

1. TO THE BEST OF OUR KNOWLEDGE, THE STRUCTURAL PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF THE FLORIDA BUILDING CODE, 2023, 8TH EDITION.
2. THE STRUCTURAL DOCUMENTS ARE TO BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DOCUMENTS. USE THESE NOTES IN CONJUNCTION WITH THE SPECIFICATIONS. IF A CONFLICT EXISTS, THE MORE STRINGENT GOVERNS.
3. COMPLY WITH REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE, AND ALL OTHER APPLICABLE FEDERAL STATE, AND LOCAL CODES, STANDARDS, REGULATIONS AND LAWS.
4. ALL REFERENCED STANDARDS REFER TO THE EDITION IN FORCE AT THE TIME THESE PLANS AND SPECIFICATIONS ARE ISSUED FOR BIDDING.
5. REVIEW ALL CONTRACT DOCUMENTS, DIMENSIONS AND SITE CONDITIONS AND COORDINATE WITH FIELD DIMENSIONS AND PROJECT SHOP DRAWINGS PRIOR TO CONSTRUCTION. NOTIFY ANY DISCREPANCIES IN WRITING TO THE ARCHITECT/ENGINEER. DO NOT CHANGE SIZE OR DIMENSIONS OF STRUCTURAL MEMBERS WITHOUT WRITTEN INSTRUCTIONS FROM THE STRUCTURAL ENGINEER OF RECORD.
6. ANY DISCREPANCIES, OMISSIONS, OR VARIATIONS NOTED ON THE DRAWINGS OR IN THE SPECIFICATIONS DISCOVERED DURING THE BIDDING PERIOD SHALL BE IMMEDIATELY COMMUNICATED IN WRITING TO THE ARCHITECT/ENGINEER.
7. PROTECT EXISTING FACILITIES, STRUCTURES AND UTILITY LINES FROM ALL DAMAGE. EACH CONTRACTOR SHALL PROTECT HIS WORK, ADJACENT PROPERTY AND THE PUBLIC. EACH CONTRACTOR IS SOLELY RESPONSIBLE FOR DAMAGE OR INJURY DUE TO HIS WORK OR NEGLIGENCE.
8. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR JOB SAFETY AND CONSTRUCTION PROCEDURES.
9. DO NOT SCALE DRAWINGS. USE DIMENSIONS.
10. ELECTRICAL AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF OPENINGS IN STRUCTURE NOT SHOWN ON STRUCTURAL DRAWINGS.
11. DETAILS LABELED "TYPICAL DETAILS" ON THE DRAWINGS APPLY TO ALL SITUATIONS THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. SUCH DETAILS APPLY WHETHER OR NOT THEY ARE KEYS IN AT EACH LOCATION. QUESTIONS REGARDING THE APPLICABILITY OF TYPICAL DETAILS SHALL BE RESOLVED BY THE ARCHITECT/ENGINEER.
12. REVISIONS ARE IDENTIFIED BY A REVISION NUMBER WITHIN A TRIANGLE. ALL REVISIONS ISSUED ON A SINGLE DATE WILL BE IDENTIFIED BY THE SAME REVISION NUMBER ISSUED CONSECUTIVELY.
13. IF REVISIONS ARE ENCIRCLED BY AN IRREGULAR "CLOUD", AS WELL AS FLAGGED WITH THE CURRENT REVISION NUMBER, CLOUDS ARE REMOVED FROM PREVIOUSLY ISSUED REVISIONS.
14. DESIGN LOADS AND CRITERIA:

FLOOR LIVE LOAD	40 PSF
PARTITION LOAD	15 PSF
ROOF LIVE LOAD	20 PSF

WIND CRITERIA	ASCE 7-22
ULTIMATE WIND SPEED	144 MPH EXPOSURE B
RISK CATEGORY	II
IMPORTANCE FACTOR	1.00
STRUCTURE TYPE	ENCLOSED

1. THE FOLLOWING REQUIREMENTS IN NO WAY REDUCE OR LIMIT ANY ADDITIONAL REQUIREMENTS OF SPECIFICATIONS.
2. REVIEW OF SUBMITTALS BY THE STRUCTURAL ENGINEER IS FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT AS PRESENTED BY THE CONTRACT DOCUMENTS. NO DETAILED CHECK OF QUANTITIES OR DIMENSIONS WILL BE MADE. ONLY THOSE SHOP DRAWINGS REQUIRED BY THE CONTRACT DOCUMENTS TO BE SUBMITTED WILL BE REVIEWED. ALL OTHERS WILL BE RETURNED WITHOUT COMMENT.
3. IN ACCORDANCE WITH THE SPECIFICATIONS, SUBMIT A COPY OF THE SHOP DRAWING SUBMITTAL REGISTER TO THE STRUCTURAL ENGINEER, SHOWING DATES OF SUBMITTAL FOR EACH SPECIFIC STRUCTURAL SECTION OF THE WORK, CONSISTENT WITH THE FOLLOWING CRITERIA:
 - A. ALLOW ADEQUATE TIME FOR TRANSIT AND PROCESSING BEFORE FABRICATION. THE STRUCTURAL ENGINEER WILL REVIEW AN AVERAGE SUBMITTAL WITHIN 10 WORKING DAYS OF RECEIPT BY THE ENGINEER.
 - B. SCHEDULE AND SUBMIT SHOP DRAWINGS FOR SPECIFIC COMPONENTS, SUCH AS COLUMNS/FOOTINGS, ETC., IN THEIR ENTIRETY. SHOP DRAWINGS FOR SIMILAR FLOORS SHALL BE SUBMITTED IN THE SAME PACKAGE.
4. THE SHOP DRAWINGS IN A TIMELY MANNER, CONSISTENT WITH THE ABOVE REQUIREMENTS.
5. ALL CHANGES AND ADDITIONS MADE ON RESUBMITTALS MUST BE CLEARLY FLAGGED AND NOTED. THE PURPOSE OF THE RESUBMITTALS MUST BE CLEARLY NOTED ON THE LETTER OF TRANSMITTAL. ARCHITECT/ENGINEER REVIEW WILL BE LIMITED TO THE ITEMS CAUSING THE RESUBMITTALS.
6. DO NOT REPRODUCE THE CONTRACT DOCUMENTS FOR USE AS SHOP DRAWINGS.
7. SHOP DRAWINGS NOT MEETING THE ABOVE CRITERIA OR SUBMITTED AFTER FABRICATION WILL NOT BE REVIEWED AND WILL BE RETURNED WITHOUT COMMENT. RESPONSIBILITIES OF DETAILER AND FABRICATOR:
 - A. GENERAL: SUBMIT SHOP DRAWINGS AND ANY OTHER SPECIAL INFORMATION NECESSARY FOR PROPER FABRICATION, ERECTION, AND PLACEMENT OF STRUCTURAL FABRICATIONS. INCLUDE PLANS, ELEVATIONS, AND SECTIONS. CLEARLY SHOW ANCHORAGES, CONNECTIONS, AND ACCESSORY ITEMS. THE DETAILER MUST INTERPRET THE CONTRACT DOCUMENTS AND CLEARLY CONVEY THIS INTERPRETATION TO THE FIELD IN THE FORM OF PLACING OR ERECTION DRAWINGS.
 - B. CONCRETE REINFORCING DETAILER - PROVIDE PLACING DRAWINGS FOR FABRICATION AND PLACING OF REINFORCING STEEL. THESE DRAWINGS SHALL INCLUDE, BUT ARE NOT LIMITED TO THE FOLLOWING: BAR LISTS, SCHEDULES, BENDING DETAILS, PLACING DETAILS, PLACING PLANS, AND PLACING ELEVATIONS.
8. CLEARLY SHOW ELEVATIONS OF ALL FOUNDATION WALLS. INDICATE CONTROL JOINTS, EXPANSION JOINTS, LINTELS, CONCRETE BOND BEAMS, AND OPENINGS, DETAILS OF ALL REINFORCING WITH LOCATIONS OF SPLICES, AND HOOKS, PILESTERS.
9. CLEARLY SHOW GRADE BEAM ELEVATIONS AND SECTIONS. INDICATE BAR LENGTHS, HOOKS, STIRRUP SPACING, LAP SPLICES, OFFSETS, AND LOCATION OF BARS WITH RESPECT TO ALL SUPPORTS.
10. CLEARLY SHOW COLUMN ELEVATIONS AND SECTIONS. INDICATE DOWELS, OFFSETS, LAP SPLICES, AND TIES. PLAN SECTIONS OF ALL COLUMNS MUST CLEARLY BE SHOWN.
11. CLEARLY SHOW FOUNDATION REINFORCING. INDICATE BAR LENGTHS, LOCATION AND SPLICES OF CONTINUOUS BARS, AND BAR SUPPORTS.
12. CLEARLY SHOW LOCATIONS OF ALL DOWELS ON PLAN.
13. INDICATE FOOTING STEP LOCATIONS AND PILE DETAIL.
14. FOR ADDITIONAL CRITERIA APPLICABLE TO SHOP DRAWINGS REQUIRING ENGINEERING INPUT BY A SPECIALTY ENGINEER, SEE BELOW

