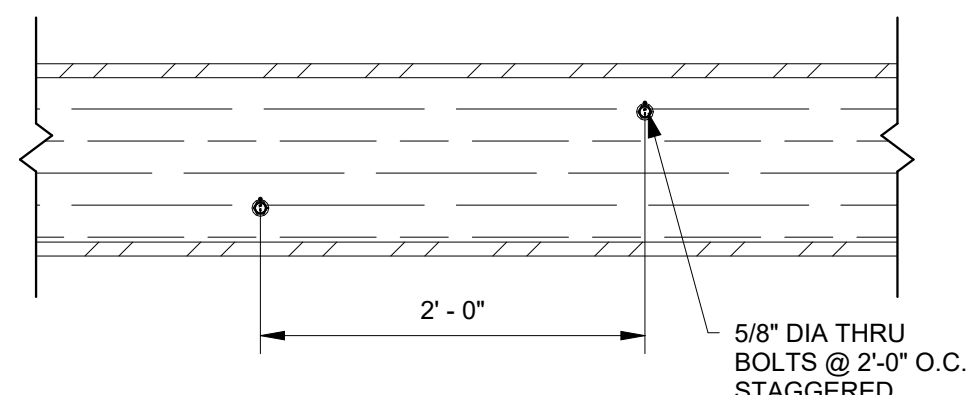
F TYP. IN-PLANE CIP COL. TO CMU WALL
1" = 1'-0"



J TYP. PARAPET TOP OUT DETAIL
1" = 1'-0"

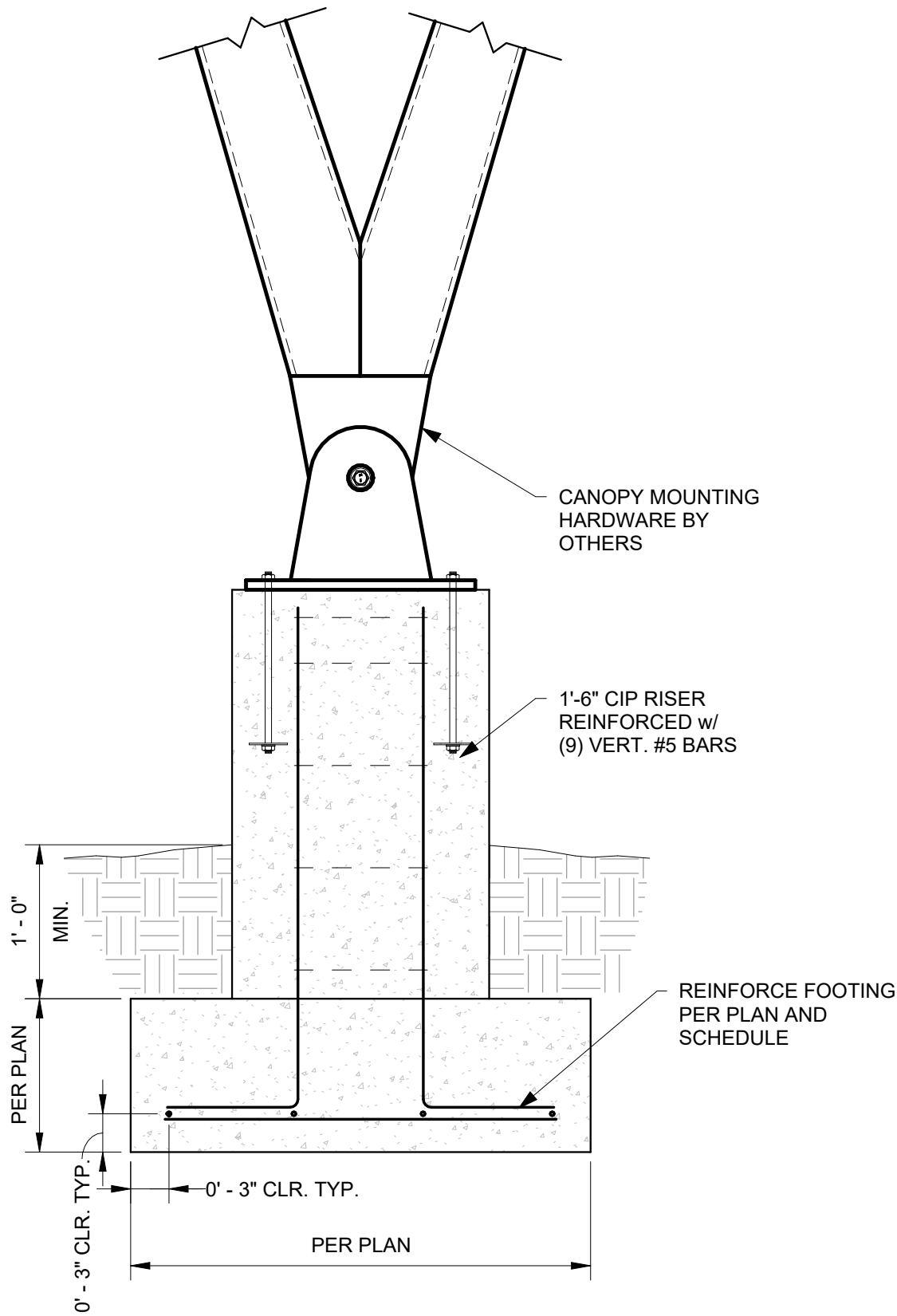


H TYP. STEEL BEAM WEB BLOCKING DETAIL
1" = 1'-0"

