

STRUCTURAL SPECIFICATIONS (CONTINUED)  
DRILL-IN BOLTS, SCREWS AND DOWELS (CONTINUED)

8. HEAVY-DUTY CONCRETE AND MASONRY SCREWS SHALL BE TESTED AND APPROVED TO MEET THE MINIMUM REQUIREMENTS OF ACI 308.2. HILTI KWICK HUS EZ (ESR-3027 FOR CONCRETE, ESR-3056 FOR GROUT FILLED MASONRY), HEAVY DUTY SCREWS BY HILTI OR EQUAL.
9. THE CONTRACTOR SHALL ARRANGE FOR AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THE ANCHORING PRODUCTS SPECIFIED. PENNONI TO RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO ARE TO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLATION.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO THE AISC "SPECIFICATION FOR BUILDINGS", LATEST EDITION.
2. WELDED CONNECTIONS SHALL CONFORM TO THE LATEST REVISED CODE OF THE AMERICAN WELDING SOCIETY, AWS D1.1. ALL WELDING SHALL BE PERFORMED USING E70XX, LOW HYDROGEN ELECTRODES. ELECTRODES ARE TO BE PROTECTED FROM MOISTURE.
3. CONNECTIONS TO BE DOUBLE ANGLE FRAMED BEAM CONNECTION PER AISC UNLESS NOTED OTHERWISE. ALL BOLTS TO BE 3/4" DIAMETER UNLESS NOTED OTHERWISE. SHOP CONNECTIONS MAY BE WELDED OR BOLTED. WELDS ARE TO BE EQUAL IN STRENGTH TO BOLTS. ALL FIELD CONNECTIONS ARE TO BE BOLTED WITH ASTM A325N OR A490 BOLTS (BEARING TYPE BOLTS WITH THREADS IN THE SHEAR PLANE) INCLUDING SUITABLE NUTS AND PLAIN HARDENED WASHERS. ALL BOLTS SHALL BE TIGHTENED SNUG TIGHT UNLESS OTHERWISE NOTED. DESIGN CONNECTIONS FOR THE LARGER OF EITHER THE SHEAR SHOWN ON THE DRAWINGS, (INDICATED AS "V =K" AT ENDS OF MEMBER) OR 55% OF THE MAXIMUM LOAD(N KIPS) LISTED IN THE TABLES FOR "MINIMUM TOTAL FACTORED UNIFORM LOADS IN KIPS FOR BRACED, SIMPLE SPAN BEAMS BENT ABOUT THE STRONG AXIS" OF THE LATEST EDITION OF THE AISC "MANUAL OF STEEL CONSTRUCTION".
4. SIZE AND USE OF HOLES: SEE AISC TABLE J3.3.
- A) LARGER HOLES ARE PERMITTED IN STANDARD COLUMN BASE PLATES. MAXIMUM HOLE DIAMETER = BOLT DIAMETER + 3/8". HARDENED WASHERS, TO COVER THE LARGER HOLE, SHALL BE PROVIDED.
- B) LARGER HOLES ARE NOT PERMITTED IN WIND FRAME COLUMN BASE PLATES. MAXIMUM HOLE DIAMETER = BOLT DIAMETER + 1/16".
- C) SLOTTED HOLES: A PLATE WASHERS OR A CONTINUOUS BAR WITHSTANDARD HOLES, HAVING A SIZE SUFFICIENT TO COMPLETELY COVER THE SLOT AFTER INSTALLATION, AND A MIN. OF 5/16" THICK SHALL BE PROVIDED. TACK WELD NUT TO BOLT AFTER ERECTION.
5. STEEL BEAMS SHALL BE FABRICATED WITH THE NATURAL CAMBER (WITHIN THE MILL TOLERANCE) LOCATED ABOVE THE HORIZONTAL CENTERLINE BETWEEN THE END CONNECTIONS.
6. VERIFY THE EXACT SIZE AND LOCATION OF ALL FLOOR AND ROOF OPENINGS FOR MECHANICAL EQUIPMENT WITH THE MECHANICAL CONTRACTOR PRIOR TO FABRICATION OF MATERIALS.
7. SHOP PRIME STEEL SURFACES EXCEPT THE FOLLOWING:
- A) SURFACES EMBEDDED IN CONCRETE OR MORTAR. EXTEND PRIMING OF PARTIALLY EMBEDDED MEMBERS TO A DEPTH OF 2 INCHES.
- B) SURFACES TO BE FIELD WELDED.
- C) SURFACES TO BE HIGH-STRENGTH BOLTED WITH SLIP-CRITICAL CONNECTIONS.
- D) SURFACES TO RECEIVE SPRAYED FIRE-RESISTIVE MATERIALS.
- E) GALVANIZED SURFACES.
8. SURFACE PREPARATION: CLEAN SURFACES TO BE PAINTED. REMOVE LOOSE RUST AND MILL SCALE AND SPATTER, SLAG, OR FLUX DEPOSITS. PREPARE SURFACES ACCORDING TO THE FOLLOWING SPECIFICATIONS AND STANDARDS.
9. PRIMING: IMMEDIATELY AFTER SURFACE PREPARATION, APPLY PRIMER ACCORDING TO MANUFACTURER'S WRITTEN INSTRUCTIONS AND AT RATE RECOMMENDED BY SSPC TO PROVIDE A DRY FILM THICKNESS OF NOT LESS THAN 1.5 MILS. USE PRIMING METHODS THAT RESULT IN FULL COVERAGE OF JOINTS, CORNERS, EDGES, AND EXPOSED SURFACES.
- A) STRIPE PAINT CORNERS; CREVICES, BOLTS, WELDS, AND SHARP EDGES.
- B) APPLY TWO COATS OF SHOP PAINT TO INACCESSIBLE SURFACES AFTER ASSEMBLY OR ERECTION. CHANGE COLOR OF SECOND COAT TO DISTINGUISH IT FROM FIRST.
10. PRIME AND PAINT ALL FIELD WELDS AFTER INSPECTION.
11. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO PERFORM THE FOLLOWING TESTS.
- A) VISUALLY INSPECT ALL STEEL MEMBERS AND CONNECTIONS.
- B) TEST 50 PERCENT OF FULL PENETRATION WELDS.
12. ONE COPY OF ALL TEST REPORTS SHALL BE SENT DIRECTLY TO OWNER, ARCHITECT, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR.
13. STEEL SHALL CONFORM TO:
- WIDE FLANGE (WF)(WT)----- ASTM A992 (50 KSI)
- SHAPES (S, L, C, MC)----- ASTM A36
- HOLLOW STRUCTURAL SECTIONS (HSS)---ASTM A500 GRADE C  
(RECTANGULAR 50 KSI; ROUND 46 KSI)
- STEEL PIPE-----ASTM A53
- ANCHOR RODS----- ASTM F1554 (55 KSI W/S1 SUPPLEMENT)
- ANCHOR BOLTS----- ASTM A307
- FRAMING BOLTS-----ASTM A325N OR A490N
- SHEAR STUDS-----ASTM A108
- WELDING ELECTRODES-----E70XX
14. FASTENERS AND MATERIALS USED FOR WELDING OR OTHERWISE SECURING COMPONENTS ONE TO ANOTHER SHALL BE OF DOMESTIC (USA MADE) MANUFACTURE. SIMILARLY, ALL MATERIALS USED IN THE MANUFACTURING PROCESS SHALL BE FROM A DOMESTIC SOURCE.
15. OPENINGS THROUGH STEEL BEAMS SHALL BE PROVIDED AS DETAILED ON THE DRAWINGS. ALL SUCH OPENINGS SHALL BE MACHINE CUT IN THE SHOP. ALL RECTANGULAR OPENINGS SHALL HAVE A CORNER RADIUS OF 2 TIMES THE WEB THICKNESS, 1/2" MINIMUM.
16. SHOP AND FIELD WELDS SHALL BE DONE BY A.W.S. CERTIFIED WELDERS. PROVIDE CURRENT CERTIFICATES UPON REQUEST.
17. NO SPLICES SHALL BE PERMITTED IN ANY STRUCTURAL STEEL MEMBER UNLESS SHOWN ON

APPROVED SHOP DRAWINGS.