1. **PREPARE SUBGRADE AS PER THE RECOMMENDATION OUTLINED IN THE GEOTECHNICAL REPORT INCLUDED IN THE SPECIFICATIONS. GCGI Project Number GCGI26-015**
2. **REINFORCE SLABS ON GRADE IN ACCORDANCE WITH PLANS, SECTIONS AND DETAILS PROVIDED FOR THIS PROJECT.**
3. **USE 10 MIL. POLYETHYLENE SHEETING BETWEEN SOIL AND CONCRETE SLAB, U.O.N.**
4. **PLACE CRACK CONTROL JOINTS AT 12 FT. MAX. SO AS TO LIMIT CONCRETE PLACEMENT AREAS TO 144 SQ. FT. MAX. IN ALL FLOATING SLABS ON GRADE. DO NOT EXCEED A 2 TO 1 WIDTH TO LENGTH RATIO. CONTRACTOR SHALL SUBMIT A CONTROL JOINT LAYOUT FOR ENGINEER'S REVIEW PRIOR TO CONCRETE PLACEMENT.**
5. **SLAB FINISHES (UNLESS OTHERWISE DIRECTED BY ARCHITECTURAL PLANS & SPECIFICATIONS):**
 - EXTERIOR WALKING SURFACES - MEDIUM BROOM
 - DRIVING SURFACES - MEDIUM BROOM
 - INTERIOR SURFACES - STEEL TROWEL

1. USE STRUCTURAL CONCRETE AND CONCRETING PRACTICES CONFORMING TO ACI-318 AND 301 AND PROPORTION CONCRETE IN ACCORDANCE WITH ACI-318 CH. 4 AND MEETING A MIN. ULTIMATE COMPRESSIVE STRENGTH IN 28 DAYS AS FOLLOWS:

FOOTING	3000 PSI
GRADE BEAMS	3000 PSI

PROVIDE CURRENT (MAX 1 YEAR OLD) STATISTICAL DATA FOR EACH CONCRETE MIX DESIGN SUBMITTED.

2. WHERE CONCENTRATION OF REINFORCING STEEL HINDERS PROPER CONSOLIDATION OF CONCRETE USE CONCRETE CONTAINING A SUPERPLASTICIZER (N.R.W.F.) AND FULFILL ASTM C494 TYPE F. SLUMP AFTER ADDITION OF SUPERPLASTICIZER SHALL BE 7"±.

3. IF CONCRETE IS PUMPED, SLUMP MAY BE INCREASED TO 6" AT THE TRUCK. PROVIDED THE SLUMP SPECIFIED IN NOTE 2 IS MAINTAINED AT THE DISCHARGE END. USE A MINIMUM UNLESS PRE-APPROVED BY ARCHITECT. TAKE CONCRETE SAMPLES FOR SLUMP AT TRUCK AND AT DISCHARGE END. TAKE CONCRETE SAMPLES FOR CYLINDER TESTING AT DISCHARGE END.

4. USE ASTM A-615 GR. 60 FOR ALL REINFORCING STEEL. CONFORM TO ACI-301, ACI-318 AND ALL OTHERS OF STANDARD PRACTICE. ALL REINFORCING SHALL BE ACCURATELY PLACED, RIGIDLY SUPPORTED AND FIRMLY TIED IN PLACE WITH BARS AND SPACERS IN ACCORDANCE WITH THE ABOVE REQUIREMENTS. PROVIDE CLASS 'B' LAP SPLICE FOR CONTINUOUS BARS. U.O.N. LAP BOTTOM STEEL OVER L.O.N. LAP TOP STEEL. MIDSPAN AND OTHER SPECIFIC JOINTS. DISCONTINUOUS ENDS OF ALL TOP BARS AND ALL BARS IN WALLS U.O.N. USE 1" COVER OVER REINFORCING EXCEPT AS FOLLOWS:

1. ALL MASONRY WORK IS TO CONFORM TO ACI 530 AND 530.1.
2. CONSTRUCT REINFORCED AND UNREINFORCED MASONRY AS NOTED ON THE PLANS AND DETAILS IN ACCORDANCE WITH THE REQUIREMENTS OF SPECIFICATION - SECTION 0420 AND CONCRETE UNIT MASONRY AND 0430 "REINFORCED UNIT MASONRY".
3. USE COMBATE MASONRY UNITS CONFORMING TO ASTM C90. PROVIDE F8 AND C90 COMPLIANCE BY UNIT TEST METHOD. USE ONLY MASONRY UNITS THAT ARE A MIN. OF 50% OF SOLID.
4. USE TYPE "S" MORTAR IN ACCORDANCE WITH ASTM C270. USE 3/8" FULL-BEDDED JOINTS FOR ALL MASONRY UNITS. REMOVE MORTAR PROTRUDING INTO CELL CAVITIES AND REPAIR TO BE SET, GROUTED AND GROUTED. ALLOW A MIN. OF 24 HOURS FOR MORTAR TO CURE BEFORE PLACING GROUT.
5. USE ALL GROUT CONFORMING TO ASTM C-476 WITH A MIN. COMPRESSIVE STRENGTH OF 3000 PSI IN 28 DAYS, TESTED IN ACCORDANCE WITH ASTM C1019. AGGREGATE TO CONFORM TO ASTM C404 FOR COARSE GROUT AND SLUMP OF 8" TO 11". TEST SAMPLES FOR COMPRESSIVE STRENGTH. TEST EVERY 30 YARDS OR EACH DAY'S PRODUCTION.
6. FOR REINFORCED MASONRY USE STANDARD (9 GAGE CROSS AND SIDE RODS) LADDER TYPE HORIZONTAL JOINT REINFORCING IN EVERY OTHER COURSE U.O.N. USE PREFABRICATED CORNERS AND TEES AT WALL INTERSECTIONS. OVERLAP DISCONTINUOUS ENDS A MIN. OF 12". HORIZONTAL REINFORCING SHALL CONFORM TO ASTM A-82.
7. REINFORCED MASONRY NOT OTHERWISE NOTED USE GROUTED CELLS WITH #5 AT 32" O/C. PROVIDE TWO CELLS EACH SIDE OF WALL OPENINGS, WALL INTERSECTIONS, AND AT THE ENDS OF WALLS. GROUT SOLID ALL CELLS CONTAINING REINFORCING. USE BAR SPACERS IN EVERY 6TH COURSE AND PROVIDE CLEANOUT OPENINGS FOR EACH GROUTED CELL. SEE TYPICAL DETAILS.
8. USE #5 #8 OR #9 REINFORCING BARS WITH 2 LEGS EACH WAY IN ALL BOND BEAMS, CORNERS AND INTERSECTIONS. PROVIDE 1" MIN. EMBEDMENT. SEE U.O.N.
9. USE ASTM A-615 GRADE 60 REINFORCING STEEL.
10. IN HIGH-LIFT GROUTING USE A MAX. LIFT OF 4'-8" WITH MIN. HALF HOUR MAX. ONE HOUR BETWEEN LIFTS. VIBRATE EACH LIFT AND RECONSOLIDATE PREVIOUS LIFT AFTER PLACING NEXT LIFT.
11. REINFORCED MASONRY WALL CONSTRUCTION SHALL BE INSPECTED BY AN ENGINEER OR ARCHITECT IN ACCORDANCE WITH ACI 530-89/ASCE 5-88.
12. WHERE ANCHOR BOLTS ARE SET IN MASONRY WALL, FILL BLOCK CELLS WITH GROUT FOR BOLTED COURSE. ONE COURSE ABOVE (WHERE PRACTICABLE) AND TWO COURSES BELOW ANCHOR ELEVATIONS.
13. USE PRESSURE-TREATED WOOD FOR ALL WOOD IN CONTACT WITH MASONRY.
14. USE WOOD CELLS TO BE REMOVED AFTER ALL MASONRY OPENINGS ARE NOT FLUSH WITH STRUCTURAL FRAME. LINTELS OR HEADERS TO BEAR MIN. 8 INCHES EACH SIDE OF OPENING. SEE TYPICAL DETAILS.