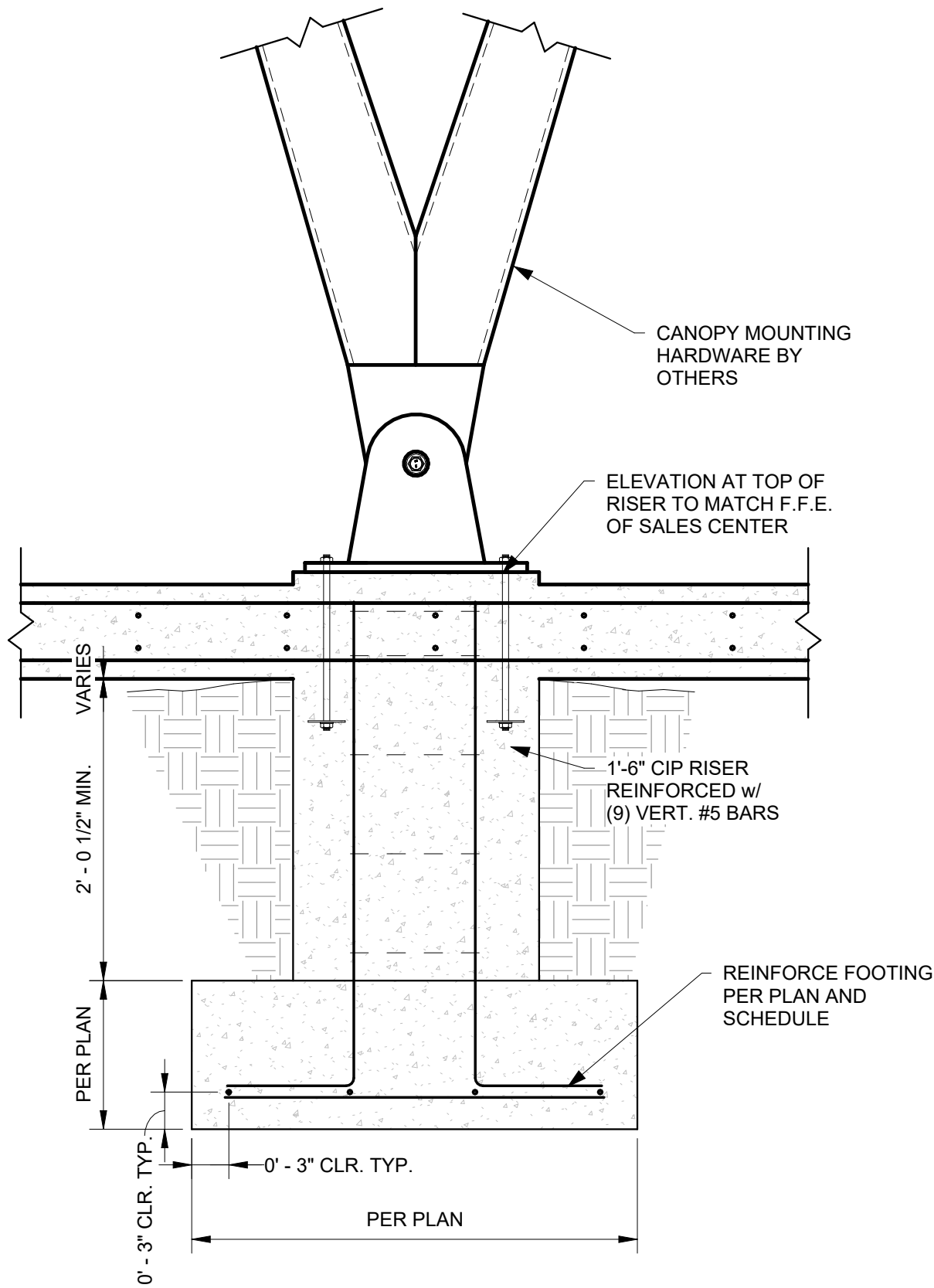
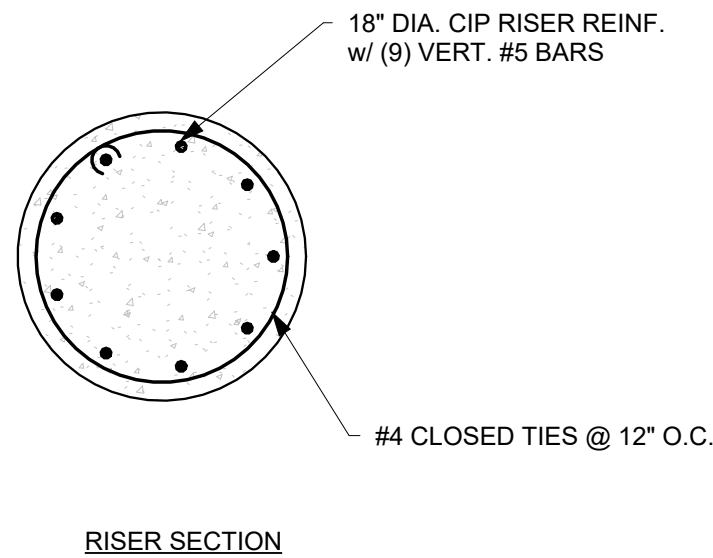


J TYP. PARAPET TOP OUT DETAIL
1" = 1'-0"

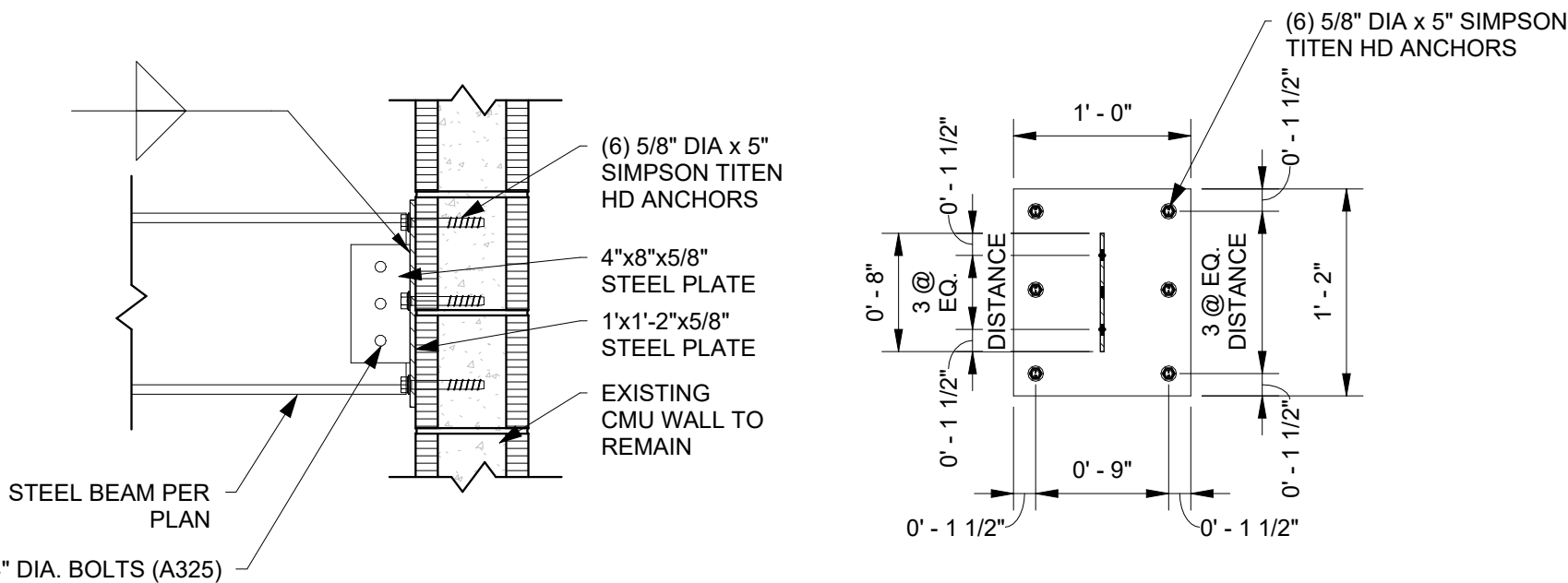
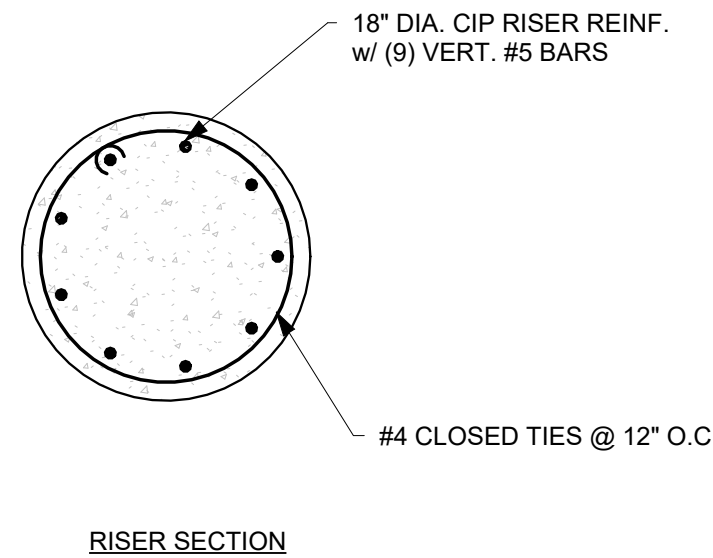
© 2024 DAG ARCHITECTS, INC. ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE COPIED, REPRODUCED OR TRANSMITTED IN ANY MANNER WITHOUT WRITTEN PERMISSION OF DAG ARCHITECTS, INC. THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY AND COPYRIGHT OF DAG ARCHITECTS, INC. AND SHALL NOT BE USED ON ANY OTHER PROJECT OR LOCATION EXCEPT AS DESCRIBED ON THE DRAWINGS WITHOUT THE WRITTEN AGREEMENT OF DAG ARCHITECTS.