18. STEEL STAIRS AND/OR LADDERS SHALL BE DESIGNED FOR 100 PSF LIVE LOAD BY A LICENSED DELEGATED ENGINEER, WHO SHALL SUBMIT SIGNED AND SEALED SHOP DRAWINGS. SHOP DRAWINGS SHALL SPECIFY ALL DESIGN LOADS.
19. SUBMITTALS: CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS SHOWING ALL STRUCTURAL STEEL LAYOUTS AND DETAILS, SIZES OF MEMBERS, TYPE OF STEEL, CONNECTION DETAILS, WELDS, BOLTS, ETC., AS REQUIRED TO FABRICATE AND ERECT ALL STRUCTURAL STEEL FRAMING. ALL CONNECTIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE DETAILER AND SUBMITTED ON SHOP DRAWINGS, SIGNED AND SEALED BY A REGISTERED FLORIDA DELEGATED ENGINEER.
20. NON-SHRINK GROUT SHALL BE: NONMETALLIC SHRINKAGE-RESISTANT GROUT, PREMIXED, NON-CORROSIVE, NON-STAINING PRODUCT CONTAINING SELECTED SILICA SANDS, PORTLAND CEMENT, SHRINKAGE COMPENSATING AGENTS, PLASTICIZING AND WATER-REDUCING AGENTS, COMPLYING WITH CRD-C621, CORPS OF ENGINEERS.
21. IF NOT SPECIFIED ON THE DRAWINGS, THE THROAT SIZE OF ANY FILLET WELD SHALL BE EQUAL TO 1/16" LESS THAN THE THINNEST CONNECTION COMPONENT.
22. NO FIELD WELDING OF GALVANIZED MEMBERS IS PERMITTED.
23. MINIMUM EMBEDMENT DEPTH OF ANCHOR BOLTS:
- A) BEAMS, COLUMNS, WALLS = 6"
- B) FOOTINGS = 3" FROM BOTTOM
24. ERECTION
- A) BEFORE ERECTION, THE CONTRACTOR IS TO REMOVE ALL MUD, DIRT OR OTHER FOREIGN MATTER, WHICH ACCUMULATES DURING HANDLING AND STORAGE.
- B) DRIFTING TO ENLARGE UNFAIR HOLES WILL NOT BE PERMITTED. DRILL SUCH HOLES TO ACCOMMODATE THE NEXT LARGER SIZE FASTENER, WHERE POSSIBLE.
- C) AFTER ERECTION, CLEAN FIELD WELDS, BOLTED CONNECTIONS, AND ABRADED AREAS WHERE SHOP COAT HAS BEEN DAMAGED. SPOT AND PRIME AREAS USING SAME MATERIAL AS SHOP COAT.
- D) SET ALL MEMBERS SO THAT, IN THEIR FINAL LOCATION, LEVEL, PLUMBNESS AND ALIGNMENT ARE WITHIN THE TOLERANCES PRESCRIBED BY AISC CODE.
- E) DOUBLE CONNECTIONS THROUGH COLUMN WEBS OR AT BEAMS THAT FRAME OVER THE TOPS OF COLUMNS MUST BE DESIGNED TO HAVE AT LEAST ONE INSTALLED BOLT REMAIN IN PLACE TO SUPPORT THE FIRST BEAM WHILE THE SECOND BEAM IS BEING ERECTED. ALTERNATIVELY, THE FABRICATOR MUST SUPPLY A SEAT OR EQUIVALENT DEVICE WITH A MEANS OF POSITIVE ATTACHMENT TO SUPPORT THE FIRST BEAM WHILE THE SECOND BEAM IS BEING ERECTED.

METAL DECKING

1. METAL DECK WORK SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL DECK INSTITUTE.
2. METAL FLOOR DECK SHALL BE 9/16" DEEP, 26 GA. GALVANIZED PERMANENT FORM DECK (VULCRAFT 0.6C26 OR EQUIVALENT).
3. FASTEN FLOOR DECK TO EACH INTERMEDIATE SUPPORT WITH (2) 5/8" DIAMETER PUDDLE WELDS AND (3) 5/8" DIAMETER PUDDLE WELDS TO EACH END LAP SUPPORT PER 36 INCH WIDTH. SIDELAPS SHALL BE FASTENED WITH #12TEK SCREWS AT 36" ON CENTER MAXIMUM.
4. MINIMUM FASTENING AT BUILDING PERIMETER OF DECK SHALL BE 5/8" DIAMETER PUDDLE WELDS AT 6" O.C.
5. METAL DECK AND SHEET METAL COATING DESIGNATION:
- A) WITH STRUCTURAL CONCRETE OR INSULATING CONCRETE TOPPING - G90
- B) WITHOUT STRUCTURAL CONCRETE OR INSULATING CONCRETE TOPPING - G60
6. INSTALL ALL DECKING 3 SPAN CONTINUOUS.
7. USE WELD WASHERS FOR ALL DECKING 24 GA. AND THINNER.
8. DO NOT HANG OR ATTACH DUCTWORK, CONDUIT, PIPING, EQUIPMENT, CEILINGS, ETC. FROM METAL DECKING.
9. PRIME AND PAINT ALL FIELD WELDS AFTER INSPECTION WITH A GALVANIZED TOUCH-UP PAINT. (SEE NOTE BELOW)
10. SUBMITTALS: CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS SHOWING LAYOUT OF DECK, TYPE OF DECK, ALL CONNECTIONS INCLUDING END WELDS, SEAM WELDS, INTERMEDIATE WELDS, AND ALL ACCESSORY MATERIAL SUCH AS CLOSURES, SUMPS FOR DRAINS, ETC.
11. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO VISUALLY INSPECT ALL DECK WELDS AND FASTENERS.

VERTICAL REINFORCEMENT BAR LAP SCHEDULE			
BAR SIZE	CLASS "B" TENSION LAP		
	3,000 PSI	4,000 PSI	5,000 PSI
#4	29"	25"	23"
#5	36"	31"	28"
#6	43"	37"	33"
#7	63"	54"	49"
#8	72"	62"	55"
#9	81"	70"	63"
#10	91"	79"	70"

NOTES:  
1.BASED ON NORMAL WEIGHT CONCRETE 4 GRADE 60  
REINFORCING BARS.

VERTICAL REINFORCEMENT BAR  
LAP SCHEDULE - CONCRETE

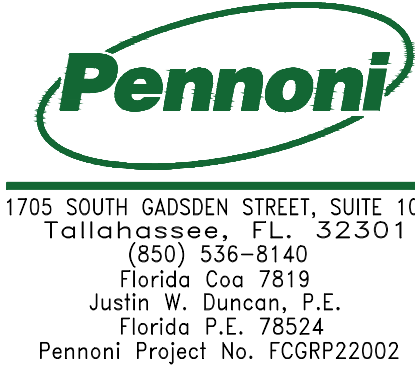
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CONCRETE BEAM TENSION LAP SPLICE SCHEDULE				
BAR SIZE	LOCATION	CONCRETE STRENGTH		
		3,000 PSI	4,000 PSI	5,000 PSI
#4	TOP BARS	37"	32"	29"
	OTHER BARS	29"	25"	22"
#5	TOP BARS	47"	40"	36"
	OTHER BARS	36"	31"	28"
#6	TOP BARS	56"	48"	43"
	OTHER BARS	43"	37"	33"
#7	TOP BARS	81"	70"	63"
	OTHER BARS	63"	54"	49"
#8	TOP BARS	93"	80"	72"
	OTHER BARS	72"	62"	55"
#9	TOP BARS	105"	91"	81"
	OTHER BARS	81"	70"	63"
#10	TOP BARS	118"	102"	91"
	OTHER BARS	91"	79"	70"

NOTES:

1. BASED ON NORMAL WEIGHT CONCRETE 4 GRADE 60  
REINFORCING BARS.
2. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES  
OF CONCRETE CAST BELOW THE BARS.
3. FOR LIGHTWEIGHT AGGREGATE, MULTIPLY ABOVE VALUES BY 1.3.

CONCRETE BEAM TENSION LAP  
SPLICE SCHEDULE



FITZGERALD  
COLLABORATIVE

ATLANTA  
1201 W. PEACHTREE ST., SUITE 630  
ATLANTA, GA 30309

TALLAHASSEE  
850 S. GADSDEN ST., SUITE 140  
TALLAHASSEE, FL 32301

FITZGERALD COLLABORATIVE GROUP, LLC.  
AA26001957



REVISION NO.	DESCRIPTION	DATE

PERMIT  
DOCUMENTS



PANAMA CITY AIRPORT NWFBI:  
ESCALATOR  
ADDITION

6300 WEST BAY PKWY,  
PANAMA CITY, FL 32409

PROJECT NUMBER  
  
NO. 21049  
  
ISSUE DATE  
  
3.15.2023

STRUCTURAL  
SPECIFICATIONS  
& SCHEDULES

SHEET NUMBER  
  
S-0.0.2

STRUCTURAL SPECIFICATIONS

MISCELLANEOUS

1.

THE STRUCTURAL SYSTEM IS UNSTABLE UNTIL ALL CONNECTIONS HAVE BEEN MADE AND ALL CONCRETE HAS REACHED ITS MINIMUM DESIGN STRENGTH, AS SHOWN IN THE STRUCTURAL DOCUMENTS.
2.

CONTRACTOR IS RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION TO ENSURE THE SAFETY OF THE BUILDING UNTIL STRUCTURAL SYSTEM IS COMPLETED. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF WHATEVER TEMPORARY BRACING, SHORING, GUYS OR TIE-DOWNS THAT MAY BE NECESSARY. SUCH MATERIAL SHALL BE REMOVED AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.
3.

CONTRACTOR TO SUPPORT, BRACE AND SECURE EXISTING STRUCTURE AS REQUIRED. CONTRACTOR IS SOLELY RESPONSIBLE FOR THE SAFETY OF THE BUILDING DURING CONSTRUCTION.
4.

APPLICABLE BUILDING CODE: 7<sup>TH</sup> EDITION (2020) FLORIDA BUILDING CODE.
5.

GRAVITY DESIGN LOADS:

AREA	SUPERIMPOSED LIVE LOAD	TOTAL DEAD LOAD
LEVEL 2 ADDITION	100 PSF	75 PSF
6.

WIND DESIGN CRITERIA:  
ULTIMATE WIND SPEED: V<sub>ULT</sub> = 143 MPH (3 SECOND GUST)  
EQUIVALENT NOMINAL BASIC WIND SPEED V<sub>ASD</sub> = 111 MPH (3 SECOND GUST)  
RISK CATEGORY = III  
EXPOSURE CATEGORY = D  
ENCLOSED BUILDING INTERNAL PRESSURE COEFFICIENT, GC<sub>PI</sub>= +/-0.18  
WIND BORNE DEBRIS REGION
7.

ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE REFERENCED BUILDING CODE.
8.

COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.
9.

CONTACT ENGINEER WITH ANY QUESTIONS OR DISCREPANCIES FOUND ON DRAWINGS.
10.