1. ALL STEEL WORK (INCLUDING FABRICATION AND ERECTION) SHALL CONFORM TO THE AISC MANUAL OF STEEL CONSTRUCTION 9TH EDITION AND PROJECT SPECIFICATIONS. USE THE FOLLOWING:

2. A. ROLLED SHAPES, FLANGES AND BARS: ASTM A992 50 KSI

3. B. COLD-FORMED STEEL TUBING: ASTM A500, GRADE B.

4. C. HOT-FORMED STEEL TUBING: ASTM A501.

5. D. STEEL PIPE: ASTM A3, TYPE E OR S, GRADE B.

6. 2. USE STRUCTURAL STEEL THAT IS FULLY WELDABLE WITHIN GRADES AND FROM ANY GRADE TO ANY OTHER GRADE. ALL SHOP CONNECTIONS, U OR N.

7. 3. ALL SHOP AND FIELD WELDING SHALL CONFORM TO THE STRUCTURAL WELDING CODE AWS D1.1, LATEST EDITION, PUBLISHED BY THE AMERICAN WELDING SOCIETY (AWS). USE ELECTRODES CONFORMING TO AWS D1.1, E70 SERIES, U.O.N. SHOW ALL SHOP WELDS ON THE FABRICATION DRAWINGS AND ALL FIELD WELDS ON THE ERECTION DRAWINGS.

8. 4. ALL SHOP AND FIELD WELDERS, WELDING OPERATORS, AND TACKERS SHALL BE CERTIFIED ACCORDING TO AWS PROCEDURES FOR THE WELDING PROCESS AND WELDING POSITION USED. SUBMIT CERTIFICATES TO ARCHITECT/ENGINEER FOR THEIR RECORDS BEFORE BEGINNING WELDING.

9. 5. ALL FABRICATION AND ERECTION PROCEDURES TO BE USED SHALL BE PREPARED BY THE FABRICATOR OR CONTRACTOR AS WRITTEN PROCEDURE SPECIFICATIONS AND SUBMITTED TO THE ARCHITECT/ENGINEER FOR THEIR RECORD. ALL JOINT WELDING PROCEDURES SHALL BE QUALIFIED PRIOR TO USE ACCORDING TO AWS PROCEDURES. SUBMIT PROPERLY DOCUMENTED EVIDENCE OF QUALIFICATION TESTS TO ARCHITECT/ENGINEER FOR THEIR RECORDS.

10. A. 325 BOLTS.

11. B. A325 BOLTS SHALL CONFORM TO ASTM A 325 TYPE 1, HIGH STRENGTH BOLTS FOR STRUCTURAL STEEL JOINTS. DO NOT USE TYPE 2 BOLTS.

12. C. PROVIDE HARDENED WASHERS CONFORMING TO ASTM F 436. PLACE HARDENED WASHERS UNDER PARTS OF ALL BOLTS.

13. D. ALL BOLTS SHALL BE NEW AND DOMESTICALLY MANUFACTURED. DO NOT REUSE BOLTS. BOLTS AND NUTS SHALL BE WAX DIPPED BY THE BOLT SUPPLIER OR LUBRICATED WITH JOHNSON'S STICK WAX 140.

14. E. BEARING-TYPE BOLTS (A-325N) SHALL BE TIGHTENED TO THE SNUG TIGHT CONDITIONS. THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTNESS THAT EXISTS WHEN ALL PLAIN IN CONTACT ARE IN FIRM CONTACT. THIS MAY BE ATTAINED BY A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH.

15. F. SLIP CRITICAL TYPE BOLTS (A325SC) SHALL BE TIGHTENED TO THE MINIMUM FASTENER TENSION INDICATED IN SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325, ASTM A490 BOLTS, SECTION 5.5, TABLE 3. TENSION SHALL BE DETERMINED SOLELY BY USE OF DIRECT TENSION INDICATORS, COMPLYING WITH ASTM F959-85, OR "LOHR" TENSION CONTROL BOLTS. HARDENED WASHERS SHALL BE USED FOR FRICTION-TYPE BOLTS. PROVIDE ONLY DOMESTICALLY MANUFACTURED BOLTS AND D.T.I.'S FRICTION-TYPE BOLTED CONNECTIONS SHALL BE INSTALLED.

16. G. CONNECTIONS NOT COMPLETELY DETAILED ON STRUCTURAL DRAWINGS SHALL BE DESIGNED AND DETAILED BY FABRICATOR'S SPECIALTY ENGINEER ACCORDING TO AISC SPECIFICATIONS AND THE SPECIFIED LOADS AND IN COMPLIANCE WITH ALL APPLICABLE PARAGRAPHS OF THIS SECTION.

17. H. USE A507 BOLTS FOR ERECTION BOLTS AND ANCHOR BOLTS OR WHEN SPECIFICALLY CALLED FOR ON THE DRAWINGS.

18. I. CUT, DRILL, OR PUNCH HOLES PERPENDICULAR TO METAL SURFACES. DO NOT FLAME CUT HOLES OR ENLARGE HOLES BY BURNING.

19. J. SPLICING OF STRUCTURAL STEEL MEMBERS IN THE FIELD OR IN THE SHOP IS PROHIBITED. ALL SPICES INCLUDING DRILLING DRAWINGS.

20. K. ALL EXTERIOR STRUCTURAL PLATES ANGLES AND EXTERIOR BEAMS SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A-153.

21. L. REFER TO ARCHITECTURAL PLANS FOR FIREPROOFING OF STRUCTURAL STEEL MEMBERS. REFER TO SPECIFICATIONS FOR PAINTING OF EXPOSED STRUCTURAL STEEL.

22. M. PROVIDE TEMPORARY BRACING AS NECESSARY TO INSURE A STABLE STRUCTURE DURING CONSTRUCTION.

23. N. DO NOT REPRODUCE THE STRUCTURAL DRAWINGS FOR USE AS SHOP DRAWINGS.

24. O. NO CUTTING OF SECTIONS, FLANGES, WEBS, OR ANGLES SHALL BE ALLOWED WITHOUT WRITTEN APPROVAL FROM THE ENGINEER OF RECORD.