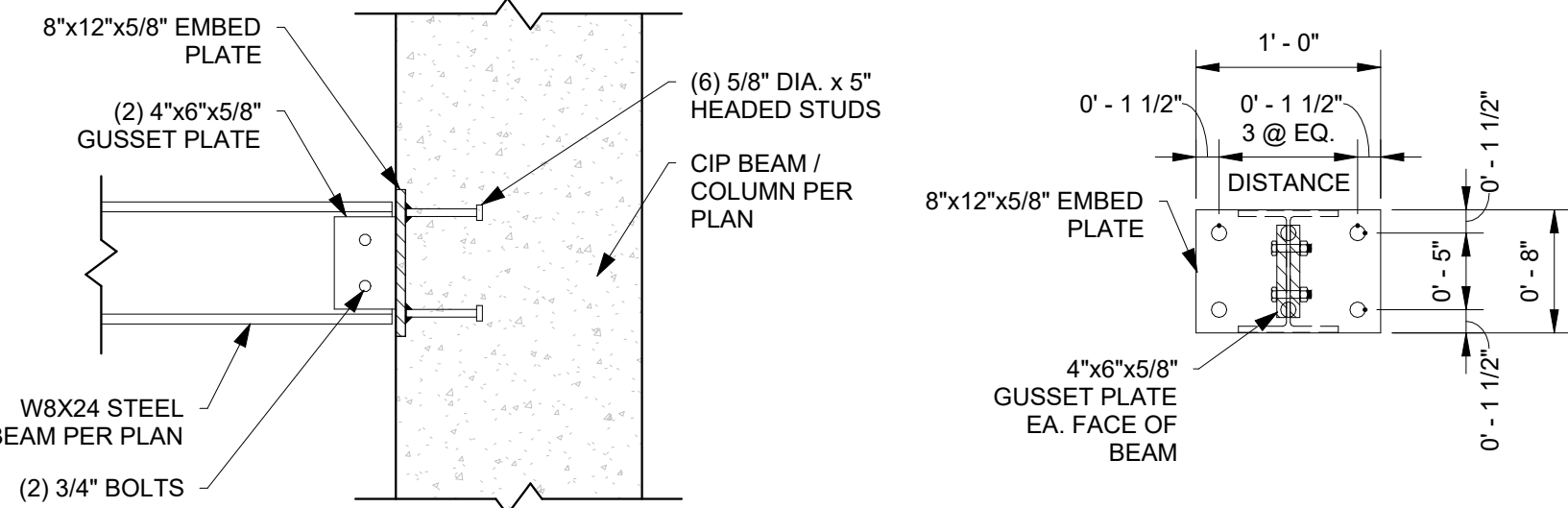
A TYP. 18" CIP PIER DETAIL
1" = 1'-0"



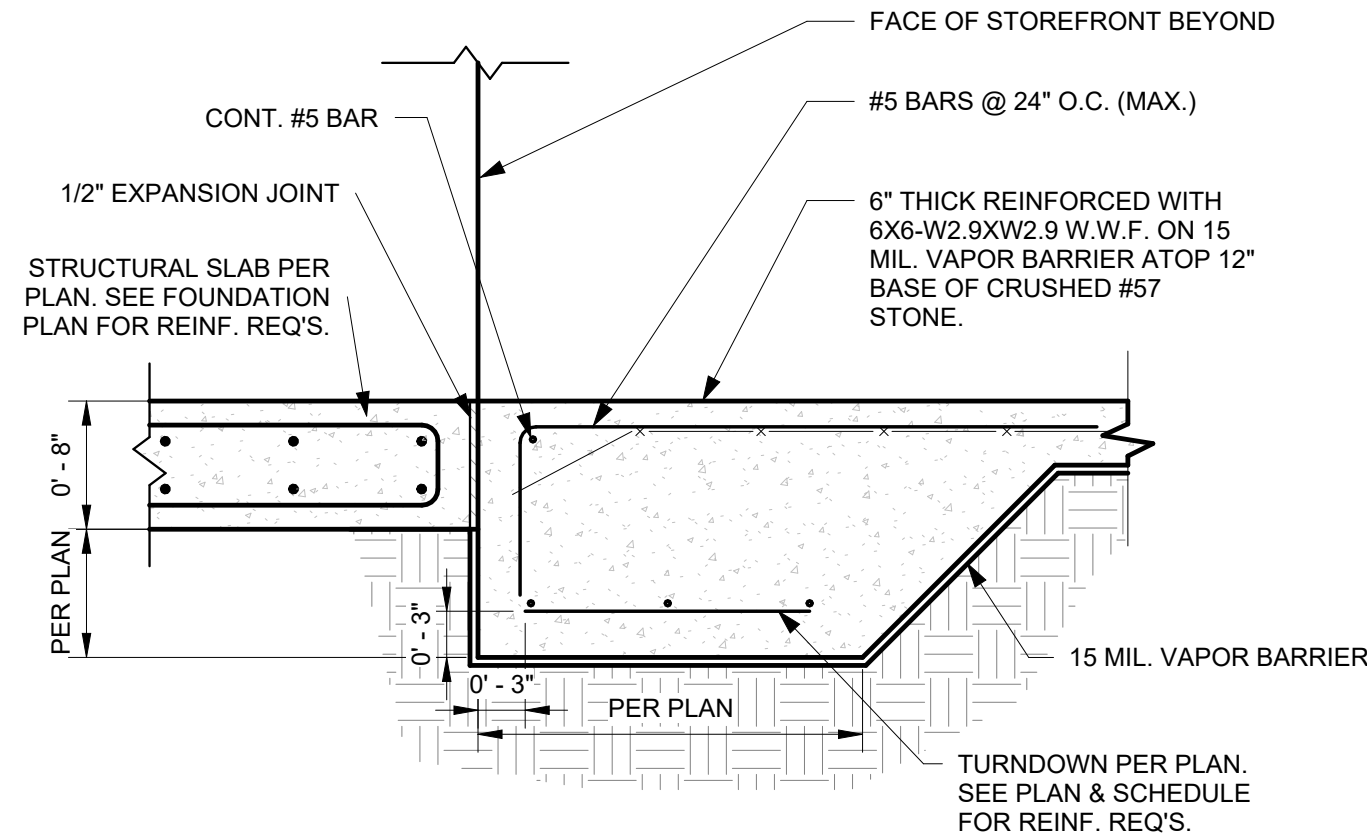
B TYP. 18" CIP PIER RECESSED IN SLAB DETAIL
1" = 1'-0"



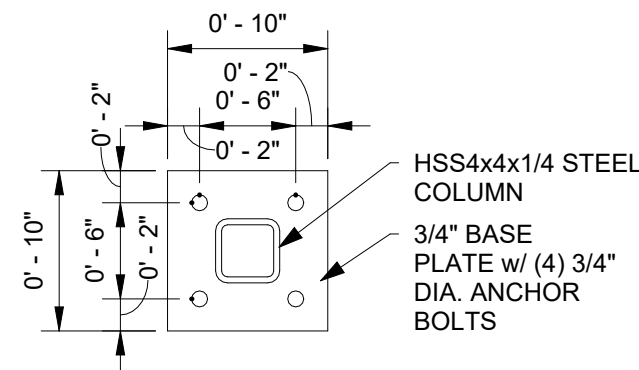
C TYP. STEEL BEAM TO WALL CONNECTION @ MEZZANINE DETAIL
1" = 1'-0"