BUILDING EXPANSION JOINTS (EJ), WHERE SHOWN, WILL EXPAND AND CONTRACT OVER THE LIFE OF THE BUILDING. JOINT SEALANTS AND COVERS MUST ACCOMMODATE THIS MOVEMENT.
11.

SECTIONS AND DETAILS ARE REFERENCED IN TYPICAL LOCATIONS BUT ALSO APPLY TO ALL OTHER SIMILAR CONDITIONS.
12.

CONTRACTOR TO VERIFY ALL EXISTING DIMENSIONS, ELEVATIONS, AND CONDITIONS PRIOR TO BEGINNING CONSTRUCTION.
13.

SUBMIT SHOP DRAWINGS AS REQUIRED HEREIN. ALLOW FOR TWO WEEKS REVIEW TIME AFTER RECEIPT OF SUBMITTALS BY THIS FIRM. ALL SUBMITTALS SHALL BE CHECKED AND SIGNED BY THE GENERAL CONTRACTOR AND SIGNED/SEALED BY THE DELEGATED ENGINEER, WHERE SPECIFIED HEREIN.
14.

CONTRACTOR SHALL NOT BE RELIEVED FROM RESPONSIBILITY FOR ERRORS OR OMISSIONS IN SHOP DRAWINGS OR MIX DESIGNS BY THE ENGINEER'S REVIEW THEREOF.
15.

ANY CHANGES TO THE STRUCTURE SHALL HAVE BEEN REVIEWED AND APPROVED IN WRITING BY THE ENGINEER PRIOR TO COMMENCING WORK ON ITEMS AFFECTED.
16.

CONTRACTOR SHALL NOTIFY THIS OFFICE WHEN THE STRUCTURAL SYSTEM IS SUBSTANTIALLY COMPLETED, AND BEFORE SHEATHING, CEILINGS, OR ROOFING IS INSTALLED.

HAND RAILS

1.

AN ENGINEER REGISTERED IN THE STATE OF FLORIDA SHALL DESIGN RAILING SYSTEM AND CONNECTION OF IT TO THIS STRUCTURE.
2.

SUBMIT SHOP DRAWINGS BEARING THE EMBOSSED SEAL AND THE SIGNATURE OF THE ENGINEER FOR REVIEW PRIOR TO FABRICATION.
3.

THE CONFIGURATION OF THE RAILING SYSTEM SHALL BE AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
4.

RAILING SYSTEM AND CONNECTIONS SHALL BE DESIGNED FOR APPLICABLE LOADS AS INDICATED ON THE PLANS AND IN THE BUILDING CODE. THE LOADS SHALL BE CLEARLY INDICATED ON SHOP DRAWINGS.
5.

SHOP DRAWINGS SHALL SHOW AND SPECIFY CONNECTIONS UTILIZED WITHIN THE RAILING SYSTEM AS WELL AS CONNECTIONS TO AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM SHOWN ON THESE PLANS.

DELEGATED ENGINEER

1.

WHERE NOTED HEREIN, A LICENSED PROFESSIONAL (DELEGATED) ENGINEER SHALL BE RETAINED TO DESIGN THE PRODUCT OR ASSEMBLY.
2.

THE DELEGATED ENGINEER SHALL BE EXPERIENCED IN THE DESIGN OF THE REFERENCED PRODUCT OR ASSEMBLY.
3.

THE DELEGATED ENGINEER MUST BE PROVIDED WITH A COPY OF THESE DRAWINGS AND SPECIFICATIONS.
4.

IT IS THE DELEGATED ENGINEER'S RESPONSIBILITY TO REVIEW THE ENGINEER OF RECORD'S WRITTEN ENGINEERING REQUIREMENTS AND AUTHORIZATION FOR THE DELEGATED ENGINEERING DOCUMENT TO DETERMINE THE APPROPRIATE SCOPE OF ENGINEERING.
5.

THE DELEGATED ENGINEERING DOCUMENT SHALL COMPLY WITH THE WRITTEN ENGINEERING REQUIREMENTS RECEIVED FROM THE ENGINEER OF RECORD. THEY SHALL INCLUDE THE PROJECT IDENTIFICATION AND THE CRITERIA USED AS A BASIS FOR ITS PREPARATION. IF A DELEGATED ENGINEER DETERMINES THERE ARE DETAILS, FEATURES OR UNANTICIPATED PROJECT LIMITS WHICH CONFLICT WITH THE WRITTEN ENGINEERING REQUIREMENTS PROVIDED BY THE ENGINEER OF RECORD, THE DELEGATED ENGINEER SHALL TIMELY CONTACT THE ENGINEER OF RECORD FOR RESOLUTION OF CONFLICTS.
6.

THE DELEGATED ENGINEER SHALL FORWARD THE DELEGATED ENGINEERING DOCUMENT TO
- THE ENGINEER OF RECORD FOR REVIEW. ALL FINAL DELEGATED ENGINEERING DOCUMENTS REQUIRE THE IMPRESSED SEAL AND SIGNATURE OF THE DELEGATED ENGINEER AND INCLUDE:

A)

DRAWINGS INTRODUCING ENGINEERING INPUT SUCH AS DEFINING THE CONFIGURATION OR STRUCTURAL CAPACITY OF STRUCTURAL COMPONENTS AND/OR THEIR ASSEMBLY INTO STRUCTURAL SYSTEMS.

B)

CALCULATIONS.
- DEFERRED SUBMITTALS BY DELEGATED ENGINEERS
1.

IN ACCORDANCE WITH FBC 107.3.4.1, THE FOLLOWING PRE-ENGINEERED SPECIALITY ITEMS FOR PORTIONS OF THE BUILDING WILL NOT BE SUBMITTED AT THE TIME OF BUILDING PERMIT APPLICATION BUT WILL BE DEFERRED UNTIL AFTER THE PERMIT HAS BEEN ISSUED:

A)

STAIRS, RAILING FOR STAIRS AND LANDINGS

2.

THESE ELEMENTS ARE PERFORMANCE-BASED DESIGN. THE CONTRACTOR SHALL CONTRACT FOR THE DESIGN AND CONSTRUCTION OF THESE ELEMENTS DURING THE CONSTRUCTION PHASE. THE SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED AND SIGNED BY A LICENSED FLORIDA DELEGATED PROFESSIONAL ENGINEER PER FLORIDA STATUTES. THEY SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.
- EXISTING BUILDINGS
- INFORMATION ON THE EXISTING BUILDING, SHOWN ON THESE PLANS, IS OBTAINED FROM EXISTING BUILDING PLANS BY HNTB ARCHITECTURE & PBS & J, DATED MAY 22, 2008. EXISTING INFORMATION DOES NOT NECESSARILY REFLECT AS-BUILT CONDITIONS. THE CONTRACTOR SHALL VERIFY ALL INFORMATION SHOWN ON THESE PLANS AND NOTIFY THE ENGINEER OF ANY VARIATION.
- CAST IN PLACE CONCRETE
1.

ALL CAST-IN-PLACE CONCRETE WORK INCLUDES REINFORCING STEEL AND RELATED WORK SHOWN INCLUDING FORMWORK, SETTING ANCHOR BOLTS, PLATES, FRAMES, DOWELS FOR MASONRY OR OTHER ITEMS EMBEDDED IN CONCRETE.

2.

APPLICABLE STANDARDS

ACI NUMBER	TITLE
117	STANDARD SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION
226	GROUND GRANULATED BLAST-FURNACE SLAG
301	STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS
302	GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION
304	GUIDE FOR MEASURING MIXING, TRANSPORTING AND PLACING CONCRETE
304.2R	PLACING CONCRETE BY PUMPING METHODS.
305R	HOT WEATHER CONCRETING
306R	COLD WEATHER CONCRETING
308	STANDARD PRACTICE FOR CURING CONCRETE
309R	GUIDE FOR CONSOLIDATION OF CONCRETE
315	MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES
318	BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE
347	RECOMMENDED PRACTICE FOR CONCRETE FORMWORK
CRSI NUMBER	TITLE
63	RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS

3.

CONCRETE MATERIALS

A)

PORTLAND CEMENT – ASTM C 150, TYPE I

B)

AGGREGATES – NORMAL WEIGHT CONCRETE, COARSE AND FINE, ASTM C33. STRUCTURAL LIGHT WEIGHT ASTM C330.

C)

AIR-ENTRAINING – ASTM C260

D)

WATER REDUCING – ASTM C494, TYPE A

E)

WATER – FRESH, CLEAN AND POTABLE

F)

NO ACCELERATORS, RETARDERS OR ADMIXTURES CONTAINING CHLORIDES WILL BE PERMITTED

G)

FLY-ASH – ASTM C618, CLASS F, 20% MAXIMUM OF CEMENTITIOUS MATERIAL BY WEIGHT. DO NOT USE FOR EXPOSED SLABS OR ARCHITECTURAL CONCRETE.

H)

SUPER PLASTICIZER – ASTM C494, TYPE F OR G, WHERE AUTHORIZED BY THE ENGINEER.

I)

GROUND GRANULATED BLAST-FURNACE SLAG CEMENT – ASTM C989, 50% MAXIMUM BY WEIGHT.

J)

MAXIMUM AGGREGATE SIZE – FOOTINGS = #57, OTHERS #67

4.

REINFORCING MATERIALS

A)

DEFORMED BARS – ASTM A615, GRADE 60

B)

SMOOTH DOWELS – ASTM A615, PLAIN BARS, MINIMUM YIELD STRENGTH OF 60,000 PSI.

C)

CORROSION RESISTANT UNCOATED STEEL (MMFX-2) – ASTM A615, GRADE 75 AND ASTM A1035 LOW-CARBON (8% MINIMUM) CHROMIUM BY MMFX OR EQUAL.