Base pressure (qh) =	42.5 psf	Kz = Kh =	0.700	Edge Strip (a) =	8.0 ft
(Kd qh) =	36.1 psf			End Zone (2a) =	16.0 ft
GCp =	+/-0.18			Zone 2 length =	46.0 ft

Wind Pressure Coefficients						
CASE A				CASE B		
Surface	GCpfl	$\theta = 12 \text{ deg}$ w/ GCpfi	w+GCpfi	GCpfl	w-GCpfi	w+GCpfi
1	0.40	0.58	0.22	-0.45	-0.27	-0.63
2	-0.69	-0.51	-0.87	-0.69	-0.51	-0.87
3	-0.37	-0.19	-0.55	-0.37	-0.19	-0.55
4	-0.29	-0.11	-0.47	-0.45	-0.27	-0.63
5				0.40	0.58	0.22
6				-0.29	-0.11	-0.47
1E	0.61	0.79	0.43	-0.48	-0.30	-0.66
2E	-1.07	-0.89	-1.25	-1.07	-0.89	-1.25
3E	-0.53	-0.35	-0.71	-0.53	-0.35	-0.71
4E	-0.43	-0.25	-0.61	-0.48	-0.30	-0.66
5E				0.61	0.79	0.43
6E				-0.43	-0.25	-0.61

1	21.0	7.9	-9.8	-22.8
2	-19.4	-31.4	-16.4	-31.4
3	-6.9	-19.9	-6.9	-19.9
4	-4.0	-17.0	-9.8	-22.8
5			21.0	7.9
6			-4.0	-17.0
1E	28.5	15.5	-10.8	-23.8
2E	-32.2	-45.2	-32.2	-45.2
3E	-12.6	-25.6	-12.6	-25.6
4E	-9.0	-22.0	-10.8	-23.8
5E			28.5	15.5
6E			-9.0	-22.0

Windward parapet =	0.0 psf (GCpn = +1.5)
Leeward parapet =	0.0 psf (GCpn = -1.0)

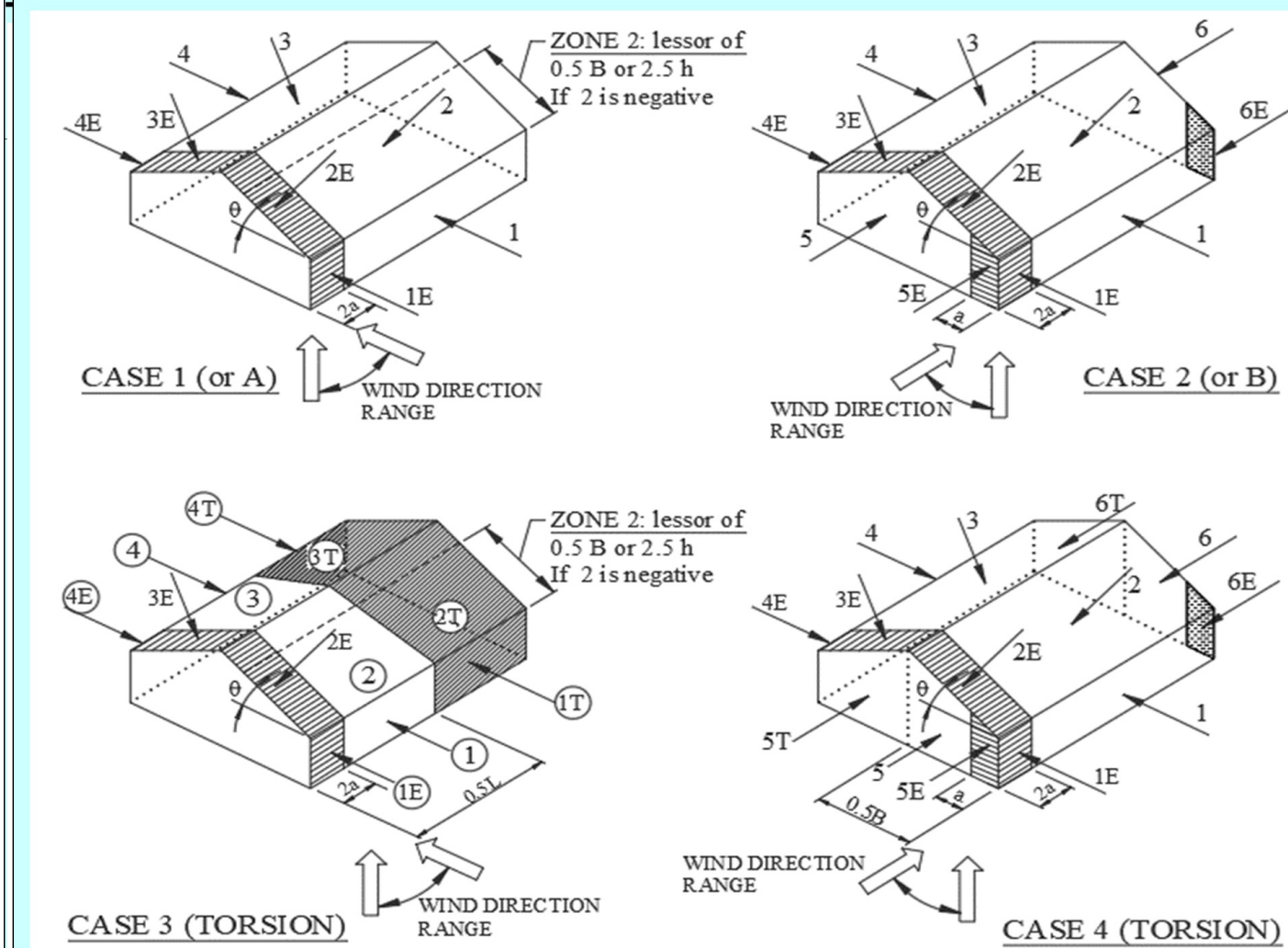
Interior Zone: Wall 24.9 nsf

Roof	-19.5 psf
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Interior Zone: Wall	24.9 psf
End Zone: Wall	37.6 psf

**** NOTE:** Total horiz force shall not be less than that determined by neglecting roof forces (except for MWFRS moment frames).

The code requires the MWFRS be designed for a min ultimate force of 16 psf multiplied by the wall area plus an 8 psf force applied to the vertical projection of the roof.



NOTE: Torsional loads are 25% of zones 1 - 6.
Exception: One story buildings $h < 30'$ and 1 to 2 story buildings framed with light-frame construction or with flexible diaphragms need not be designed for the torsional load case.

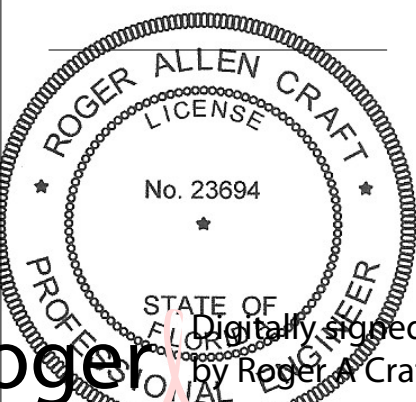
Wind Loads - Components & Cladding : $h \leq 60'$										Ultimate Wind Pressures			
Base pressure (qh) =		0.0	Kh =		0.619								
(Kd qh) =		31.9 psf			20.0	0.2h = 4.0 ft							
Minimum passep (ht) =		0.0	0.0h =		12.0 ft								
Roof Angle (θ) =		0.0	GCpI =		+/-0.18								
Type of roof = Monoslope			Kd qI =		31.9 psf								
Roof										User input			
		GCp +/- GCpi				Surface Pressure (psf)				100	100		
Area	10 sf	100 sf	500 sf	1000 sf	10 sf	100 sf	500 sf	1000 sf					
Negative Zone 1	-1.88	-1.47	-1.16	-1.18	-60.0	-46.9	-37.7	-37.7	-46.9	-46.9	-46.9		
Negative Zone 1	-1.08	-1.08	-0.73	-0.58	-34.5	-34.5	-23.3	-18.5	-34.5	-34.5	-34.5		
Negative Zone 2	-2.48	-1.95	-1.58	-1.58	-79.2	-62.3	-50.4	-50.4	-62.3	-62.3	-62.3		
Negative Zone 3	-3.38	-2.32	-1.58	-1.58	-107.9	-74.1	-50.4	-50.4	-74.1	-74.1	-74.1		
Positive All Zones	0.48	0.38	0.38	0.38	16.0	16.0	16.0	16.0	16.0	16.0	16.0		
Overhang Zone 1&1	-1.70	-1.60	-1.00	-1.00	-64.3	-51.1	-31.9	-31.9	-51.1	-51.1	-51.1		
Overhang Zone 2	-2.30	-1.59	-1.10	-1.10	-73.4	-50.9	-35.1	-35.1	-50.9	-50.9	-50.9		
Overhang Zone 3	-3.20	-1.96	-1.10	-1.10	-102.2	-62.7	-35.1	-35.1	-62.7	-62.7	-62.7		
Walls										User input			
wall a = 6.0 ft		GCp +/- GCPI				Surface Pressure at h				100	100		
Area	10 sf	100 sf	200 sf	500 sf	10 sf	100 sf	200 sf	500 sf					
Negative Zone 4	-1.17	-1.01	-0.96	-0.90	-39.3	-32.3	-30.7	-28.7	-32.3	-30.7	-30.7		
Negative Zone 5	-1.44	-1.12	-1.03	-0.90	-46.0	-35.6	-32.8	-26.7	-35.6	-32.8	-26.7		
Positive Zone 4 & 5	1.08	0.92	0.87	0.81	34.5	29.4	27.9	25.9	29.4	27.9	25.9		