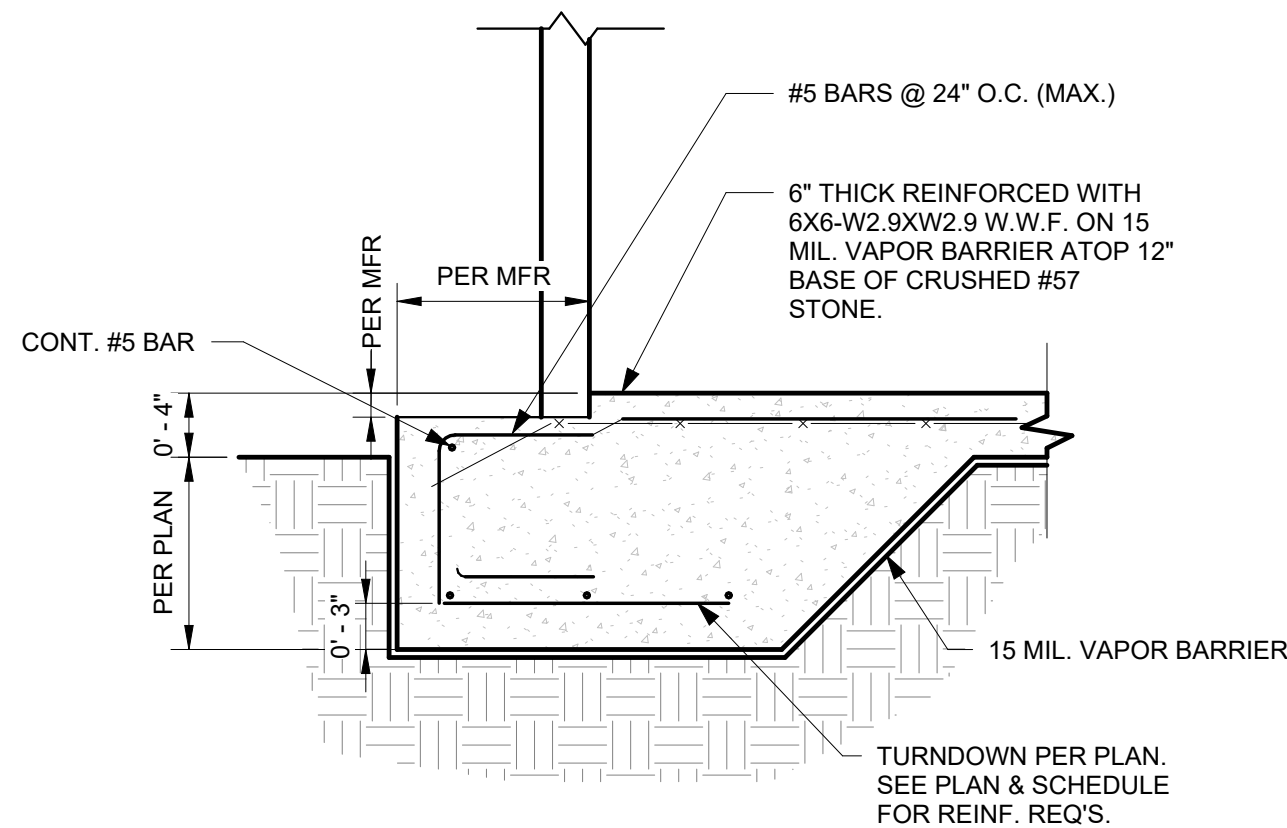
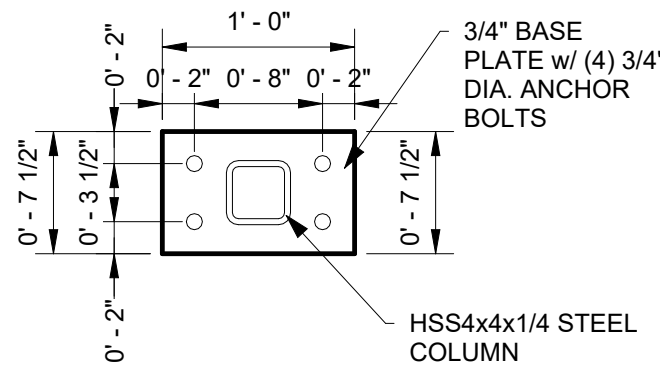
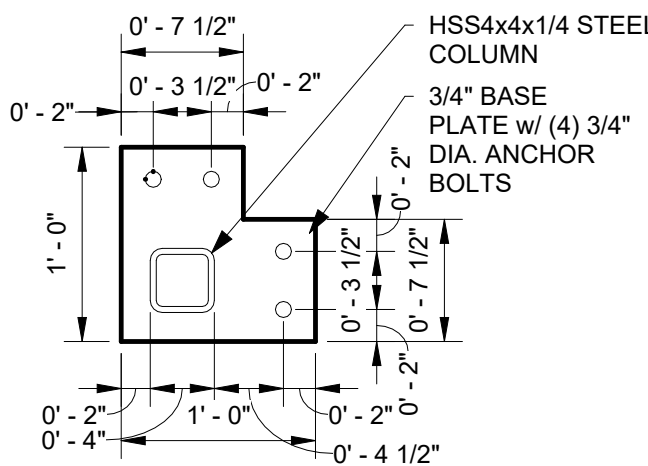
D TYP. STEEL BEAM TO CIP WALL CONNECTION DETAIL
1" = 1'-0"



E TYP. TURNDOWN @ STRUCTURAL SLAB DETAIL
1" = 1'-0"

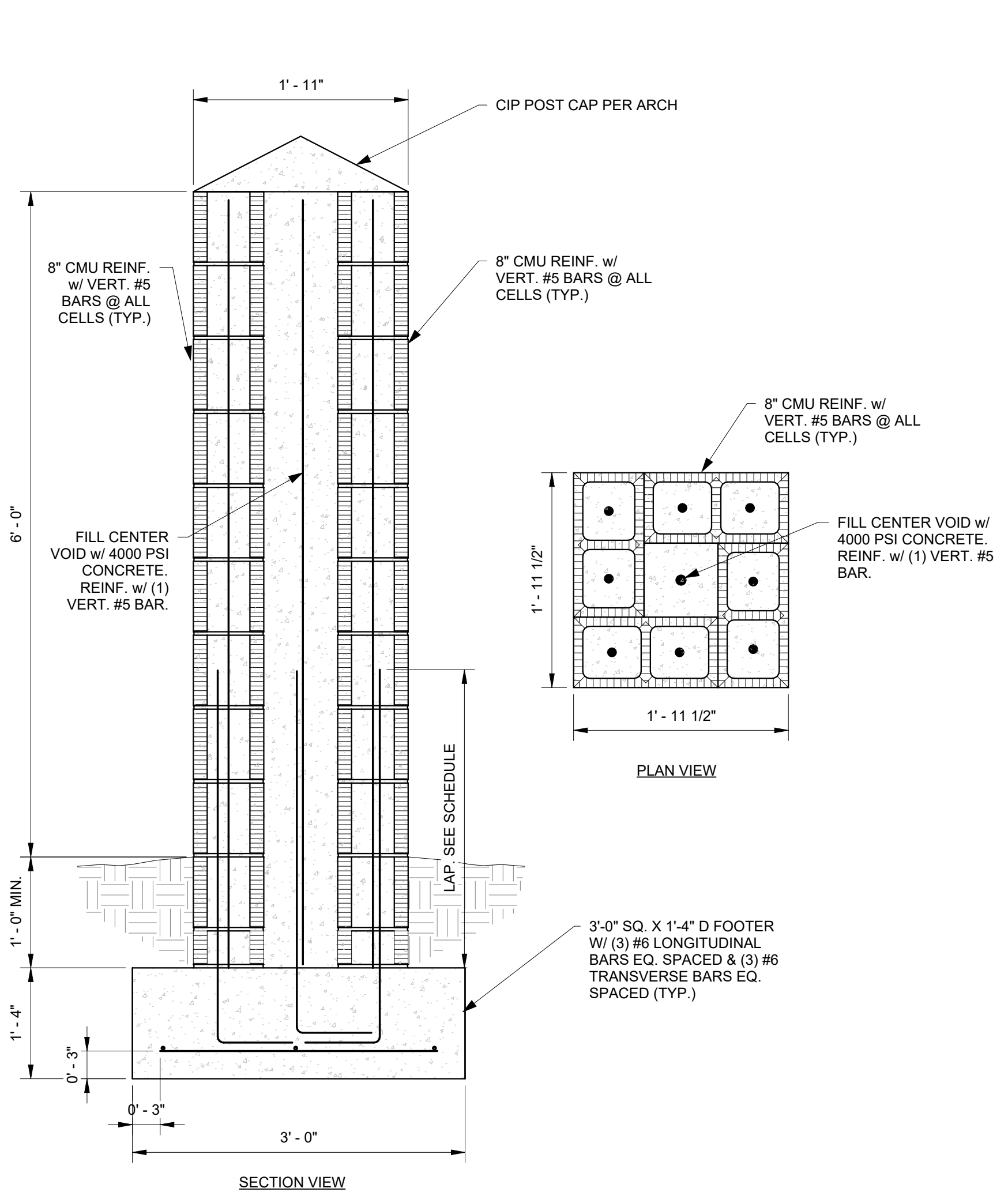


F TYP. BASE PLATE DETAIL
1" = 1'-0"