D)

WELDED WIRE FABRIC – ASTM A1064, PLAIN WIRE FABRIC IN FLAT SHEETS ONLY.

E)

ACCESSORIES TO CONFORM TO ACI 315.

F)

WHERE CONCRETE SURFACES ARE EXPOSED, MAKE THOSE PORTIONS OF ALL ACCESSORIES IN CONTACT WITH THE CONCRETE SURFACE OR WITHIN 1/2 INCH THEREOF, OF PLASTIC OR STAINLESS STEEL.

5.

PROVIDE THE FOLLOWING MINIMUM CONCRETE STRENGTHS AT 28 DAYS:

A)

FOOTINGS, SLAB-ON-GRADE----- 3000 PSI

B)

REGULAR/LIGHTWEIGHT FILL ON METAL DECK----- 3000 PSI

C)

MASONRY WALL BEAMS, TIE COLUMNS----- 3000 PSI

D)

FORMED COLUMNS, WALLS, BEAMS & SLABS----- 4000 PSI

6.

CONCRETE MUST BE BATCHED, MIXED AND TRANSPORTED IN ACCORDANCE WITH THE SPECIFICATIONS FOR READY-MIXED CONCRETE ASTM C94.

7.

REQUIRED SLUMP = 4 PLUS OR MINUS ONE INCH.

8.

CONCRETE MUST BE PLACED WITHIN 90 MINUTES OF BATCH TIME. WHEN AIR TEMPERATURE IS BETWEEN 85 AND 90 DEGREES F, REDUCE MIXING AND DELIVERY TIME TO 75 MINUTES. WHEN AIR TEMPERATURE IS HIGHER THAN 90 DEGREES F, REDUCE MIXING AND DELIVERY TIME TO 60 MINUTES.

9.

DO NOT ADD WATER AT THE JOB SITE WITHOUT APPROVAL OF THE PROJECT SUPERINTENDENT. DO NOT EXCEED THE SLUMP LIMITATION. USE ONLY COLD WATER FROM THE TRUCK TANK. ANY ADDED WATER MUST BE INDICATED ON THE DELIVERY TICKET PLUS THE NAME OF THE PERSON AUTHORIZING. TEST CYLINDERS SHALL BE TAKEN AFTER THE ADDITION OF WATER.
10.

LAP SPlice REINFORCING PER CONCRETE LAP SCHEDULE MINIMUM UNLESS OTHERWISE SHOWN OR NOTED.

11.

PROVIDE CORNER BARS AT ALL WALL FOOTING, WALL AND BEAM CORNERS. SIZE AND NUMBER TO MATCH HORIZONTAL BARS.
12.

PROVIDE FOUNDATION DOWELS TO MATCH SIZE AND NUMBER OF VERTICAL BARS. EMBED DOWELS TO:

A)

3” ABOVE BOTTOM OF FOOTINGS

13.

REINFORCEMENT SHALL BE FASTENED AND SECURED TOGETHER TO PREVENT DISPLACEMENT BY CONSTRUCTION LOADS OR THE PLACING OF CONCRETE.
14.

REINFORCING BAR COVER

A)

FOOTINGS 2” (TOP), 3” (SIDES AND BOTTOM)

B)

COLUMNS AND BEAMS 1–1/2”

C)

SLABS 3/4” (INTERIOR), 1–1/2” (EXTERIOR)

15.

WHERE BAR LENGTHS ARE GIVEN ON THE DRAWINGS, LENGTH OF HOOK, IF REQUIRED, IS NOT INCLUDED.
16.

SELECT PROPORTIONS IN ACCORDANCE WITH ACI 301 TO PROVIDE CONCRETE CAPABLE OF BEING PLACED WITHOUT EXCESSIVE SEGREGATION AND WITH ACCEPTABLE FINISHING PROPERTIES, DURABILITY, SURFACE HARDENERS, APPEARANCE, AND STRENGTH REQUIREMENTS REQUIRED BY THESE SPECIFICATIONS.
17.

CHAIR WELDED WIRE FABRIC REINFORCING AT 3’–0” ON CENTER MAXIMUM IN EACH DIRECTION.
18.

MAXIMUM WATER TO CEMENT RATIO WHEN NO BACK-UP DATA IS AVAILABLE:

A)

4000 PSI, 28-DAY COMPRESSIVE STRENGTH; W/C RATIO, 0.44 MAXIMUM (NON-AIR-ENTRAINED), 0.36 MAXIMUM (AIR-ENTRAINED).

B)

3000 PSI, 28-DAY COMPRESSIVE STRENGTH; W/C RATIO, 0.58 MAXIMUM (NON-AIR-ENTRAINED), 0.47 MAXIMUM (AIR-ENTRAINED).
19.

DATA TO BE SUBMITTED:

A)

INTENDED USAGE AND LOCATION FOR EACH TYPE

B)

MIX DESIGN FOR EACH TYPE

C)

CEMENT CONTENT IN POUNDS-PER-CUBIC YARD

D)

COARSE AND FINE AGGREGATE IN POUNDS/CUBIC YARD

E)

WATER CEMENT RATIO BY WEIGHT

F)

CEMENT TYPE AND MANUFACTURER

G)

SLUMP RANGE

H)

AIR CONTENT

I)

ADMIXTURE TYPE AND MANUFACTURER

J)

PERCENT ADMIXTURE BY WEIGHT

K)

STRENGTH TEST DATA REQUIRED TO ESTABLISH MIX DESIGN.

L)

COMPLETE DETAIL AND PLACING SHOP DRAWINGS FOR ALL REINFORCING STEEL INCLUDING ACCESSORIES THAT HAVE BEEN REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR. INCLUDE ALL REQUIRED DIMENSIONS AND ELEVATIONS (IE. TOP OF CONCRETE)

20.

THE GENERAL CONTRACTOR IS RESPONSIBLE FOR PROVIDING THE CONSTRUCTION OF FORMWORK, SHORING AND RE-SHORING IN ACCORDANCE WITH ACI 347.

A)

FORM AND SHORING DESIGN BY A P.E. REGISTERED IN THE STATE OF FLORIDA.
21.

SUBMIT FORM WORK AND SHORING DRAWINGS TO LOCAL BUILDING DEPARTMENT WHEN REQUIRED BY FLORIDA THRESHOLD LAW.
22.

CONSTRUCTION JOINTS NOT SHOWN ON THE DRAWINGS MUST BE MADE AND LOCATED TO LEAST IMPAIR THE STRENGTH OF THE STRUCTURE.

A)

NO HORIZONTAL CONSTRUCTION JOINTS WILL BE PERMITTED IN BEAMS, GIRDERS AND SLABS.

B)

LOCATION OF ANY CONSTRUCTION JOINT NOT SHOWN IS SUBJECT TO REVIEW AND ACCEPTANCE BY ENGINEER.
23.

INTERNAL VIBRATION, PROPERLY APPLIED IS THE REQUIRED METHOD OF CONSOLIDATING PLASTIC CONCRETE.
24.

PROVIDE 3/4” CHAMFER ON ALL EXPOSED CORNERS OF COLUMNS, BEAMS AND WALLS UNLESS OTHERWISE NOTED ON ARCHITECTURAL DRAWINGS.
25.

CONTRACTOR SHALL VERIFY LOCATIONS OF ALL OPENINGS, SLEEVES, AND SLAB RECESSES AS REQUIRED BY OTHER TRADES BEFORE CONCRETE IS PLACED. NO SLEEVE, OPENINGS, OR INSERT MAY BE PLACED IN BEAMS, JOISTS, OR COLUMN UNLESS APPROVED BY THE ENGINEER.
26.

CONTRACTOR SHALL VERIFY EMBEDDED ITEMS INCLUDING, BUT NOT LIMITED TO, ANCHOR BOLTS, BOLT CLUSTERS, WELD PLATES, ETC., BEFORE PLACING CONCRETE. NOTIFY ENGINEER OF ANY CONFLICTS WITH REBAR.
27.

ALL EXPOSED CONCRETE SURFACES TO BE IN ACCORDANCE WITH ACI 301 SECTION 5.3.3.(C), INCLUDING SURFACE TOLERANCE CLASS A AS SPECIFIED IN ACI 117.U.N.O.
28.

SEE ARCHITECTURAL DRAWINGS FOR REQUIRED CONCRETE FINISHES.
29.

SLOPE WALKWAYS TO DRAIN AWAY FROM THE BUILDING.
30.

BUILDING FLOOR AND SITE SLABS-ON-GRADE SHALL BE 4” MINIMUM THICKNESS, UNLESS NOTED OTHERWISE.

A)

REINFORCED WITH 6X6 – W1.4 X W1.4 W.W.F.

B)

PLACED ON 10 MIL POLYETHYLENE VAPOR RETARDER. LAP 6” AND TAPE ALL JOINTS.

C)

SAW-CUT CONTROL JOINTS @ LESS THAN OR EQUAL TO 15’–0” EACH WAY.

D)

PROVIDE HOUSEKEEPING PADS AS REQUIRED.

E)

SEE DRAWINGS FOR ANY ADDITIONAL CONDITIONS.

31.

TESTING

A)

A QUALIFIED TESTING LAB SHALL BE RETAINED TO PERFORM QUALITY CONTROL WORK AND ON-SITE TESTING.

B)

SLUMP TEST – ASTM 143

C)

MOLD AND CURE TEST CYLINDERS (ASTM C-31) AND TEST CYLINDERS FOR STRENGTH (ASTM C39). TAKE ONE TEST – THREE CYLINDERS FOR EACH DAYS POUR OF 100 CUBIC YARDS, OR FRACTION THEREOF. TEST ONE CYLINDER AT 7 DAYS, TWO AT 28 DAYS. TEST CYLINDER SAMPLES SHALL BE TAKEN AT THE POINT OF DISCHARGE WHEN USING A PUMP.