WALL

**Walls $h \leq 60^\circ$
& alt design $h=90^\circ$**

**Monoslope roofs
 $3^\circ \leq \theta \leq 10^\circ$
 $h \leq 60^\circ$ & alt design $h=90^\circ$**

This item has been electronically signed and sealed by Roger A. Craft using digital signature and data. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies



Roger A Craft
Digitally signed by Roger A Craft
Date: 2026.09.02 11:34:01 -05'00'

Date:
Drawn By:
Project Number:

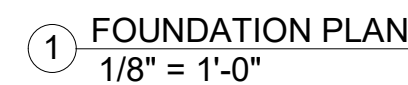


RAC ENGINEERING, INC.
129 HIGHPOINT DRIVE
GULF BREEZE, FLORIDA 32561
850-712-8290

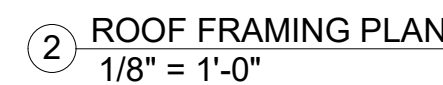
GENERAL
NOTES

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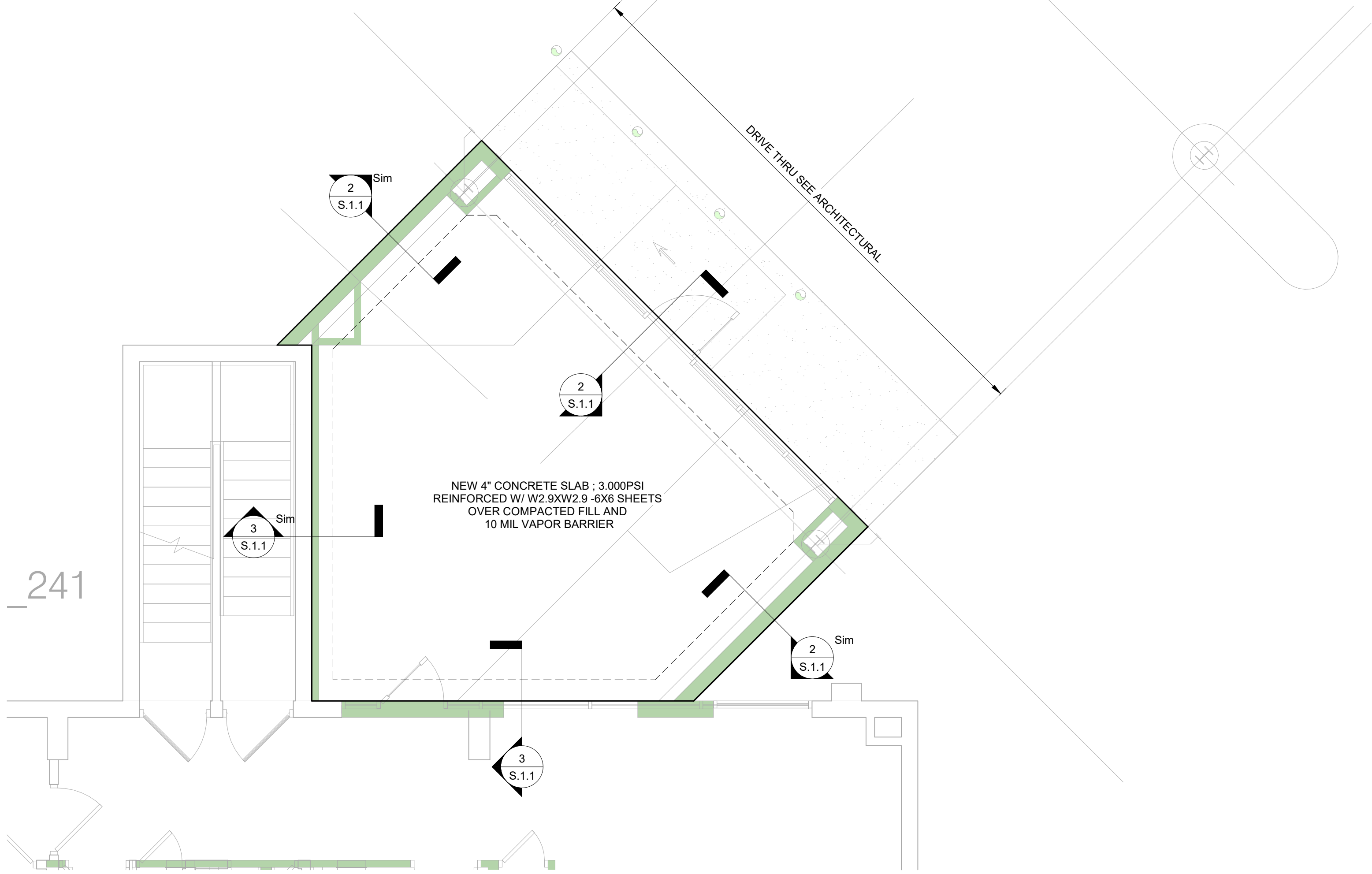
FOOTING ID	SIZE	REINFG	NOTES
F44	4' X 4' X 1'-4" DEEP	#5@10"OC E.W. T.&B	
F66	6' X 6' X 1'-4" DEEP	#5@10"OC E.W. T.&B	
GB24X16	2' X 1'-4" DEEP CONT.	(3)#5 CONT T&B ; #4 TIES @ 12"OC	
GB36X16	3' X 1'-4" DEEP CONT.	(4)#5 CONT T&B ; #4 TIES @ 12"OC	



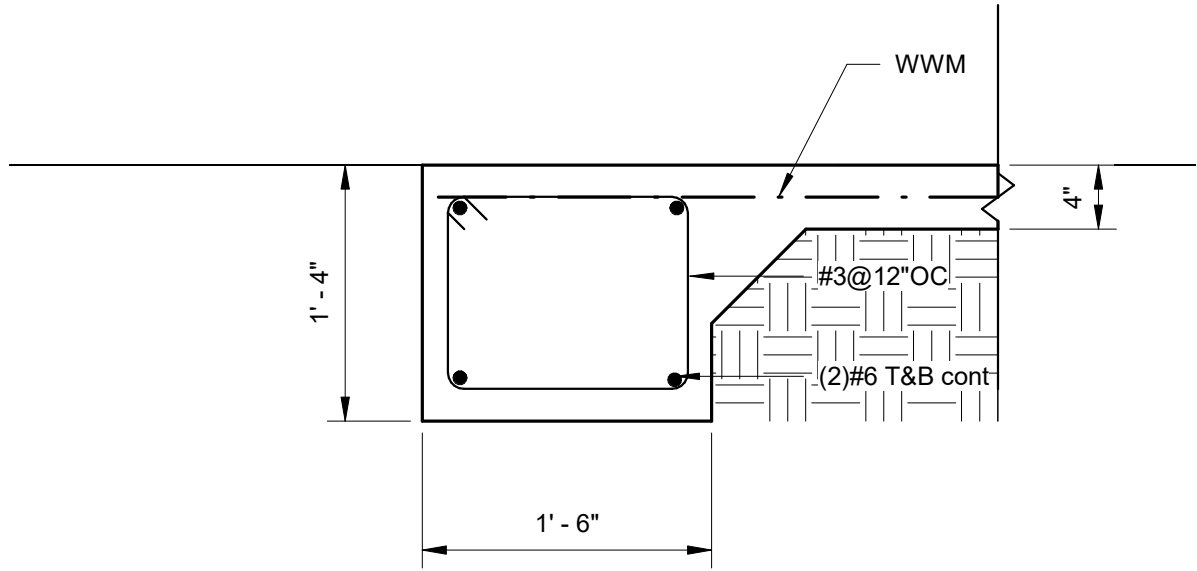
1. ROOF DECK SHALL BE TYPE "B" 1.5" X20 GA. GALVANIZE
2. FASTENING PATTERN SHALL BE 36/7 @ SUPPORTS AND (4) SIDE LAP CONNECTIONS.
3. FASTENERS SHALL BE #12 SDS SCREWS AT SUPPORTS ANDS # 10 SDS SCREWS AT SIDE LAP TYPICAL.