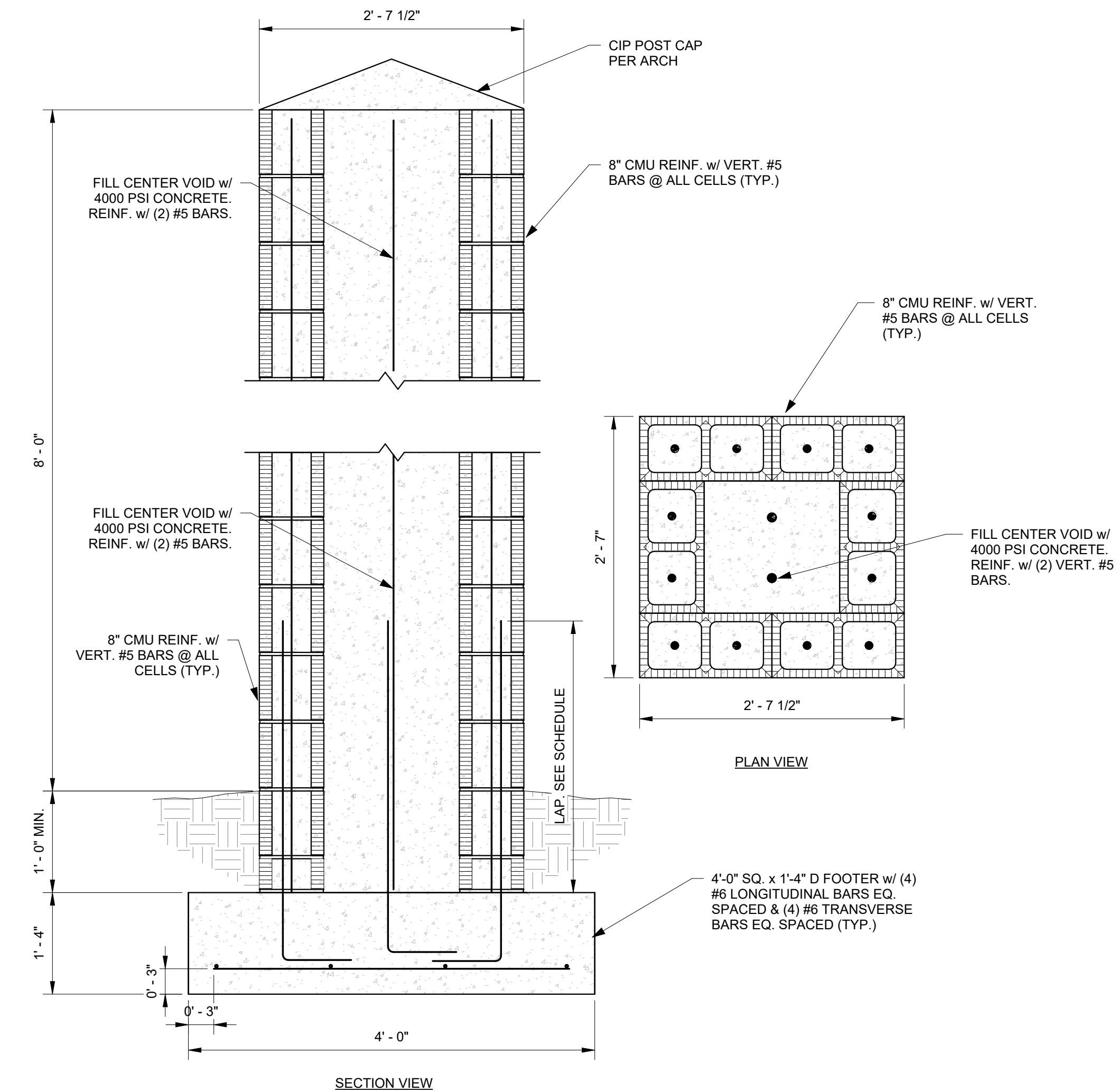


G TYP. TURNDOWN @ GARAGE DOOR
1" = 1'-0"

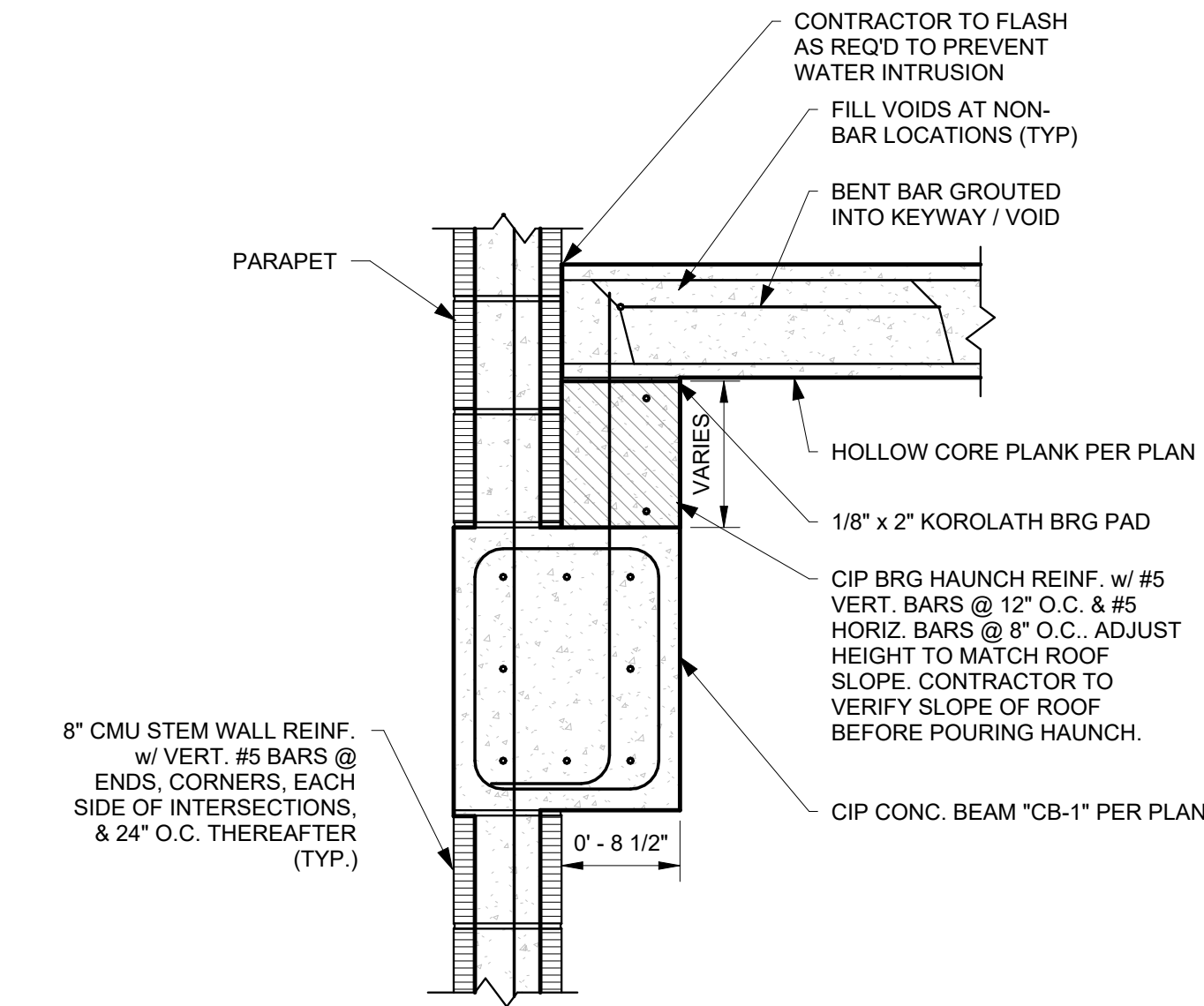
© 2023 DAG ARCHITECTS, INC. ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE COPIED, REPRODUCED OR TRANSMITTED BY ANY MEANS WITHOUT WRITTEN PERMISSION OF DAG ARCHITECTS, INC. THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY AND COPYRIGHT OF DAG ARCHITECTS, INC. AND SHALL NOT BE LOANED OR ANY OTHER PROJECT OR LOCATION EXCEPT AS DESCRIBED ON THE DRAWINGS. WITHOUT THE PRIOR WRITTEN AGREEMENT OF DAG ARCHITECTS.