D)

ONE COPY OF ALL TEST REPORTS SHALL BE SENT DIRECTLY TO THE OWNER, ENGINEER, ARCHITECT AND GENERAL CONTRACTOR.

32.1.

CONTRACTOR SHALL PROVIDE FLATNESS AND LEVELNESS IN CONCRETE SLABS PER ACI 302.1R, FIG. 10.7 MINIMUM REQUIRED “F” NUMBERS FOR TYPE OF SLAB USE. REFER TO ACI 117 FOR FLOOR TOLERANCES.

33.

REPAIR ANY CRACKS OR DEFECTIVE AREAS THAT WILL RESTORE THE AFFECTED SURFACE OR AREAS TO THEIR FULL DESIGN STRENGTH AND APPEARANCE. CONTACT THE STRUCTURAL ENGINEER FOR ADVICE AND EVALUATION.

34.

ACCEPTANCE OF THE STRUCTURE WILL BE MADE IN CONFORMANCE WITH ACI 301.

35.

ALL CAST-IN-PLACE CONCRETE MUST BE MAINTAINED WITH MINIMAL MOISTURE LOSS AT A RELATIVELY CONSTANT TEMPERATURE FOR A MINIMUM OF 7 DAYS FOLLOWING THE PLACING OF THE CONCRETE BY THE USE OF A WATER SPRAY, WATER SATURATED FABRIC, MOISTURE RETAINING MEMBRANE OR LIQUID CURING COMPOUND.

36.

CURE SLABS-ON-GRADE FOR THE FIRST 72 HOURS BY THE USE OF:

A)

FOG SPRAYING

B)

PONDING

C)

SPRINKLING

D)

CONTINUOUSLY WET ABSORPTIVE MATS OR FABRIC

E)

CONTINUE CURING BY USE OF MOISTURE RETAINING COVER UNTIL CONCRETE HAS OBTAINED ITS SPECIFIED 28 DAY COMPRESSIVE STRENGTH.

F)

OR LIQUID CURING COMPOUND AFTER FINISHING PROCESS IS COMPLETED.

G)

CONCRETE WET CURE TIME TO BE 7 DAYS MINIMUM AT 50 DEGREES MINIMUM TEMPERATURE.

37.

SUBMIT MATERIALS AND METHOD OF CURING FOR REVIEW.

38.

DO NOT USE MOISTURE RETAINING CURING COMPOUNDS FOR CURING SURFACES TO RECEIVE CARPET, FLEXIBLE FLOORING, CERAMIC TILED FLOORS OR OTHER SPECIFIED FLOOR SYSTEMS, UNLESS IT HAS BEEN DEMONSTRATED THAT SUCH COMPOUNDS WILL NOT PREVENT BOND.

39.

DO NOT PERMIT CONCRETE NOT FULLY CURED TO BE EXPOSED TO EXCESSIVE TEMPERATURE CHANGES OR HIGH WINDS.

40.

POUR ALL GROUND SLABS ON 10 MIL MINIMUM VAPOR RETARDER IN COMPLIANCE WITH ASTM E1745, LAPPED 6” MINIMUM AND FULLY TAPED.

41.

EQUIPMENT MADE OF ALUMINUM OR ALUMINUM ALLOYS, SHALL NOT BE USED FOR PUMP LINES, TREMIES, OR CHUTES OTHER THAN SHORT CHUTES SUCH AS THOSE USED TO CONVEY CONCRETE FROM A TRUCK MIXER.

42.

THE CODE PROHIBITS THE USE OF ALUMINUM (CONDUIT, PIPES, ETC.) IN STRUCTURAL CONCRETE UNLESS IT IS EFFECTIVELY COATED OR COVERED.

DRILL-IN BOLTS, SCREWS AND DOWELS

1.

ADHESIVE DOWELING RODS/BOLTS SHALL BE CARBON STEEL THREADED ROD CONFORMING TO ISO 898 5.8 WITH A MINIMUM TENSILE STRENGTH OF 72.5 KSI (500MPa) AND A MINIMUM YIELD OF 58 KSI (400MPa). THREADED RODS WITH NUTS AND WASHERS INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

2.

ANCHORING ADHESIVE SHALL BE A TWO-COMPONENT SYSTEM SUPPLIED IN MANUFACTURER'S STANDARD SIDE-BY-SIDE FOIL PACKAGE AND DISPENSED THROUGH A STATIC-MIXING NOZZLE SUPPLIED BY THE MANUFACTURER. ADHESIVE SHALL BE TESTED AND APPROVED TO MEET THE MINIMUM REQUIREMENTS OF ACI 355.4 FOR CRACKED AND UNCRACKED CONCRETE RECOGNITION. PROVIDE HILTI HY 200 SAFE SET (ESR 3187) OR RE 500 V3 (ESR 3814) ANCHORS BY HILTI OR EQUAL (E.G. SIMPSON SET-XP, ATC ULTRABOND 365CC)UNLESS SPECIFIED OTHERWISE IN THE STRUCTURAL DOCUMENT.

3.

DRILL-IN REBAR DOWELS SHALL BE SET USING A TWO-PART ADHESIVE AS DESCRIBED ABOVE.

4.

EXPANSION BOLTS SHALL BE HILTI KB TZ (ESR 1917) OR EQUAL. BOLT SHALL MEET DUCTILITY REQUIREMENTS OF ACI 318 SECTION D1.

5.

EXPANSION BOLTS SHALL HAVE CARBON STEEL ANCHOR BODY AND NUT AND WASHER SHALL BE ELECTROPLATED ZINC COATING CONFORMING TO ASTM B633 TO A MINIMUM OF 5µm. THE STAINLESS STEEL ANCHOR BODY, NUT AND WASHER, AND EXPANSION SLEEVE SHALL CONFORM TO TYPE 316 STAINLESS STEEL. EXPANSION ANCHORS SHALL MEET THE MINIMUM REQUIREMENTS OF ACI 355.2 FOR CRACKED AND UNCRACKED CONCRETE. INSTALL IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

6.

MASONRY SCREWS SHALL BE 1/4” DIAMETER WITH 1–5/8” MINIMUM EMBEDMENT INSTALLED IN DRILLED HOLES USING AN APPROPRIATE BIT DIAMETER.

7.

SCREWS SHALL HAVE A BODY MADE OF CARBON STEEL AND SHALL BE HEAT TREATED AND SHALL HAVE 8µm ZINC COATING IN ACCORDANCE WITH EN ISO 4042. PROVIDE HUS EZ (ESR 3027) SCREWS BY HILTI OR EQUAL.

DRAWING INDEX

S-0.01 STRUCTURAL SPECIFICATIONS

S-0.02 STRUCTURAL SPECIFICATIONS AND SCHEDULES

S-1.01 PARTIAL FOUNDATION PLAN

S-1.02 PARTIAL LEVEL 2 FRAMING PLAN

S-2.01 FOUNDATION DETAILS

S-3.01 FRAMING DETAILS

S-4.01 ESCALATOR SECTION

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AA26001957

REVISION	NO.	DESCRIPTION	DATE

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DOCUMENTS

CLIENT  
**NORTHWEST  
FLORIDA BEACHES  
INTERNATIONAL  
AIRPORT (ECP)**

PROJECT TITLE

**PANAMA CITY AIRPORT NWFBIA:  
ESCALATOR  
ADDITION**

**6300 WEST BAY PKWY,  
PANAMA CITY, FL 32409**

PROJECT NUMBER

NO. 21049

ISSUE DATE

3.15.2023

DRAWING TITLE

**STRUCTURAL  
SPECIFICATIONS**

SHEET NUMBER

**S-0.0.1**



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**INTERNATIONAL**  
**AIRPORT (ECP)**