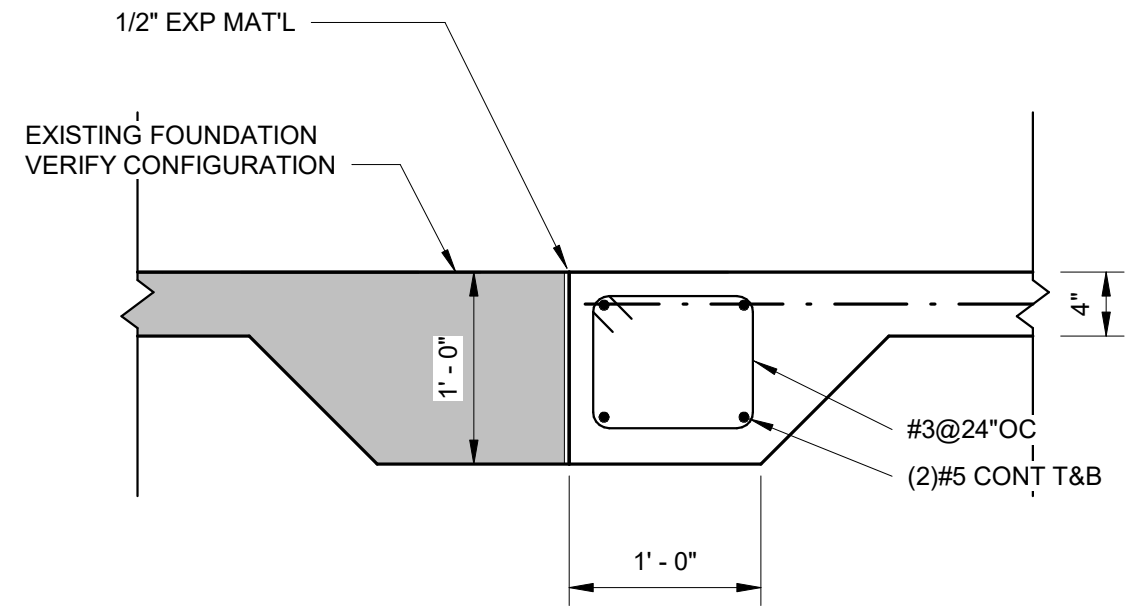
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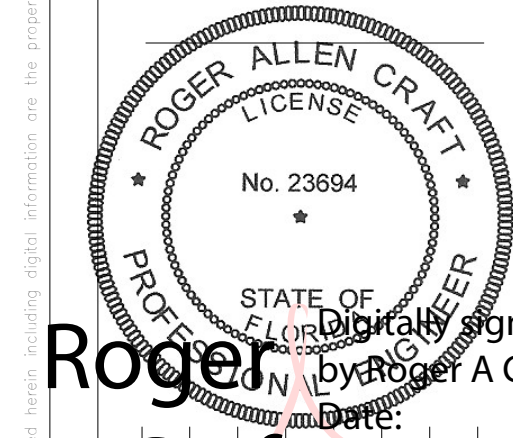
1 FF-ENTRY ENCLOSURE
1/4" = 1'-0"



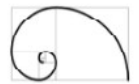
2 SECTION @ NEW ENTRY
1" = 1'-0"



3 SECTION @ EXISTING FOUNDATION
1" = 1'-0"



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By Roger A Craft
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11:34:39 -05'00'



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PLANS &
SECTIONS

S.1.1

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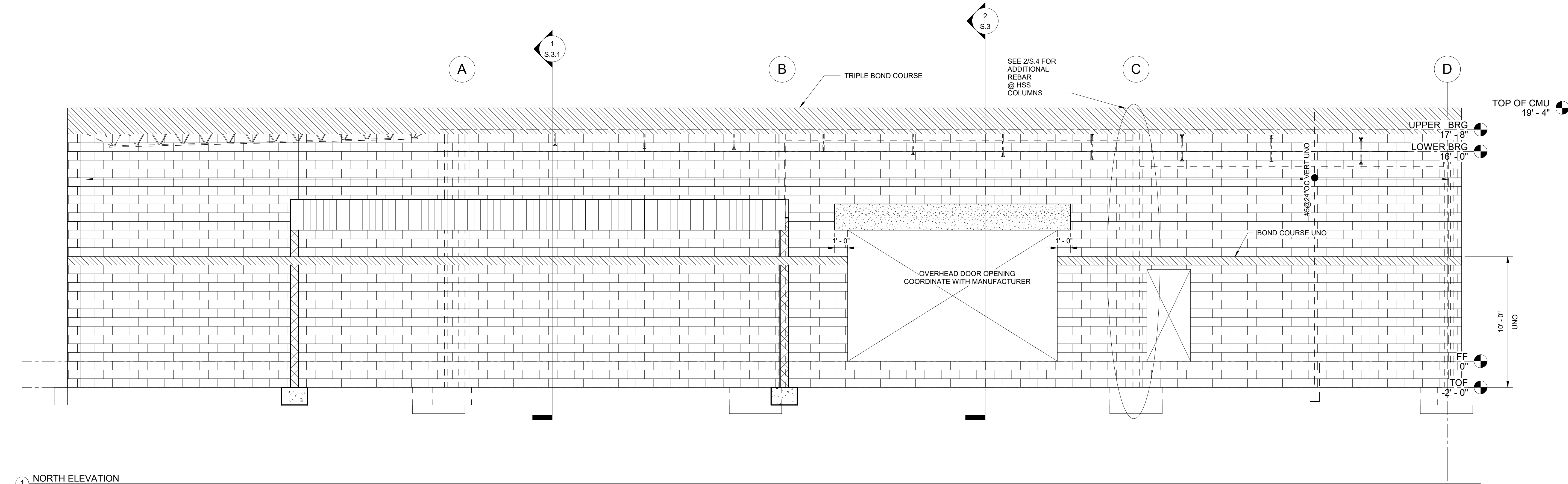
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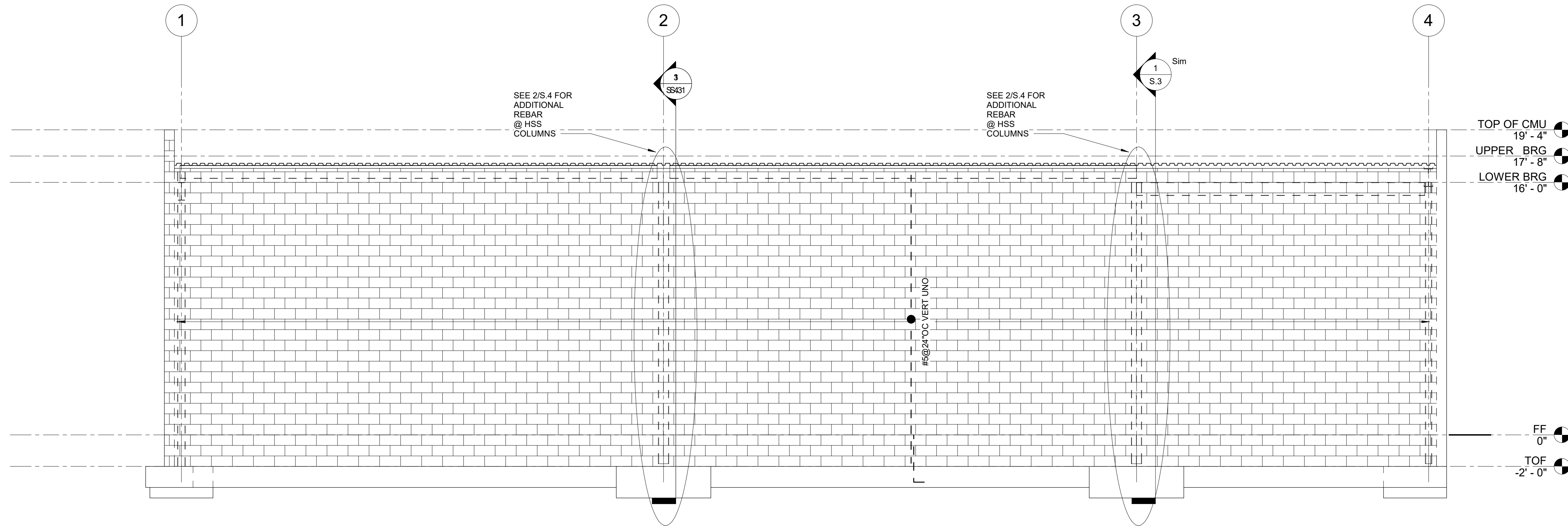
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1 NORTH ELEVATION
1/4" = 1'-0"