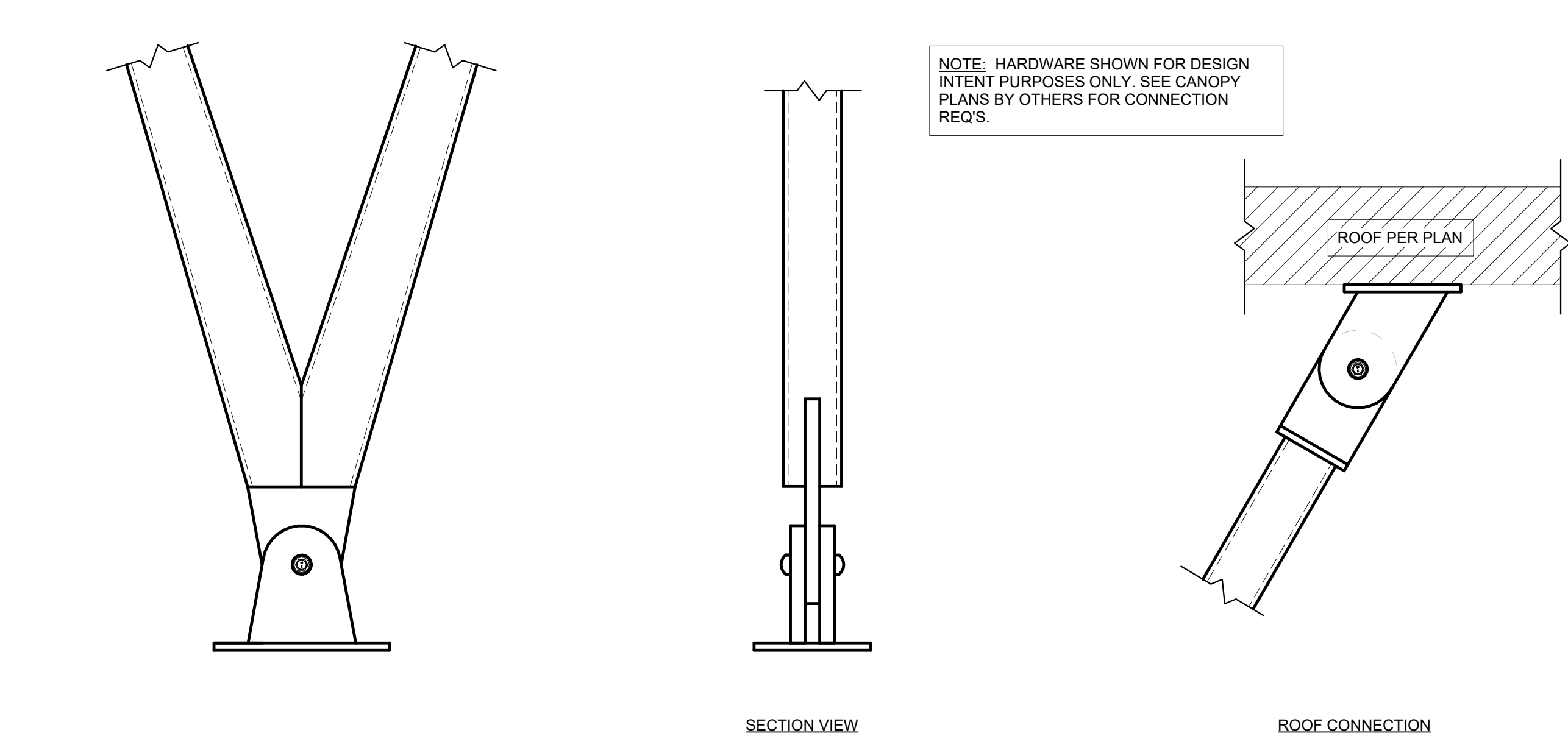
A TYP. 6FT CMU GATE SUPPORT POST REINF. DETAIL
1" = 1'-0"



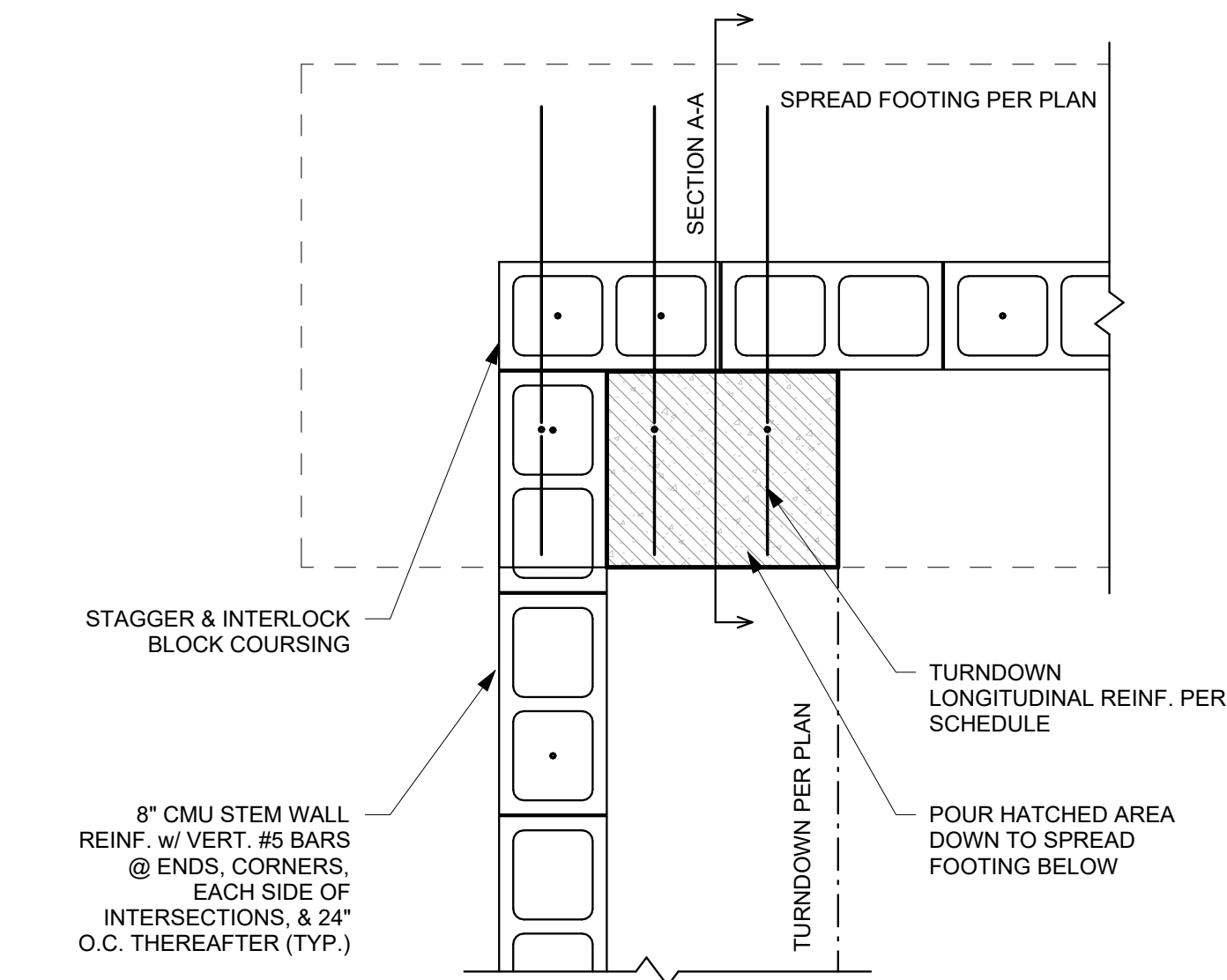
B TYP. 8FT CMU GATE SUPPORT POST REINF. DETAIL
1" = 1'-0"