PROJECT TITLE

**PANAMA CITY AIRPORT NWFBI:**

## ESCALATOR ADDITION

**6300 WEST BAY PKWY,**  
**PANAMA CITY, FL 32409**

PROJECT NUMBER

NO. 21049

ISSUE DATE

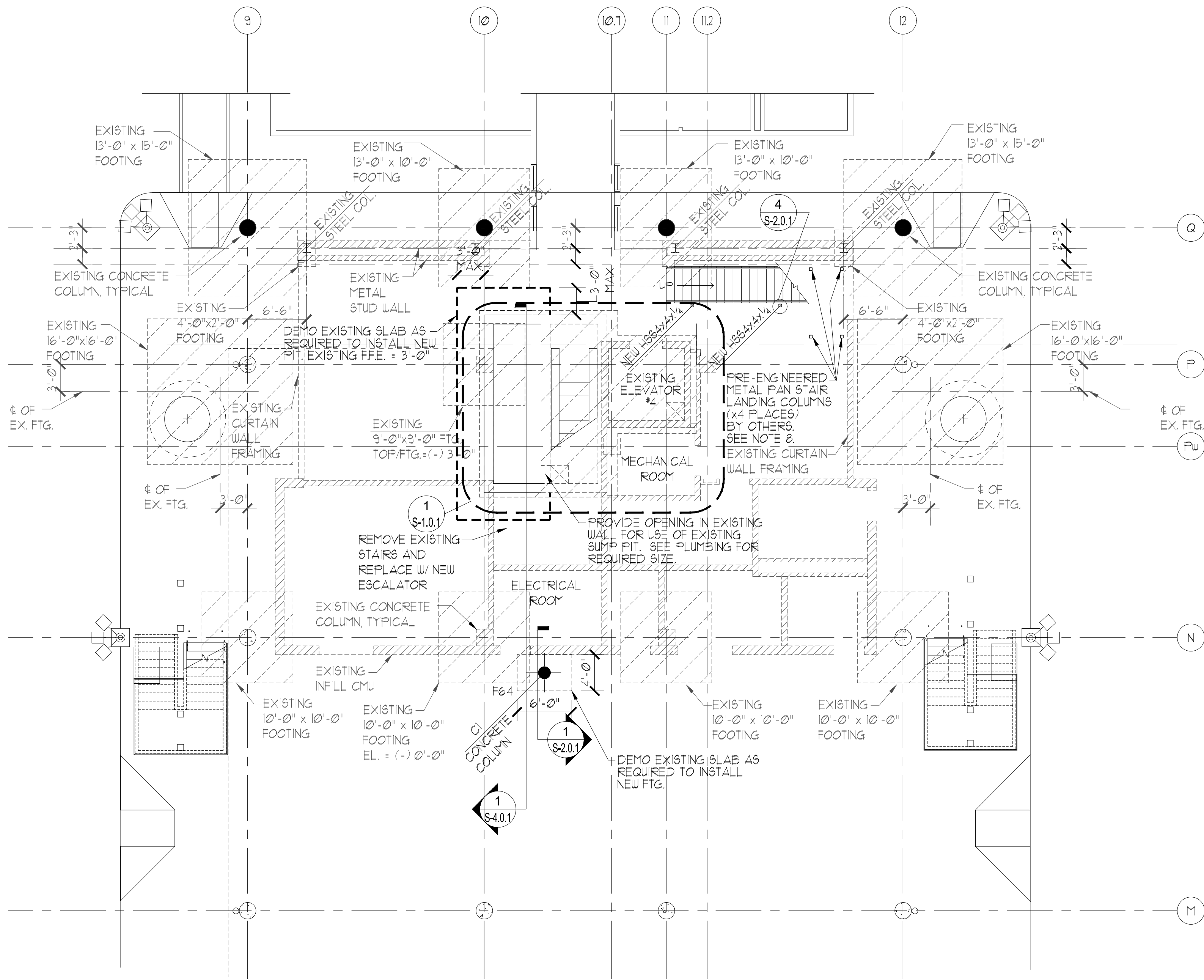
3.15.2023

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## PARTIAL FOUNDATION PLAN

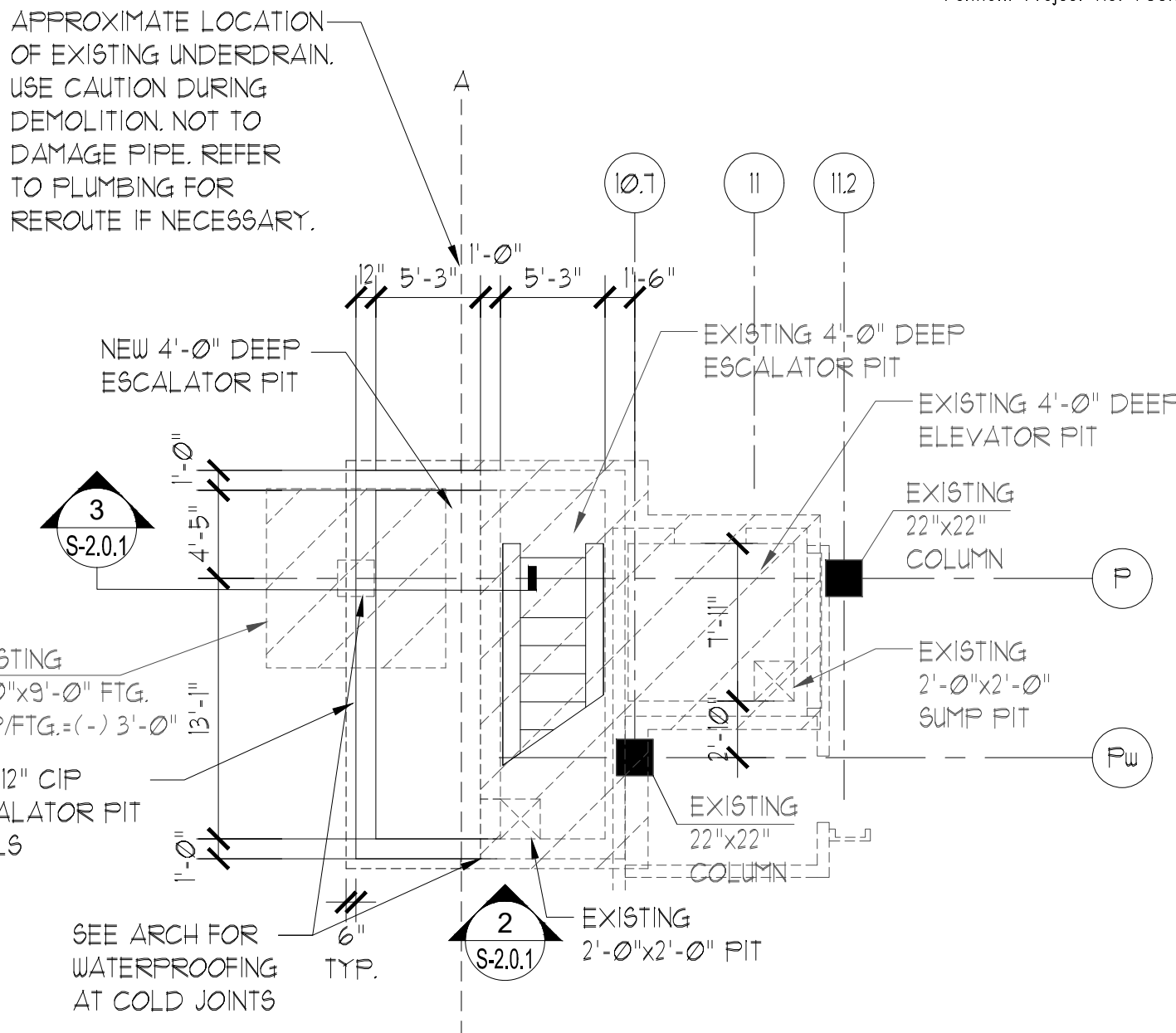
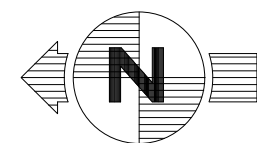
SHEET NUMBER

**S-1.0.1**



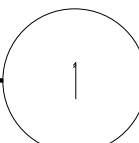
## PARTIAL FOUNDATION PLAN

SCALE: 1/8" = 1'-0"



## ESCALATOR PIT PARTIAL PLAN

SCALE: 1/8" = 1'-0"



### FOUNDATION PLAN NOTES:

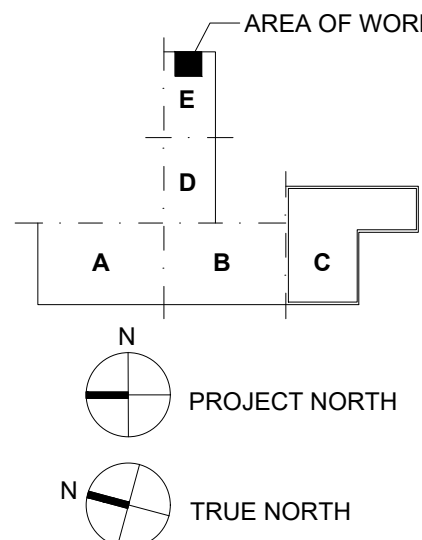
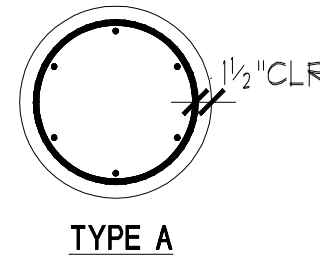
- TOP OF FOOTING = SEE PLAN  
TOP OF EXISTING SLAB = 3'-0"
- CENTER ALL FOOTINGS BELOW COLUMN UNO.
- REFER TO SHEET S-1.0.1 FOR FOOTING SCHEDULE.
- SEE S-3.0.1 FOR TYPICAL FOUNDATION DETAILS.
- VERIFY ALL DIMENSIONS W/ ARCH. AND EXISTING CONDITIONS PRIOR TO FABRICATION AND CONSTRUCTION.
- REFER TO SHEETS S-0.0.1 AND S-0.0.2 FOR SPECIFICATIONS.
- INFORMATION ON THE EXISTING BUILDING, SHOWN ON THESE PLANS, IS OBTAINED FROM EXISTING BUILDING PLANS BY HNTB ARCHITECTURE & PBS & J, DATED MAY 22, 2008 & DECEMBER 5, 2008 (ISSUED FOR CONSTRUCTION). EXISTING INFORMATION DOES NOT NECESSARILY REFLECT AS-BUILT CONDITIONS. THE CONTRACTOR SHALL VERIFY ALL INFORMATION SHOWN ON THESE PLANS AND NOTIFY THE ENGINEER OF ANY VARIATION.
- PRE-ENGINEERED METAL PAN STAIRS TO BE DESIGNED BY A FLORIDA LICENSED DELEGATED ENGINEER FOR 100 PSF LIVE LOAD. SUBMIT SIGNED AND SEALED CALCULATIONS AND SHOP DRAWINGS, INCLUDING ATTACHMENT TO STRUCTURE.