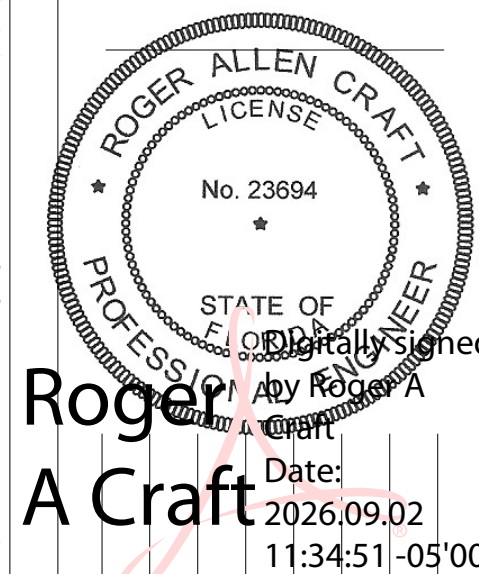
2 WEST ELEVATION
1/4" = 1'-0"

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Project Number:	
Revisions	

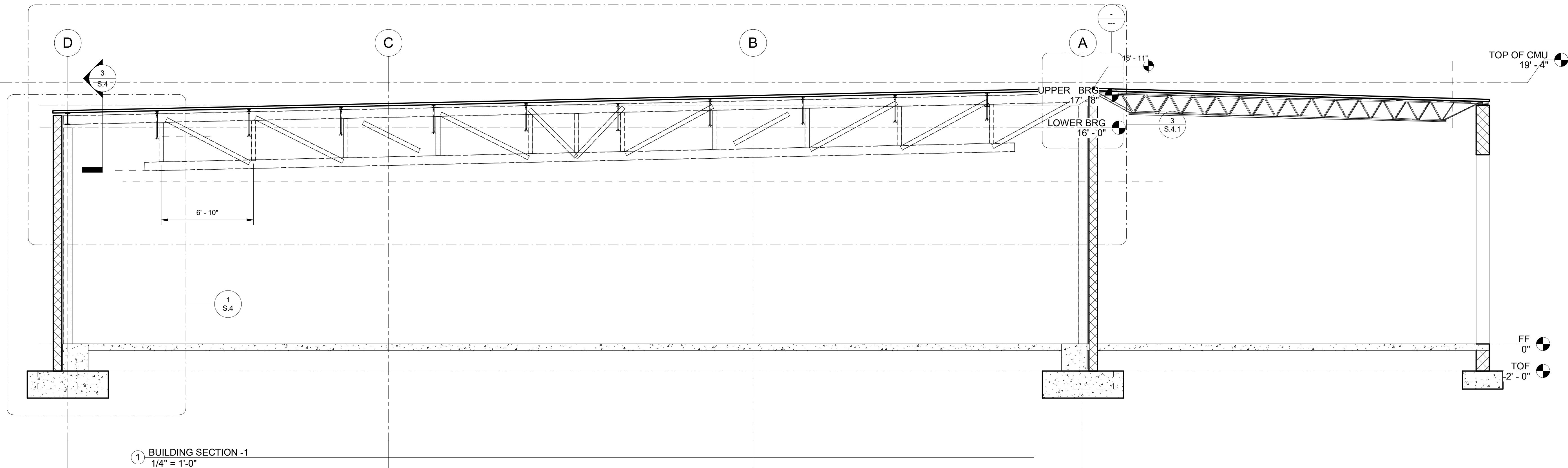
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BUILDING
ELEVATIONS

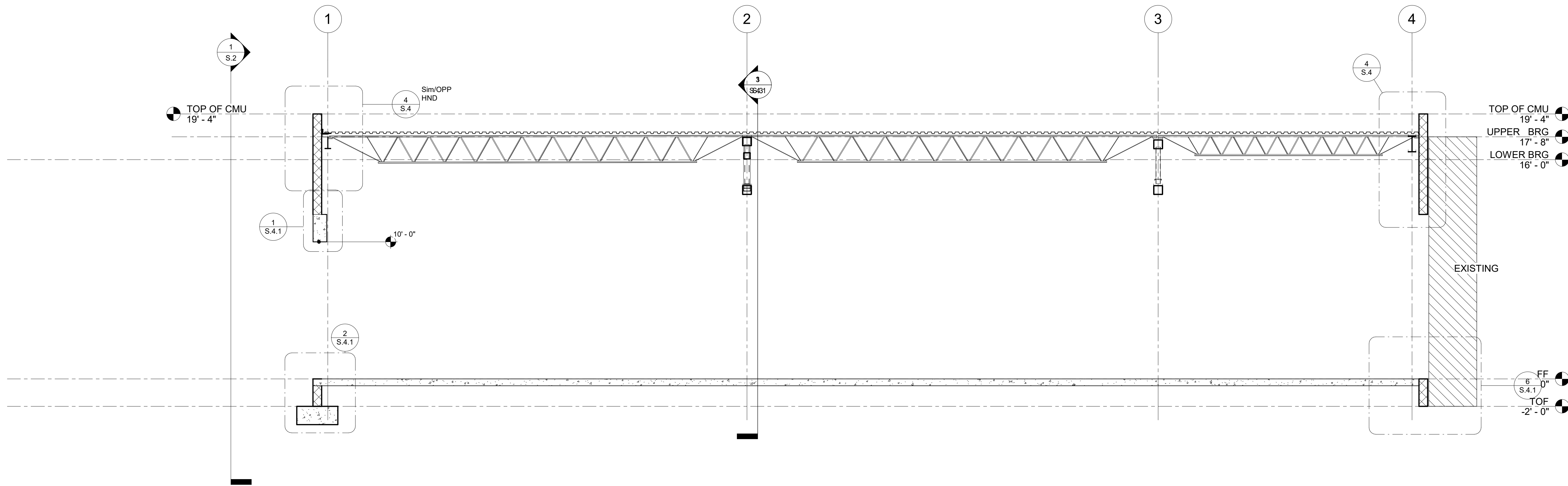
S.2

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2
S.2



1 BUILDING SECTION -1
1/4" = 1'-0"



2 BUILDING SECTION -2
1/4" = 1'-0"

2024.09.02

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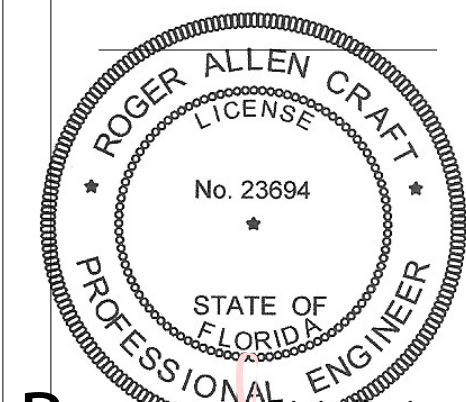
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BUILDING
SECTIONS