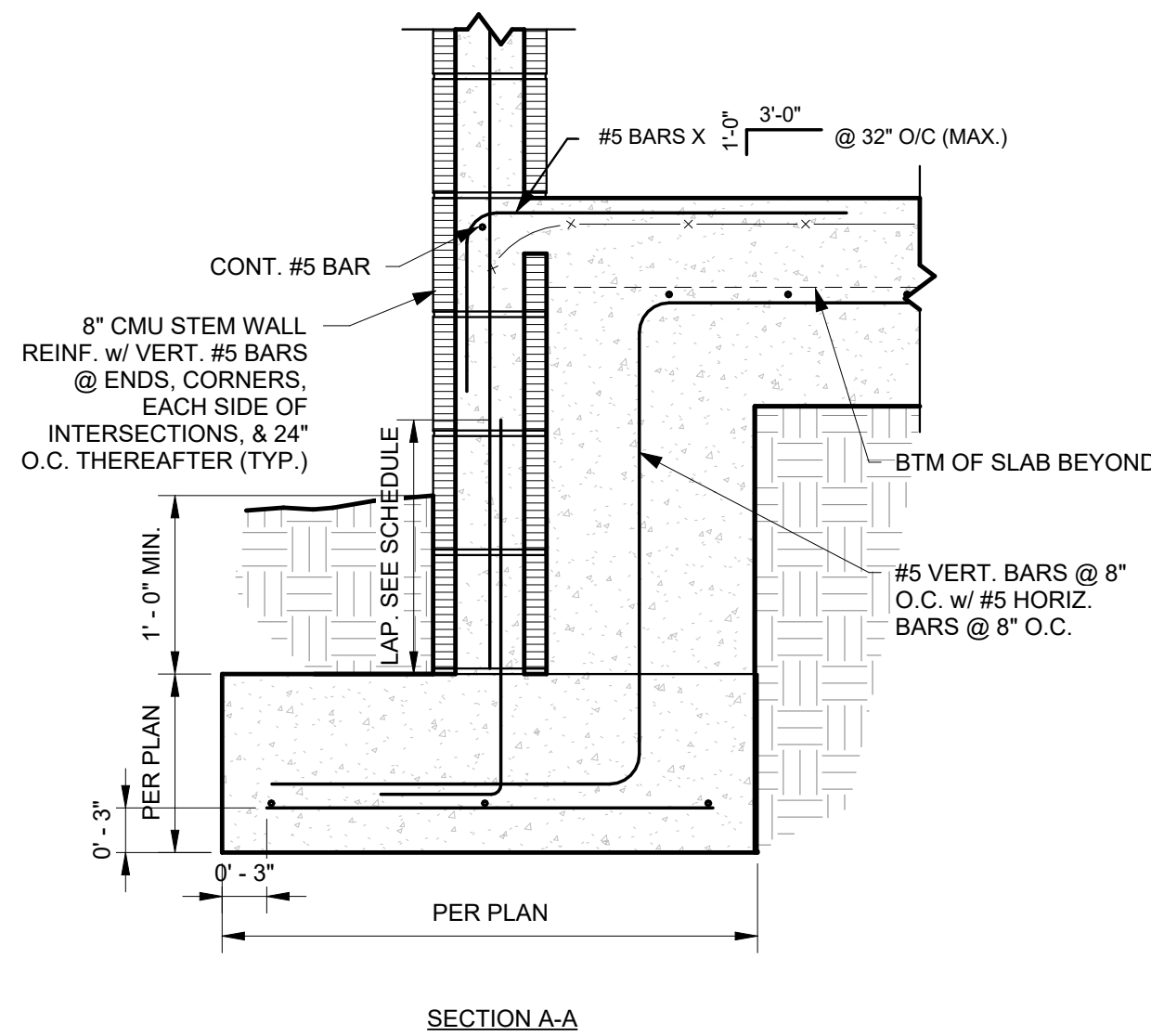
C TYP. CIP MASONRY BEARING
1" = 1'-0"



D TYP. KNIFE PLATE CONNECTION DETAIL
1" = 1'-0"



E TYP. SPREAD FOOTING TO TURNDOWN DETAIL
1" = 1'-0"





GARAGES LUXURY AT MIRAMAR - PHASE 1

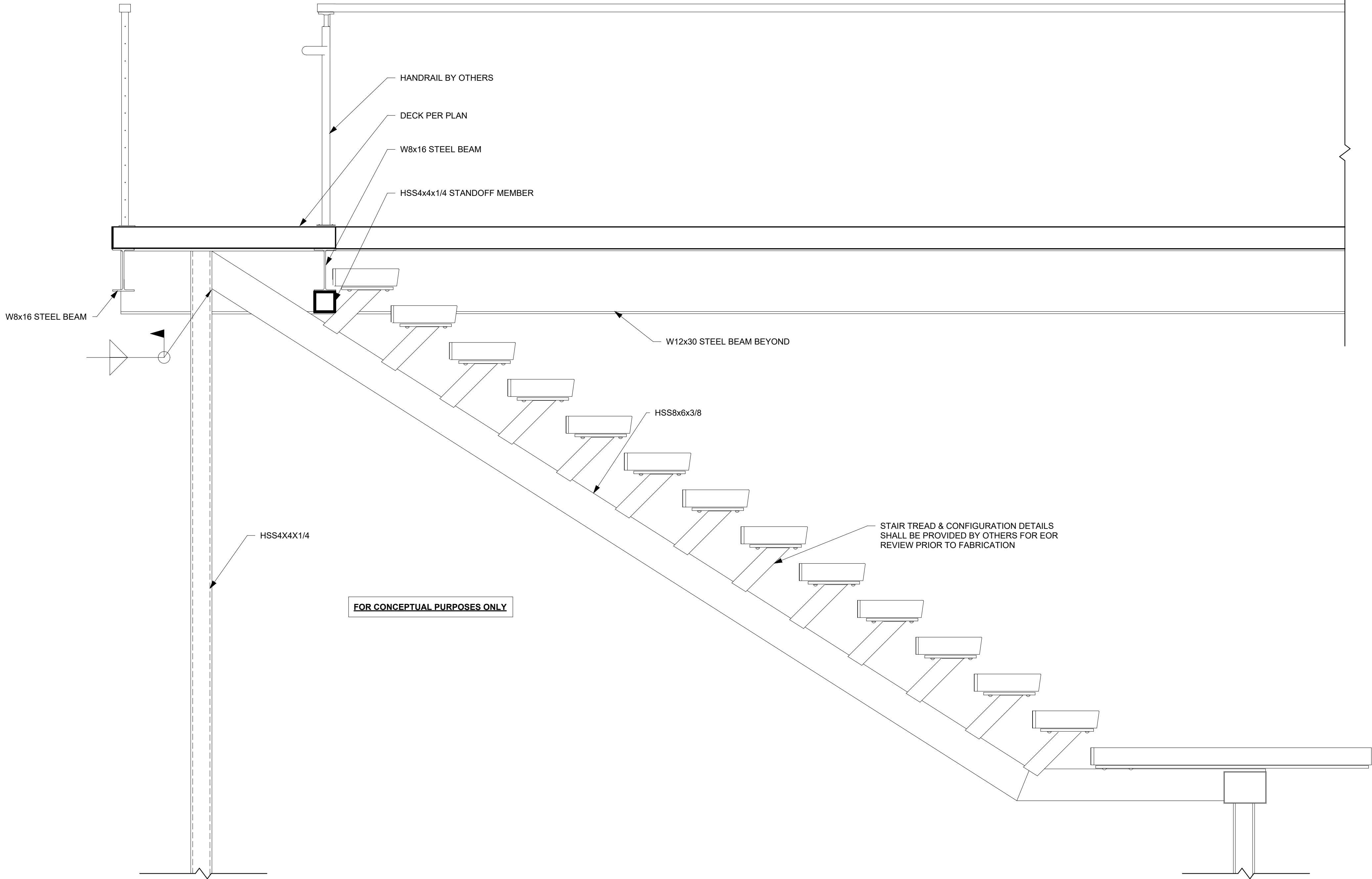
[illegible]