### FOOTING SCHEDULE

MARK	SIZE	DEPTH	REINF. EA. WAY	REMARKS	DWL/A.B. EMBEDMENT
F64	6'-0" x 4'-0"	1'-0"	(1) #6 x 3'-6" (4) #6 x 5'-6"		9"

### CONCRETE COLUMN SCHEDULE

MARK	SIZE	REINFORCING		REMARKS
		VERT.	TIES	
C1	16"Ø	(6) #6	#3 @ 8"	TYPE A





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FLORIDA BEACHES  
INTERNATIONAL  
AIRPORT (ECP)**



PROJECT TITLE

**PANAMA CITY AIRPORT NWFBI:  
ESCALATOR  
ADDITION**

**6300 WEST BAY PKWY,  
PANAMA CITY, FL 32409**

PROJECT NUMBER

NO. 21049

ISSUE DATE

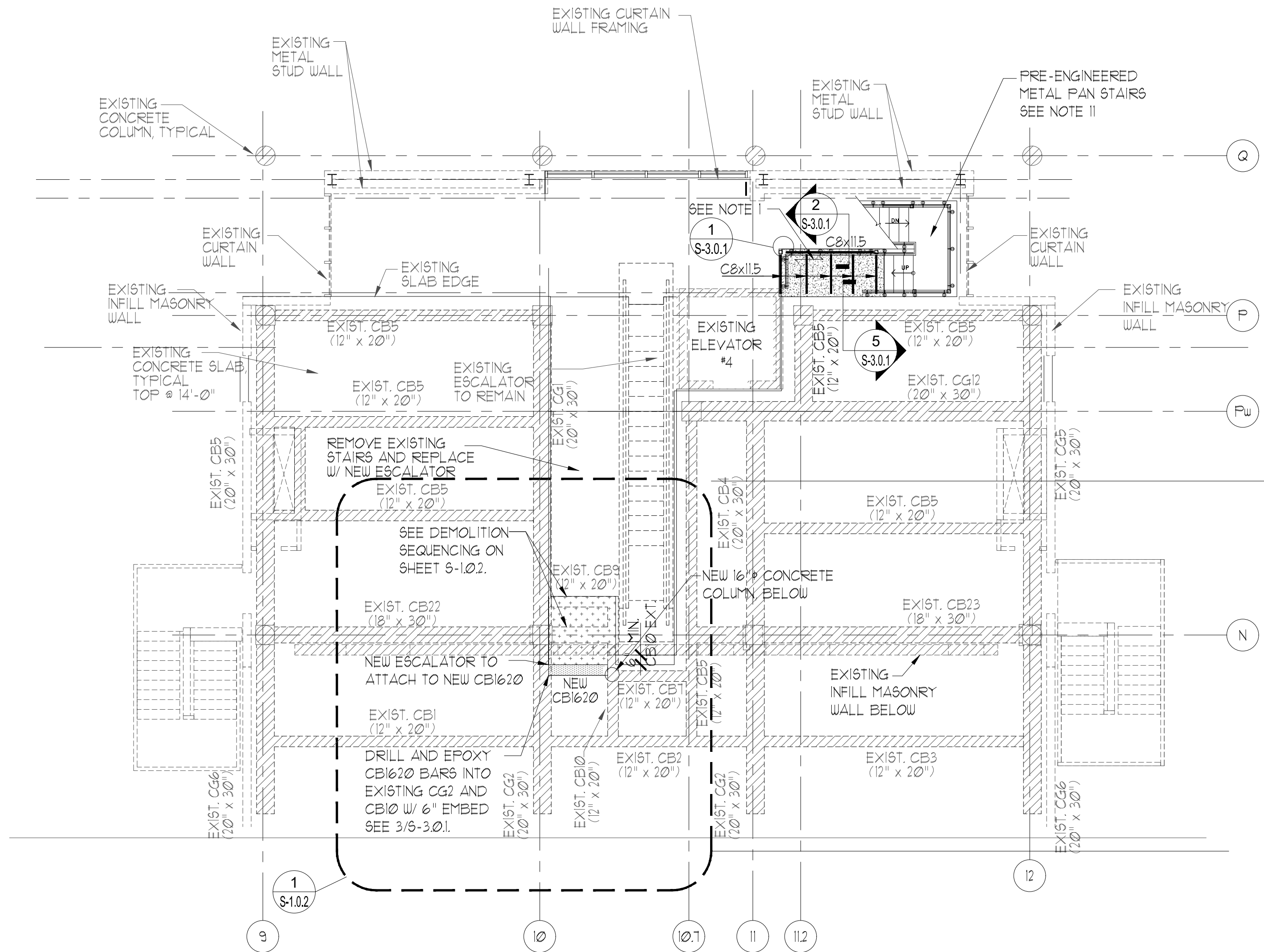
3.15.2023

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**PARTIAL  
LEVEL 2  
FRAMING PLAN**

SHEET NUMBER

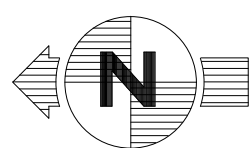
**S-1.0.2**



## PARTIAL LEVEL 2 FRAMING PLAN

SCALE: 1/8" = 1'-0"