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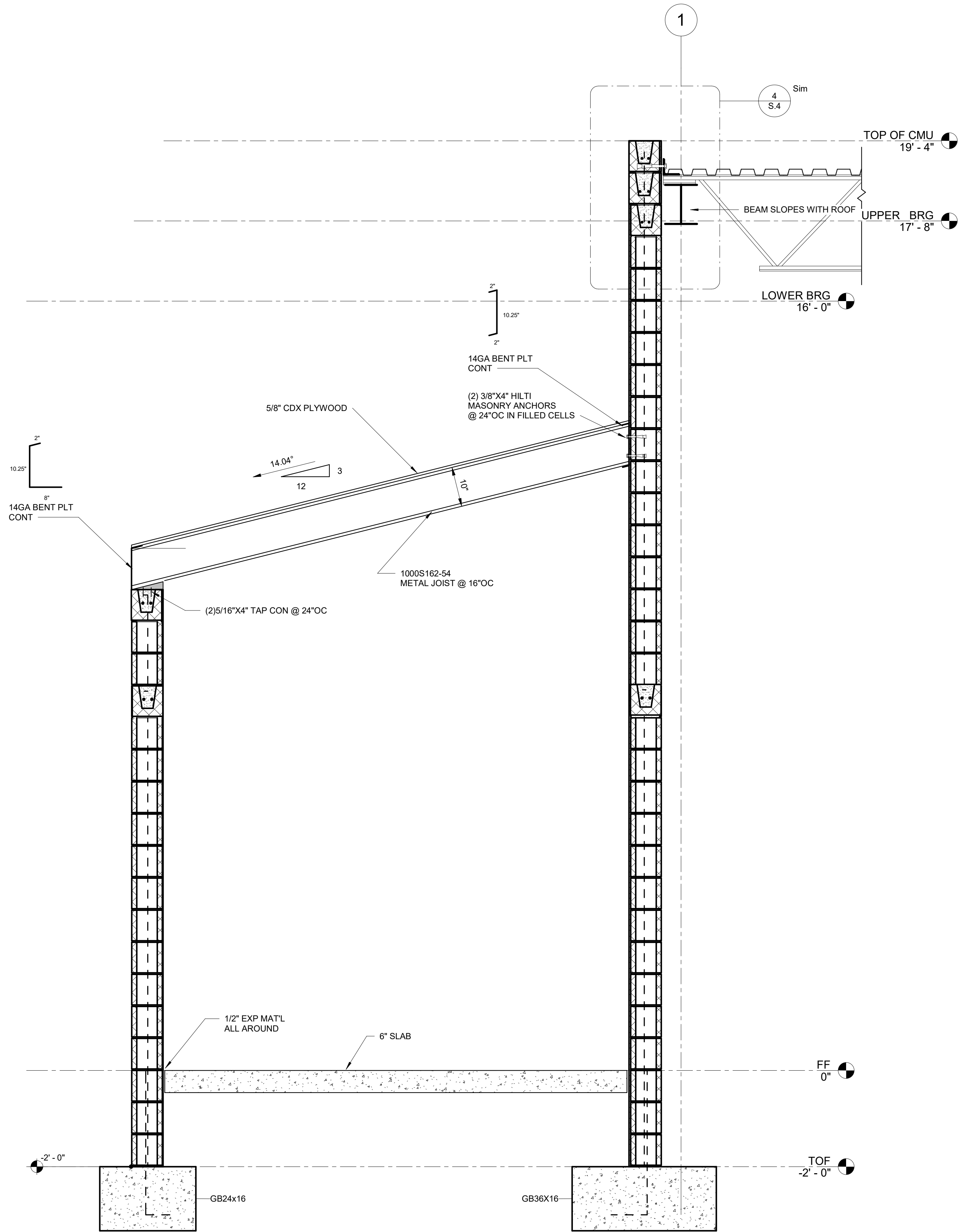
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by Roger A. Craft
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S.3



1 PARTIAL BUILDING SECTION -1
3/4" = 1'-0"

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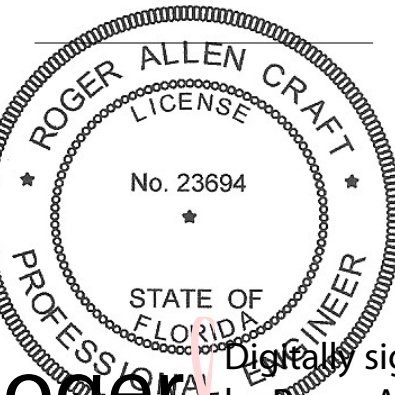
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O

BUILDING
SECTIONS

S.3.1



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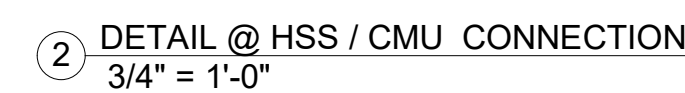
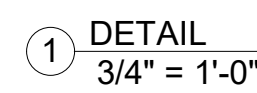
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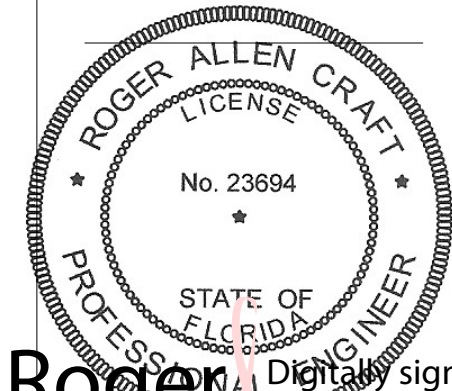


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S.4

1 DETAIL @ GARAGE DOOR HEADER
3/4" = 1'-0"

2 DETAIL @ GARAGE DOOR THRESHOLD
3/4" = 1'-0"

⑥ DETAIL @ EXPANSION JOINTS SLAB
3/4" = 1'-0"

4 DETAIL MAIN TRUSS SPLICE CONNECTION
1" = 1'-0"

THIS DETAIL APPLIES @ LINE D
EXCEPT NO SLOT IN COLUMN
CAP PLT

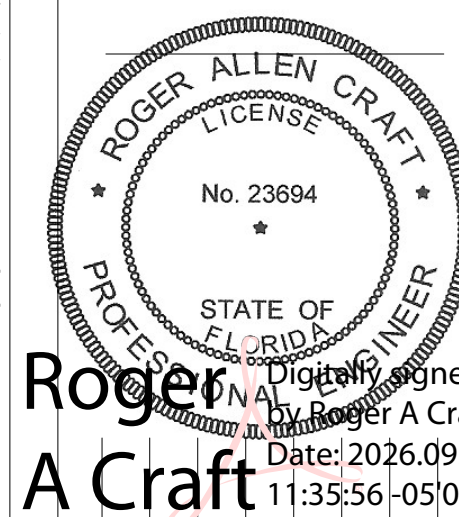
5 MAIN TRUSS BEARING PLT DETAIL
1 1/2" = 1'-0"

3 MAIN TRUSS DETAIL
3/8" = 1'-0"

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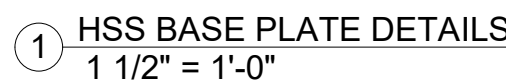


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DETAILS /
SECTIONS
MAIN
TRUSS
S.4.1

○ COLUMN SCHEDULE
1/4" = 1'-0"



③ CMU DETAILS
3/4" = 1'-0"