STRUCTURAL DETAILS

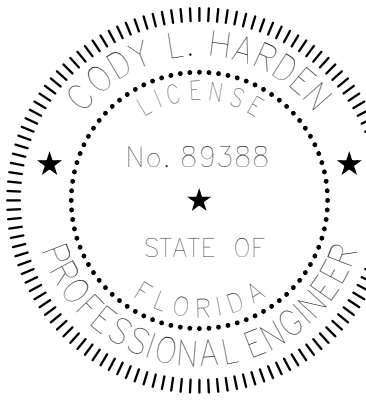
S506



© 2024 DAG ARCHITECTS, INC. ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE COPIED, REPRODUCED OR TRANSMITTED IN ANY MANNER WITHOUT WRITTEN PERMISSION OF DAG ARCHITECTS, INC. THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY AND COPYRIGHT OF DAG ARCHITECTS, INC. AND SHALL NOT BE USED ON ANY OTHER PROJECT OR LOCATION EXCEPT AS DESCRIBED ON THE DRAWINGS WITHOUT THE PRIOR WRITTEN AGREEMENT OF DAG ARCHITECTS.



A TYP. MEZZANINE STAIR CONNECTION
1" = 1'-0"



Digital Signature
FOR PERMIT

GROVES & KING, LLC

**GARAGES
LUXURY AT
MIRAMAR -
PHASE 1**

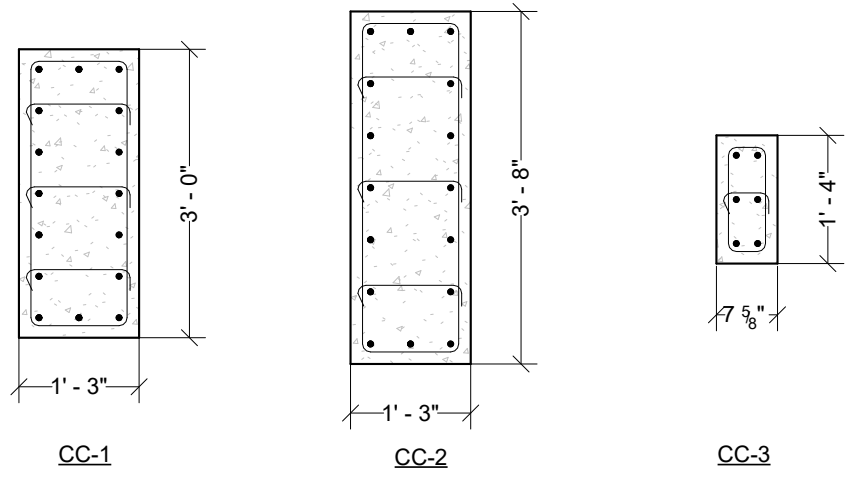
U.S. Highway 98
Miramar Beach, FL

REVISIONS		
NO.	DESCRIPTION	DATE

Project Number	25038
Dated	10.05.2026

© 2023 DAG ARCHITECTS, INC. ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE COPIED, REPRODUCED OR TRANSMITTED BY ANY MEANS WITHOUT WRITTEN PERMISSION OF DAG ARCHITECTS, INC. THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY AND COPYRIGHT OF DAG ARCHITECTS, INC. AND SHALL NOT BE USED ON ANY OTHER PROJECT OR LOCATION EXCEPT AS DESCRIBED ON THE DRAWINGS WITHOUT THE WRITTEN AGREEMENT OF DAG ARCHITECTS.

CONCRETE COLUMN SCHEDULE

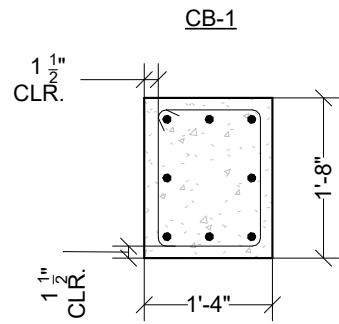


NOTE: NOT TO SCALE.

DESIGNATION	FLEXURAL REINF.	SHEAR REINF.
CC-1	(10) #5 BARS	#4 CLOSED TIES @ 12" O.C.
CC-2	(10) #6 BARS	#4 CLOSED TIES @ 12" O.C.
CC-3	(6) #5 BARS	#4 CLOSED TIES @ 12" O.C.

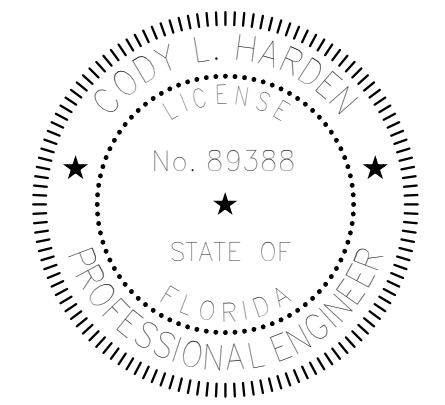
NOTES: 1) INSTALL #4 LATERAL TIES AS SHOWN. ALTERNATE ORIENTATION WHEN PLACING SUCCESSIVE SETS OF TIES. SPACING TO MATCH STIRRUPS.
2) MAINTAIN 1 1/2" CLEAR COVER FROM O.F. OF STEEL TO O.F. OF CONCRETE.

CONCRETE BEAM SCHEDULE



NOTE: NOT TO SCALE.

CONCRETE BEAM SCHEDULE				
DESIGNATION	TOP REINF.	MID REINF.	BTM REINF.	SHEAR REINF.
CB-1	(3) #6 BARS	(2) #5 BARS	(3) #6 BARS	#4 CLOSED TIES @ 4" O.C.



Digital Signature

FOR PERMIT

GROVES & KING, LLC

**GARAGES
LUXURY AT
MIRAMAR -
PHASE 1**

U.S. Highway 98
Miramar Beach, FL

REVISIONS		
NO.	DESCRIPTION	DATE

Project Number	25038
Dated	10.05.2026

STRUCTURAL
DETAILS

S601