PRE-DEMOLITION

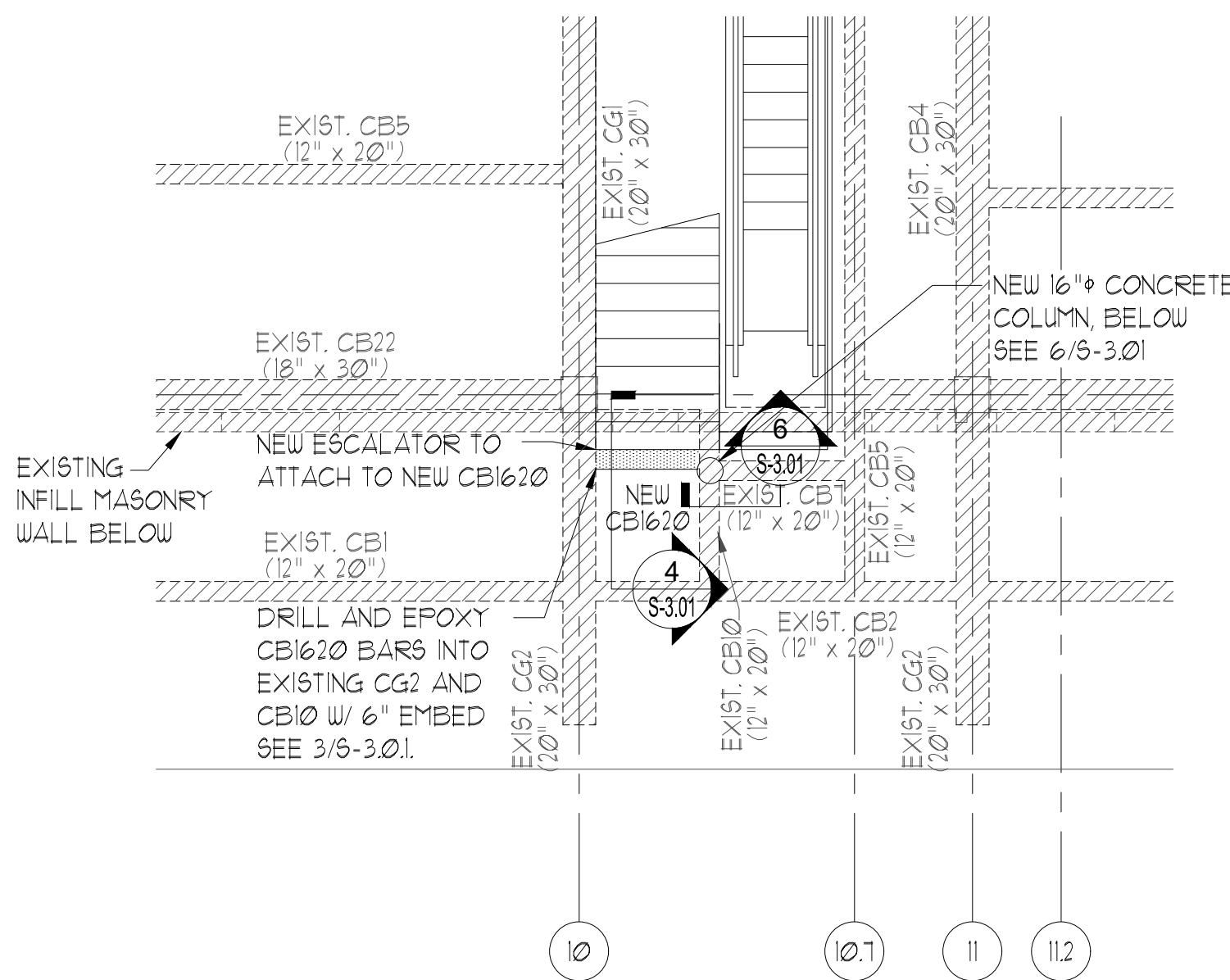


### FLOOR FRAMING PLAN NOTES:

- FLOOR SLAB SHALL BE 5" NORMAL WEIGHT STRUCTURAL CONCRETE ON 3/16" METAL FORM DECK. SLAB SHALL BE REINFORCED WITH 6x6- W2.1xW2.1 WWF. UNO. (↔) INDICATES DIRECTION OF DECK SPAN.
- INSTALL DECK IN (3) SPAN CONDITION, MIN.
- TOP OF FLOOR SLAB EL. = (+) 14'-0" (MATCH EXISTING FIELD VERIFY) UNO.  
TOP OF STEEL BEAM EL. = (+) 13'-11" UNO.
- REFER TO ARCH. DRAWINGS FOR EDGE OF SLAB CONDITIONS.
- REFER TO ELECTRICAL DRAWINGS FOR LEVEL 2 SLAB CORE HOLE LOCATIONS. GENERAL CONTRACTOR TO USE GPR, X-RAY, OR SIMILAR METHOD TO ENSURE NO REINFORCING IS CUT AT EACH LOCATION. CONTACT A/E IF HOLES ARE CLOSER THAN 5 FT. APART, OR LARGER THAN 6" DIAMETER.
- REFER TO SHEET S-0.0.1 AND S-0.0.2 FOR SPECIFICATIONS.
- BEAMS ARE EQUALLY SPACED BETWEEN SUPPORTS. UNO.
- REFER TO SHEETS S-3.0.1 FOR TYPICAL FRAMING DETAILS.
- REFER TO ARCH. FOR ANY DIMENSIONS OR ELEVATIONS MISSING OR DISCREPANCIES. VERIFY PRIOR TO FABRICATION AND INSTALLATION.
- INFORMATION ON THE EXISTING BUILDING, SHOWN ON THESE PLANS, IS OBTAINED FROM EXISTING BUILDING PLANS BY HNTB ARCHITECTURE & PBS & J, DATED MAY 22, 2008 & DECEMBER 5, 2008 (ISSUED FOR CONSTRUCTION). EXISTING INFORMATION DOES NOT NECESSARILY REFLECT AS-BUILT CONDITIONS. THE CONTRACTOR SHALL VERIFY ALL INFORMATION SHOWN ON THESE PLANS AND NOTIFY THE ENGINEER OF ANY VARIATION.
- PRE-ENGINEERED METAL PAN STAIRS TO BE DESIGNED BY A FLORIDA LICENSED DELEGATED ENGINEER FOR 100 PSF LIVE LOAD. SUBMIT SIGNED AND SEALED CALCULATIONS AND SHOP DRAWINGS, INCLUDING ATTACHMENT TO STRUCTURE.

### DEMOLITION SEQUENCING

- INSTALL NEW CONCRETE COLUMN AND FOOTING WHERE EXISTING CB1 AND CB10 MEET.
- SHORE EXISTING SLAB WEST OF NEW CB1620.
- AFTER NEW COLUMN HAS REACHED 4,000 PSI COMPRESSIVE STRENGTH, DEMO. EXISTING CB3, CB22, CB10 (PARTIAL, VERIFY 6" CANTILEVER EAST OF NEW CB1620 TO REMAIN), AND SLAB.
- INSTALL NEW CB1620.
- AFTER NEW CB1620 HAS REACHED 4,000 PSI COMPRESSIVE STRENGTH, SHORING MAY BE REMOVED.



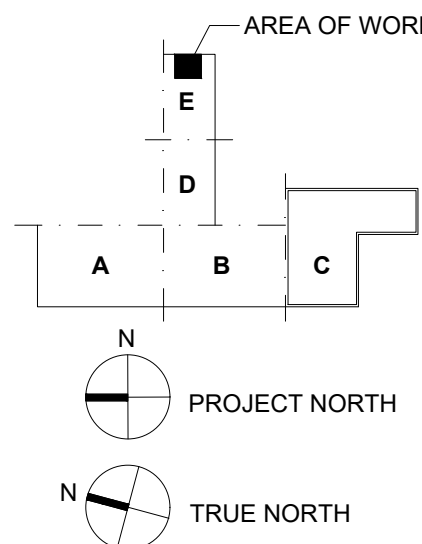
## POST DEMOLITION LEVEL 2 FRAMING PLAN

SCALE: 1/8" = 1'-0"

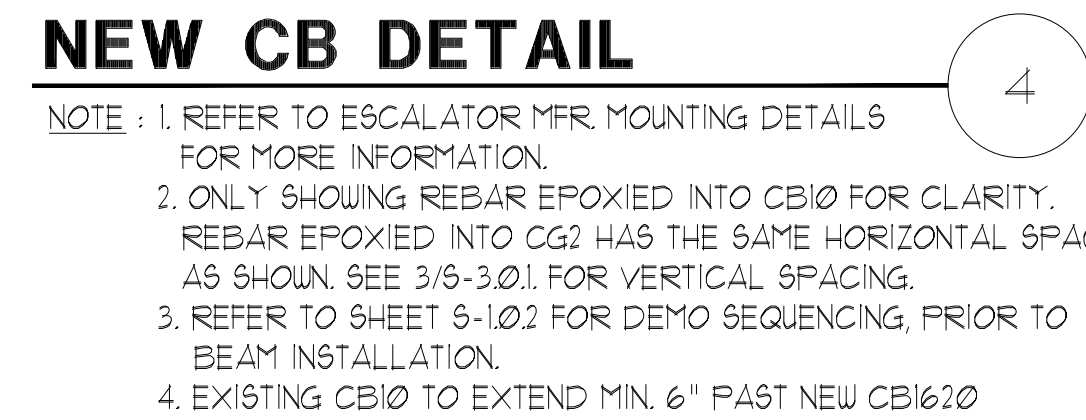
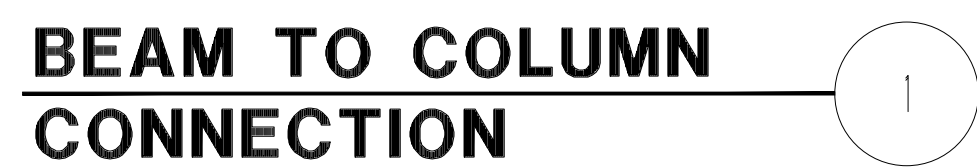
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### NOTES:

- CONTACT A/E ONCE FINAL LOCATION OF CB1620 IS DETERMINED, PRIOR TO THE NEW COLUMN AND FOOTING INSTALLATION.



## S-2.0.1





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AIRPORT (ECP)**



PROJECT TITLE  
**PANAMA CITY AIRPORT NWFBI  
ESCALATOR  
ADDITION**

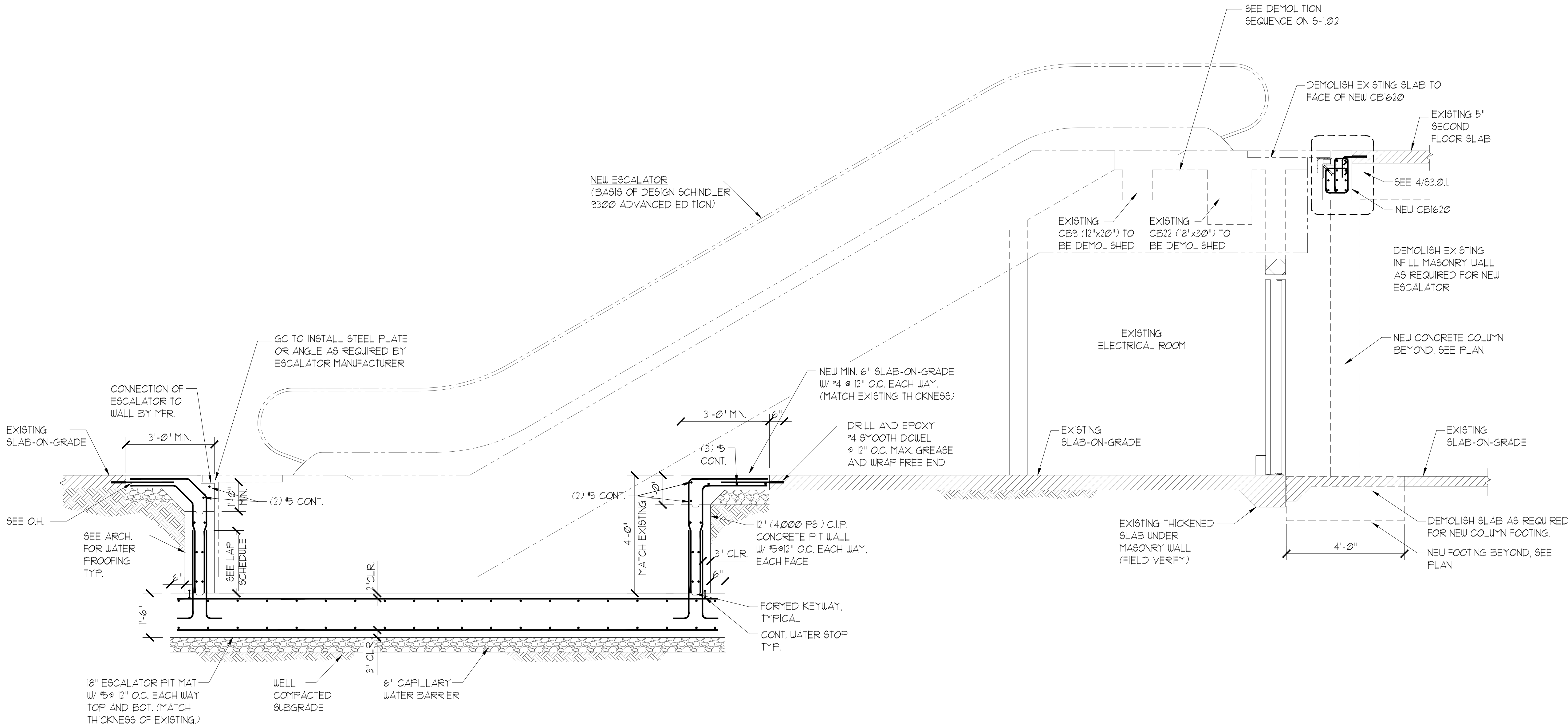
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DRAWING TITLE  
**ESCALATOR  
SECTION**

SHEET NUMBER  
**S-4.0.1**



## ESCALATOR SECTION

SCALE : 1/2" = 1'-0"

NOTES:  
1. SEE DEMOLITION SEQUENCE ON S-1.02